## TRAFFIC IMPACT STUDY

REPORT FOR:

## PRAIRIE WINDS OF ST. CHARLES



BRICHER ROAD WEST OF RANDALL ROAD ST. CHARLES, ILLINOIS

PREPARED BY:


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7325 Janes Avenue
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V3 Project No. 16262
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## I. INTRODUCTION

V3 Companies has been retained by Executive Capital Corporation to conduct a traffic impact study for a proposed multi-family residential development located along the north side of Bricher Road and west of Randall Road in the City of St. Charles, Kane County, Illinois. The name of the proposed development is Prairie Winds of St. Charles (PW). The property is bounded by Bricher Road to the south, undeveloped land to the west, cultivated land to the north, and a retail development to the east. A site location map is included as Figure 1.

The proposed multifamily residential development will consist of 25 buildings with a total of 250 rental units. A clubhouse facility is also planned on the site. The proposed access plan consists of a full access driveway on Bricher Road that aligns with Camden Street and a second full access driveway west of Camden Street. A conceptual site plan is included as Figure 2.

The purpose of this report is to evaluate the potential traffic impacts of the proposed development which is expected to be built out in 2017. Traffic estimates are projected for 2022, which is five years beyond the opening date. The study area consists of the existing intersection of Bricher Road and Camden Street, which will align with the proposed primary site driveway, and the proposed site driveway to the west. It is assumed that new traffic generated by the proposed development will not negatively impact operations of the signalized intersections on Bricher Road to the east due to the additional capacity along the roadway and at the signalized intersections. Therefore, the intersection of Bricher Road and Randall Road is not included in this study.

This report includes a description of existing conditions, data collection, capacity analysis, evaluation of data, and conclusions.


PRAIRIE WINDS OF ST. CHARLES

FIGURE 1
SITE LOCATION MAP


## II. PROJECT CONDITIONS

## Land Uses

A variety of land uses exist near the project site, primarily consisting of institutional, residential, retail and service uses. The surrounding land uses are illustrated in Figure 3.

## Roadway System

The characteristics of the roadways in the vicinity of the site are presented below. The existing lane configurations at the study area intersections are illustrated in Figure 4.

Bricher Road is a three lane major collector under the jurisdiction of the City of Geneva. A striped median is provided through the entire site frontage. The speed limit on Bricher Road changes along the site frontage, with a limit of 45 miles per hour to the west and 40 miles per hour to the east. The signs indicating the speed limit change are located approximately 300 feet west of the Camden Street intersection. A westbound left turn lane and an eastbound right turn lane are provided on the approaches to Camden Street.

Camden Street is a two lane residential street that serves the Lincoln Square subdivision. No outlets are provided on Camden Street, so all Lincoln Square traffic travels through the intersection of Bricher Road and Camden Street. The northbound approach to Bricher Road consists of a 20 foot lane with no pavement marking. Although the section is wide enough to accommodate one left turn lane and one right turn lane, the lack of roadway striping results in inconsistent usage. For the purposes of this study, the northbound approach to Bricher Road is considered to consist of one shared lane for all movements.



## Traffic Volumes

To assist in the evaluation of the traffic impact on the roadway system resulting from the proposed development, existing vehicular volumes were collected in the study area.

Vehicle counts were conducted on Wednesday, November 30, 2016 at the intersection of Bricher Road and Camden Street. The morning peak period counts occurred from 7:00 am to 9:00 am and the evening peak period counts occurred from 4:00 pm to 6:00 pm. The count periods were selected to be consistent with traditional peak hours for collector roadways and residential neighborhoods.

The traffic volumes collected indicate that the weekday peak hours occur from 7:00 am to 8:00 am and 5:00 pm to $6: 00 \mathrm{pm}$. The existing peak hour vehicular volumes at the study area intersections are illustrated in Figure 5. A summary of the traffic volumes collected in fifteen minute increments is provided in Appendix $A$.

## Proposed Development

## Land Use Development

The Lincoln Square residential subdivision, managed by Pulte Homes, on the south side of Bricher Road is not currently at full occupancy. There was evidence that approximately 18 homes are occupied during the November 30, 2016 site visit. A total of 47 home lots are advertised in published maps for the Lincoln Square development. It is assumed that all homes will be constructed and fully occupied by the 2022 design year of this study. It is also assumed that the number of trips currently traveling through the intersection of Bricher Road and Camden Street will proportionally increase from the current occupancy of 18 homes to full occupancy of 47 homes as Camden Street is the only access point to Lincoln Square

The City of St. Charles has recently approved the redevelopment of the 27 -acre St. Charles Mall property located north of IL 38 and east of Randall Road, east of the proposed Prairie Winds development. The proposed mixed use development, Prairie Center, will include multi-family residential and commercial uses. It is not anticipated that this development will significantly increase traffic along Bricher Road adjacent to Prairie Winds since the roadway dead ends to the west at Peck Road and other east/west options are available for Prairie Center traffic, such as IL 38 and Prairie Street. Additional traffic that may use Bricher Road to and from Prairie Center is likely included in the background growth that was applied to the existing Bricher Road traffic and included in the future traffic analysis.

There are no other known proposed land development projects in the vicinity of the site that will impact the study area.

## Roadway Development

The conceptual site plan for the proposed development includes two proposed driveways on Bricher Road. The City of St. Charles and City of Geneva have an intergovernmental agreement for access points along the north of Bricher Road. The agreement allows two full access points along the property's frontage of Bricher Road, but not within the area 270 feet east of the center line of Fischer Drive.

The primary driveway, which is aligned with Camden Street, is expected to consist of two lanes with a landscaped median. The lanes are expected to be wide enough to function as a left turn lane and a shared through/right turn lane. The secondary driveway is located approximately 500 feet west of the primary driveway, and forms a three leg intersection with Bricher Road. The secondary driveway is proposed to consist of a two lane section. It is recommended that the striped median on Bricher Road is restriped to provide eastbound left turn lanes at both the primary and secondary driveways. A right-turn lane is proposed on the westbound approach at the primary driveway. Both proposed driveways will be stop controlled on the minor legs of the intersections.

There are no other known proposed roadway projects in the vicinity of the site that will impact the study area. The anticipated future lane configuration for the study area intersections are illustrated in Figure 6.


## LEGEND

\#\# - AM PEAK HOUR
(\#\#)- PM PEAK HOUR

AM PEAK HOUR: 7:00 AM - 8:00 AM PM PEAK HOUR: 5:00 PM - 6:00 PM


## LEGEND

:
(STOP) - stop sign

## III. TRAFFIC FORECASTS

## Project Traffic Volumes

## Trip Generation

The proposed site plan consists of 250 rental units. Project traffic is estimated using the Institute of Transportation Engineers (ITE) Trip Generation Manual, $9^{\text {th }}$ Edition. The following land use categories are used to determine project traffic:

Apartment (ITE Land Use Code 220) - Apartments are rental dwelling units located within the same building with at least three other dwelling units, for example, quadraplexes and all types of apartment buildings.

The Trip Generation Manual assigns trip generation rates based on a peak period and an independent variable. In this case, dwelling units is the applicable variable for apartment. The am and pm trip generation rates are selected as the average rate for weekday, peak hour of adjacent street traffic for one hour from 7 am to 9 am and 4 pm to 6 pm .

A summary of trip generation is provided in Table 1.
Table 1: Trip Generation

| ITE Category Information |  |  | Daily | AM Peak Hour |  |  | PM Peak Hour |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ITE <br> LUC | Land Use | Size | Variable | Trips | In | Out | Total | In | Out | Total |
| 220 | Apartment | 250 | Dwelling <br> Units | 1,663 | 26 | 102 | 128 | 101 | 54 | 155 |

## Trip Distribution and Assignment

The direction from which traffic approaches and departs a site is a function of numerous variables, including location of residences, location of employment centers, location of commercial/retail centers, available roadway systems, location and number of access points, and level of congestion on adjacent road systems.

It is assumed that the distribution of site trips will be comparable to the distribution of existing traffic observed on Bricher Road. The directionality of traffic differs in the am and pm peak hours. During the am peak hour external trips are heavily weighted toward Randall Road to the east, which is the nearest major arterial. The directionality is more evenly split during the pm peak hour, so the directionality of project trips is assumed to be muted as well. It is assumed that traffic slightly favors traveling to and from the east of the site, as commuter traffic is likely to travel on Randall Road.

Site traffic will be split between the primary and secondary driveways. It is anticipated that more than half of the overall traffic will use the primary entrance due the location of the residential buildings on the conceptual site plan and ease of access from Bricher Road. However, a substantial number of dwelling units are located closer to the secondary entrance, which is also expected to experience significant use. A 60/40 split between the primary and secondary driveways is assumed for both peak hours.

The directional distribution and assignment of new project traffic is illustrated in Figure 7.

## Background Traffic Volumes

Background traffic volumes are estimated for the year 2022, which is five years beyond the anticipated build out in 2017. These volumes account for future non-project related growth in the area. The growth rate is determined after reviewing historic average daily traffic (ADT) volumes along Bricher Road west of Randall Road that are available from IDOT. An evaluation of historic daily volumes along Bricher Road near the study area is summarized in Table 2.

Table 2: Historic IDOT ADT Growth Rates - Bricher Road

| Count <br> Year | IDOT <br> ADT | Annual Growth <br> Rate from Previous <br> Count Year |
| :---: | :---: | :---: |
| 2002 | 2150 | - |
| 2006 | 5900 | $43.6 \%$ |
| 2010 | 5300 | $-2.5 \%$ |
| 2014 | 5200 | $-0.5 \%$ |

Substantial ADT growth occurred from 2002 to 2006, which coincides with the construction of several residential developments to the west of the study area. Since 2006, ADT's along Bricher Road have decreased. In order to maintain conservative analysis, a negative growth rate will not be used in this study. Instead, the annual growth rate is assumed to be one percent per year at all study area intersections. This methodology is typical in situations involving potentially negative traffic volume growth rates.

As previously stated, it is assumed that the Lincoln Square residential development to south will be fully constructed and occupied by 2022. Currently, approximately 18 of the 47 potential homes are occupied. The additional projected trips for full occupancy are included in the background condition. The distribution of the projected trips matches the distribution of existing Lincoln Square trips.

The background traffic volumes are illustrated in Figure 8.

## Future Traffic Volumes

The project traffic volumes are added to the background volumes to obtain the future traffic volumes for the study intersections. Future with project traffic volumes are depicted in Figure 9.


## LEGEND

\#\# - AM PEAK HOUR
(\#\#)- PM PEAK HOUR

AM PEAK HOUR: 7:00 AM - 8:00 AM
PM PEAK HOUR: 5:00 PM - 6:00 PM


## LEGEND

\#\# - AM PEAK HOUR
(\#\#)- PM PEAK HOUR

AM PEAK HOUR: 7:00 AM - 8:00 AM PM PEAK HOUR: 5:00 PM - 6:00 PM

## NOTE:

| BACKGROUND | $=$ | EXISTING | $+\quad$ ADDITIONAL | $+1 \%$ GROWTH |
| :---: | :---: | :---: | :---: | :---: |
| TRAFFIC | TRAFFIC | LINCOLNSQUARE | PERYEAR |  |
| VOLUME | VOLUME | TRAFFIC | TO 2022 |  |



## IV. TRAFFIC ANALYSIS

## Capacity Analysis

The operation of a facility is evaluated based on level of service (LOS) calculations obtained by analytical methods defined in the Transportation Research Board's Highway Capacity Manual (HCM), 2010 Edition. The concept of LOS is defined as a quality measure describing operational conditions within a traffic stream, generally in terms of such service measures as speed and travel time, freedom to maneuver, traffic interruptions, and comfort and convenience.

There are six LOS letter designations, from A to $F$, with LOS A representing the best operating conditions and LOS F the worst.

The LOS of an intersection is based on the average control delay per vehicle. For a signalized intersection, the delay is calculated for each lane group and then aggregated for each approach and for the intersection as a whole. Generally, the LOS is reported for the intersection as a whole. For an unsignalized intersection, the delay is only calculated and reported for each minor movement. An overall intersection LOS is not calculated.

There are different LOS criteria for signalized and unsignalized intersections primarily due to driver perceptions of transportation facilities. The perception is that a signalized intersection is expected to carry higher traffic volumes and experience a greater average delay than an unsignalized intersection. Typically, various state and local governments adopt operating standards varying between LOS C and LOS E, depending on the area's size and roadway characteristics. The LOS criteria for signalized and unsignalized intersections are provided in Table 3.

Table 3: Level of Service Definitions for Signalized and Unsignalized Intersections

| Level of Service | Signalized Intersection <br> Control Delay <br> (seconds/vehicle) | Unsignalized Intersection <br> Control Delay <br> (seconds/vehicle) |
| :---: | :---: | :---: |
| A | $\leq 10$ | $\leq 10.0$ |
| B | $>10.0$ and $\leq 20.0$ | $>10.0$ and $\leq 15.0$ |
| C | $>20.0$ and $\leq 35.0$ | $>15.0$ and $\leq 25.0$ |
| D | $>35.0$ and $\leq 55.0$ | $>25.0$ and $\leq 35.0$ |
| E | $>55.0$ and $\leq 80.0$ | $>35.0$ and $\leq 50.0$ |
| F | $>80.0$ | $>50.0$ |

Source: Transportation Research Board, Highway Capacity Manual 2010, National Research
Council, 2010.
The study area consists of the two proposed site driveways on Bricher Road. The primary driveway aligns with Camden Street. Capacity analysis is performed with HCS 2010 (ver. 6.90). Multiple HCS 2010 scenarios are created to evaluate the existing, background, and future with project traffic volumes for the weekday am and pm peak hours. Results for the study
intersections are summarized in Table 4. Supporting analysis worksheets from HCS 2010 for the existing, background and future traffic conditions are provided in Appendices B, C and D.

Table 4: Unsignalized Intersection LOS

| Intersection / Approach | AM Peak Hour |  |  |  |  |  | PM Peak Hour |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Existing |  | Background |  | Future w/ Project |  | Existing |  | Background |  | Future w/ Project |  |
|  | Delay (s/veh) | LOS | Delay (s/veh) | LOS | Delay (s/veh) | LOS | Delay (s/veh) | LOS | Delay (s/veh) | LOS | Delay <br> (s/veh) | LOS |

Bricher Road \& Camden Street/Primary Driveway (East Driveway)

| NB Approach | 10.4 | B | 10.9 | B | 11.4 | B | 10.6 | B | 11.0 | B | 11.7 | B |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SB Left | - | - | - | - | 14.4 | B | - | - | - | - | 18.2 | C |
| SB Thru/Right | - | - | - | - | 8.9 | A | - | - | - | - | 10.5 | B |
| EB Left | - | - | - | - | 7.5 | A | - | - | - | - | 8.2 | A |
| WB Left | 7.9 | A | 8.0 | A | 8.0 | A | 7.8 | A | 7.9 | A | 8.0 | A |

Bricher Road \& Secondary Driveway (West Driveway)

| SB Approach | - | - | - | - | 11.6 | B | - | - | - | - | 13.1 | B |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| EB Left | - | - | - | - | 7.5 | A | - | - | - | - | 8.2 | A |

Under existing conditions, the minor movements at the intersection of Bricher Road and Camden Street experience little delay with all movements operating at LOS A or LOS B during the am and pm peak hours. Delay increases slightly for all movements in the background scenario but the levels of service do not change.

The addition of project related trips has little impact on performance of the study area intersections during the am and pm peak hours, with only slight delay increases for the existing movements in the future with project scenario. There are no changes in level of service. Likewise, the new minor movements at the two site driveways operate at LOS C or better during both the am and pm peak hours.

Therefore, it is concluded that the proposed development does not adversely impact the study area intersections on Bricher Road with the restriping of the median to provide eastbound left turn lanes at both site driveways. No other mitigation is necessary.

## Queue Length Analysis

The $95^{\text {th }}$ percentile queue lengths are also analyzed using HCS 2010. The applicable minor movement queue lengths for the am and pm peak hours are summarized in Table 5.

All queue storage lengths are adequate in the existing, background, and future with project scenarios. The two proposed southbound driveway approaches are expected to mirror the northbound approach of Camden Street in that the approach travel lane is the storage lane. Therefore, the storage length will be equal to the throat length within the site and there is not an applicable taper length. The queue lengths at the two proposed eastbound left turn lanes are expected to be quite short and will not be the controlling factor for storage length. Therefore the
design defers to the IDOT BRL Manual requirements for queue storage and taper lengths based on the posted speed limit of 40 miles per hour at the East Driveway and 45 miles per hour at the West Driveway. However, a variance could be considered for one or both of the driveways to allow shorter storage and taper lengths.

Table 5: $95^{\text {th }}$ Percentile Queue Lengths

| Intersection | Scenario | 95\% Queue Length (feet) |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM Peak Hour |  |  |  | PM Peak Hour |  |  |  |
|  |  | EB Left | WB Left | NB App. | SB Left | EB Left | WB Left | NB App. | SB Left |
| Bricher Road and Camden Street/ Primary Driveway | Existing | - | 0 | 3 | - | - | 0 | 0 | - |
|  | Background | - | 0 | 5 | - | - | 0 | 3 | - |
|  | Future with Project | 0 | 0 | 5 | 10 | 3 | 0 | 3 | 5 |
|  | Existing Storage | $\mathrm{N} / \mathrm{A}^{2}$ | 185 | $75^{1}$ | $N / A^{4}$ | N/A ${ }^{2}$ | 185 | $75^{1}$ | $N / A^{4}$ |
|  | Existing Taper | $N / A^{2}$ | 95 | N/A ${ }^{1}$ | $N / A^{4}$ | $N / A^{2}$ | 95 | N/A ${ }^{1}$ | $N / A^{4}$ |
| Bricher Road and Secondary Driveway | Existing | - | - | - | - | - | - | - | - |
|  | Background | - | - | - | - | - | - | - | - |
|  | Future with Project | 0 | - | - | 5 | 0 | - | - | 5 |
|  | Existing Storage | $N / A^{3}$ | - | - | $N / A^{4}$ | N/A ${ }^{3}$ | - | - | $N / A^{4}$ |
|  | Existing Taper | $N / A^{3}$ | - | - | $N / A^{4}$ | $N / A^{3}$ | - | - | $N / A^{4}$ |

Notes:

1. Approximately 75 feet from stop bar to multi-use path crossing
2. Proposed lengths based on BLR standards for 40 speed limit - 115' Storage/132' Taper min.
3. Proposed lengths based on BLR standards for 45 speed limit - 115' Storage/156' Taper min.
4. Travel lane is the storage lane as no add lanes are proposed for SB approach

## Traffic Signal Warrant Analysis

It is worthwhile to conduct a traffic signal warrant analysis given the addition of an approach to the intersection of Bricher Road and Camden Street, and an overall increase in traffic associated with the proposed development. The investigation for the need for a traffic control signal is based on the methodology established in the Manual on Uniform Traffic Control Devices (MUTCD). The MUTCD establishes nine individual warrants. Installation of a traffic signal should be further investigated at locations that meet one or more warrants. Analysis worksheets for the appropriate warrants are included in Appendix E.

Warrant 1 is the eight-hour vehicular volume warrant. Warrant 1 is met if a total of 8 hours in the day exceed the thresholds established in the MUTCD. Traditionally this warrant requires more than eight hours of data collection and substantial projections of future trips. However, additional guidance from IDOT declares that in cases involving future volumes, the eight hour vehicular volume hour can be estimated as 55 percent of the peak hour volumes. The IDOT methodology also requires a reduction of the minor approach right turn volume based on factors such as lane configuration and conflicting volumes. Ultimately, the eight-hour vehicular warrant is not met at the intersection of Bricher Road and Camden Street using 55 percent of the pm peak hour volumes.

Warrant 2 is the four-hour vehicular volume warrant. The conditions in this warrant are intended to be applied where the volume of intersecting traffic is the principal reason to consider installing a traffic signal. The MUTCD establishes the thresholds for major and minor street traffic. The traffic projections in this study established only the am and pm peak hours, both of which do not meet this warrant.

Although the MUTCD describes a warrant based on peak hour volumes (Warrant 3), this warrant is intended to be used in cases that are expected to have substantial spikes in minor approach traffic, such a large office building with many employees departing at the same time. However, the projected peak hour intersection volumes do not meet Warrant 3.

Overall, the projected future volumes at Bricher Road and Camden Street/Primary Driveway do not meet the eight-hour, four-hour, or peak hour warrants. Therefore, it is concluded that a traffic signal will not be warranted at this intersection following the completion of the proposed development.

## Right Turn Lane Warrant Analysis

The addition of right-turn lanes at intersections can significantly improve operations if warranted. Right turn warrant criteria is provided both by IDOT and Kane County DOT. It is likely that the KDOT criteria controls over the criteria present in the IDOT BDE and BLR, but both criteria are included in this report.

The IDOT criteria is based on only the approach volume and right turn volume, with no separate consideration for the speed limit in most cases. When the future with project volumes are plotted against the IDOT threshold, it is apparent that the volumes are well below the threshold that would require a right turn lane to be considered at either intersection. Therefore, it is concluded that right turn lanes are not required at the two proposed driveways per IDOT requirements.

Kane County DOT requirements differ from IDOT since the posted speed limit is also considered, along with the approach and right turn volumes. The posted speed limit is 40 miles per hour at the Primary Driveway and 45 mph and the Secondary Driveway. In both of these scenarios, the plotted right turn volume is below the warrant threshold. Therefore, it is concluded that right turn lanes are not required at the two proposed driveways, per Kane County DOT requirements.

Figures displaying the plotted right turn lane threshold criteria for both IDOT and Kane County DOT requirements are included as Appendix F.

It should be noted that despite falling below the criteria requiring a right turn lane, a right turn lane is proposed on the westbound approach at the Primary Driveway. The addition of the lane highlights the primary nature of this driveway and matches the lane configuration of the eastbound approach. A right turn lane is not proposed at the secondary driveway.

## v. CONCLUSIONS

The purpose of this report is to evaluate the potential traffic impacts of the proposed 250 unit Prairie Winds rental community located on Bricher Street in St. Charles, Illinois. The conceptual site plan includes a primary full access driveway aligned with Camden Street, and a secondary full access driveway located approximately 500 feet to the west. It is recommended that eastbound left turn lanes be striped within the existing median at the two proposed site driveways.

Capacity analysis was conducted for existing, background, and future with project conditions during the am and pm peak hours at the intersection of Bricher Road and Camden Street, as the site driveways. Traffic was estimated to 2022, which is five years beyond the anticipated opening of the Prairie Winds development. The projects also include additional trips on Camden Street to account for future growth in the Lincoln Square development to the south.

Results of the capacity and queue analysis indicate that there are no performance issues at the study area intersection in either the am or pm peak hours. No additional roadway improvements are recommended.

Despite the addition of a fourth approach and an overall increase in traffic, the intersection of Bricher Road and Camden Street/Primary Driveway does not meet the eight hour, four hour, or peak hour traffic signal warrants. Right turn lane warrant analysis confirms that the future volumes at the proposed driveways do not meet warrants. However, a right turn lane is proposed on the westbound approach to the Primary Driveway to highlight the primary nature of this driveway and to match the configuration of the existing eastbound approach.

## APPENDIX A

## EXISTING TRAFFIC COUNTS

## V3 Companies

7325 Janes Avenue
Woodridge, IL 60517

Project: Randall \& Bricher
Location: St. Charles, Illinois
Weather: Dry
Counted by: V3

File Name : Camden and Bricher
Site Code : 16262
Start Date : 11/30/2016
Page No : 1

|  | CAMDEN STREET Northbound |  |  |  | BRICHER ROAD Eastbound |  |  |  | BRICHER ROAD Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Right | Peds | App. Total | Thru | Right | Peds | App. Total | Left | Thru | Peds | App. Total | Int. Total |
| 07:00 AM | 3 | 1 | 0 | 4 | 66 | 2 | 0 | 68 | 1 | 19 | 0 | 20 | 92 |
| 07:15 AM | 0 | 0 | 0 | 0 | 104 | 0 | 0 | 104 | 0 | 28 | 0 | 28 | 132 |
| 07:30 AM | 0 | 2 | 0 | 2 | 58 | 1 | 0 | 59 | 2 | 24 | 0 | 26 | 87 |
| 07:45 AM | 1 | 6 | 0 | 7 | 73 | 0 | 0 | 73 | 2 | 35 | 0 | 37 | 117 |
| Total | 4 | 9 | 0 | 13 | 301 | 3 | 0 | 304 | 5 | 106 | 0 | 111 | 428 |
| 08:00 AM | 0 | 3 | 0 | 3 | 41 | 1 | 0 | 42 | 1 | 30 | 0 | 31 | 76 |
| 08:15 AM | 0 | 2 | 0 | 2 | 55 | 0 | 0 | 55 | 0 | 20 | 0 | 20 | 77 |
| 08:30 AM | 1 | 0 | 0 | 1 | 53 | 0 | 0 | 53 | 4 | 31 | 0 | 35 | 89 |
| 08:45 AM | 0 | 3 | 0 | 3 | 51 | 1 | 0 | 52 | 1 | 23 | 0 | 24 | 79 |
| Total | 1 | 8 | 0 | 9 | 200 | 2 | 0 | 202 | 6 | 104 | 0 | 110 | 321 |

*** BREAK ***

| 04:00 PM | 1 | 1 | 0 | 2 | 60 | 0 | 0 | 60 | 3 | 71 | 0 | 74 | 136 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:15 PM | 0 | 1 | 0 | 1 | 55 | 0 | 0 | 55 | 0 | 89 | 0 | 89 | 145 |
| 04:30 PM | 0 | 2 | 0 | 2 | 71 | 2 | 0 | 73 | 1 | 71 | 0 | 72 | 147 |
| 04:45 PM | 0 | 0 | 0 | 0 | 46 | 0 | 0 | 46 | 0 | 65 | 0 | 65 | 111 |
| Total | 1 | 4 | 0 | 5 | 232 | 2 | 0 | 234 | 4 | 296 | 0 | 300 | 539 |
| 05:00 PM | 1 | 0 | 0 | 1 | 79 | 0 | 0 | 79 | 0 | 91 | 0 | 91 | 171 |
| 05:15 PM | 0 | 1 | 0 | 1 | 60 | 1 | 0 | 61 | 2 | 90 | 0 | 92 | 154 |
| 05:30 PM | 0 | 2 | 0 | 2 | 76 | 3 | 0 | 79 | 1 | 86 | 0 | 87 | 168 |
| 05:45 PM | 0 | 1 | 0 | 1 | 65 | 0 | 0 | 65 | 4 | 69 | 0 | 73 | 139 |
| Total | 1 | 4 | 0 | 5 | 280 | 4 | 0 | 284 | 7 | 336 | 0 | 343 | 632 |
| Grand Total | 7 | 25 | 0 | 32 | 1013 | 11 | 0 | 1024 | 22 | 842 | 0 | 864 | 1920 |
| Apprch \% | 21.9 | 78.1 | 0 |  | 98.9 | 1.1 | 0 |  | 2.5 | 97.5 | 0 |  |  |
| Total \% | 0.4 | 1.3 | 0 | 1.7 | 52.8 | 0.6 | 0 | 53.3 | 1.1 | 43.9 | 0 | 45 |  |


|  | CAMDEN STREET Northbound |  |  |  | BRICHER ROAD Eastbound |  |  |  | BRICHER ROAD Westbound |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Right | Peds | App. Total | Thru | Right | Peds | App. Total | Left | Thru | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Entire Intersection Begins at 07:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 07:00 AM | 3 | 1 | 0 | 4 | 66 | 2 | 0 | 68 | 1 | 19 | 0 | 20 | 92 |
| 07:15 AM | 0 | 0 | 0 | 0 | 104 | 0 | 0 | 104 | 0 | 28 | 0 | 28 | 132 |
| 07:30 AM | 0 | 2 | 0 | 2 | 58 | 1 | 0 | 59 | 2 | 24 | 0 | 26 | 87 |
| 07:45 AM | 1 | 6 | 0 | 7 | 73 | 0 | 0 | 73 | 2 | 35 | 0 | 37 | 117 |
| Total Volume | 4 | 9 | 0 | 13 | 301 | 3 | 0 | 304 | 5 | 106 | 0 | 111 | 428 |
| \% App. Total | 30.8 | 69.2 | 0 |  | 99 | 1 | 0 |  | 4.5 | 95.5 | 0 |  |  |
| PHF | . 333 | . 375 | . 000 | . 464 | 724 | . 375 | . 000 | . 731 | . 625 | . 757 | . 000 | . 750 | 811 |

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 05:00 PM

| 05:00 PM | 1 | 0 | 0 | 1 | 79 | 0 | 0 | 79 | 0 | 91 | 0 | 91 | 171 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 05:15 PM | 0 | 1 | 0 | 1 | 60 | 1 | 0 | 61 | 2 | 90 | 0 | 92 | 154 |
| 05:30 PM | 0 | 2 | 0 | 2 | 76 | 3 | 0 | 79 | 1 | 86 | 0 | 87 | 168 |
| 05:45 PM | 0 | 1 | 0 | 1 | 65 | 0 | 0 | 65 | 4 | 69 | 0 | 73 | 139 |
| Total Volume | 1 | 4 | 0 | 5 | 280 | 4 | 0 | 284 | 7 | 336 | 0 | 343 | 632 |
| \% App. Total | 20 | 80 | 0 |  | 98.6 | 1.4 | 0 |  | 2 | 98 | 0 |  |  |
| PHF | . 250 | . 500 | . 000 | . 625 | . 886 | . 333 | . 000 | . 899 | . 438 | . 923 | . 000 | . 932 | . 924 |

## APPENDIX B

## CAPACITY ANALYSIS WORKSHEETS EXISTING

General Information

| Analyst | V3 Co. | Intersection | Bricher Rd and Camden St |
| :--- | :--- | :--- | :--- |
| Agency/Co. | Village of St. Charles | Jurisdiction | St. Charles Township |
| Date Performed | $12 / 8 / 2016$ | East/West Street | Bricher Rd |
| Analysis Year | 2016 | North/South Street | Camden St |
| Time Analyzed | Existing - AM Peak Hour | Peak Hour Factor | 0.95 |
| Intersection Orientation | East-West | Analysis Time Period (hrs) | 0.25 |
| Project Description | Prairie Winds |  |  |

Lanes

Vehicle Volumes and Adjustments


Critical and Follow-up Headways

| Base Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## Delay, Queue Length, and Level of Service



## HCS 2010 Two-Way Stop-Control Report

## General Information

| Analyst | V3 Co. | Intersection | Bricher Rd and Camden St |
| :--- | :--- | :--- | :--- |
| Agency/Co. | Village of St. Charles | Jurisdiction | St. Charles Township |
| Date Performed | $12 / 8 / 2016$ | East/West Street | Bricher Rd |
| Analysis Year | 2016 | North/South Street | Camden St |
| Time Analyzed | Existing - PM Peak Hour | Peak Hour Factor | 0.95 |
| Intersection Orientation | East-West | Analysis Time Period (hrs) | 0.25 |
| Project Description | Prairie Winds |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 1 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  | T | R |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume, V (veh/h) |  |  | 280 | 4 |  | 7 | 336 |  |  | 1 |  | 4 |  |  |  |  |
| Percent Heavy Vehicles (\%) |  |  |  |  |  | 0 |  |  |  | 0 |  | 0 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Type/Storage |  |  |  |  | ded |  |  |  |  |  |  |  |  |  |  |  |
| Critical and Follow-up Headways |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Critical Headway (sec) |  |  |  |  |  | 4.1 |  |  |  | 7.1 |  | 6.2 |  |  |  |  |
| Critical Headway (sec) |  |  |  |  |  | 4.10 |  |  |  | 6.40 |  | 6.20 |  |  |  |  |
| Base Follow-Up Headway (sec) |  |  |  |  |  | 2.2 |  |  |  | 3.5 |  | 3.3 |  |  |  |  |
| Follow-Up Headway (sec) |  |  |  |  |  | 2.20 |  |  |  | 3.50 |  | 3.30 |  |  |  |  |

## Delay, Queue Length, and Level of Service



## APPENDIX C

## CAPACITY ANALYSIS WORKSHEETS BACKGROUND

General Information

| Analyst | V3 Co. | Intersection | Bricher Rd and Camden St |
| :--- | :--- | :--- | :--- |
| Agency/Co. | Village of St. Charles | Jurisdiction | St. Charles Township |
| Date Performed | $12 / 8 / 2016$ | East/West Street | Bricher Rd |
| Analysis Year | 2022 | North/South Street | Camden St |
| Time Analyzed | Background - AM Peak Hour | Peak Hour Factor | 0.95 |
| Intersection Orientation | East-West | Analysis Time Period (hrs) | 0.25 |
| Project Description | Prairie Winds |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 1 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  | T | R |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume, V (veh/h) |  |  | 319 | 8 |  | 13 | 112 |  |  | 10 |  | 24 |  |  |  |  |
| Percent Heavy Vehicles (\%) |  |  |  |  |  | 0 |  |  |  | 0 |  | 0 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) |  |  |  |  |  |  |  |  | 0 |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


General Information

| Analyst | V3 Co. | Intersection | Bricher Rd and Camden St |
| :--- | :--- | :--- | :--- |
| Agency/Co. | Village of St. Charles | Jurisdiction | St. Charles Township |
| Date Performed | $12 / 8 / 2016$ | East/West Street | Bricher Rd |
| Analysis Year | 2022 | North/South Street | Camden St |
| Time Analyzed | Background - PM Peak Hour | Peak Hour Factor | 0.95 |
| Intersection Orientation | East-West | Analysis Time Period (hrs) | 0.25 |
| Project Description | Prairie Winds |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 1 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  | T | R |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume, V (veh/h) |  |  | 297 | 11 |  | 18 | 356 |  |  | 3 |  | 10 |  |  |  |  |
| Percent Heavy Vehicles (\%) |  |  |  |  |  | 0 |  |  |  | 0 |  | 0 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) |  |  |  |  |  |  |  |  | 0 |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## Delay, Queue Length, and Level of Service



## APPENDIX D

## CAPACITY ANALYSIS WORKSHEETS FUTURE WITH PROJECT

## HCS 2010 Two-Way Stop-Control Report

General Information

| Analyst | V3 Co. | Intersection | Bricher Rd and Camden St |
| :--- | :--- | :--- | :--- |
| Agency/Co. | Village of St. Charles | Jurisdiction | St. Charles Township |
| Date Performed | $12 / 8 / 2016$ | East/West Street | Bricher Rd |
| Analysis Year | 2022 | North/South Street | Camden St |
| Time Analyzed | Future w/ Proj. - AM PH | Peak Hour Factor | 0.95 |
| Intersection Orientation | East-West | Analysis Time Period (hrs) | 0.25 |
| Project Description | Prairie Winds |  |  |

Lanes

Vehicle Volumes and Adjustments


Critical and Follow-up Headways

| Base Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


## HCS 2010 Two-Way Stop-Control Report

## General Information

| Analyst | V3 Co. | Intersection | Bricher Rd and West Drive |
| :--- | :--- | :--- | :--- |
| Agency/Co. | Village of St. Charles | Jurisdiction | St. Charles Township |
| Date Performed | $12 / 8 / 2016$ | East/West Street | Bricher Rd |
| Analysis Year | 2022 | North/South Street | West Drive |
| Time Analyzed | Future w/ Proj - AM PH | Peak Hour Factor | 0.95 |
| Intersection Orientation | East-West | Analysis Time Period (hrs) | 0.25 |
| Project Description | Prairie Winds |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | L | T |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume, V (veh/h) |  | 4 | 330 |  |  |  | 132 | 6 |  |  |  |  |  | 26 |  | 15 |
| Percent Heavy Vehicles (\%) |  | 3 |  |  |  |  |  |  |  |  |  |  |  | 0 |  | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) |  |  |  |  |  |  |  |  |  |  |  |  | 0 |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


## HCS 2010 Two-Way Stop-Control Report

General Information

| Analyst | V3 Co. | Intersection | Bricher Rd and Camden St |
| :--- | :--- | :--- | :--- |
| Agency/Co. | Village of St. Charles | Jurisdiction | St. Charles Township |
| Date Performed | $12 / 8 / 2016$ | East/West Street | Bricher Rd |
| Analysis Year | 2022 | North/South Street | Camden St |
| Time Analyzed | Future w/ Proj. - PM PH | Peak Hour Factor | 0.95 |
| Intersection Orientation | East-West | Analysis Time Period (hrs) | 0.25 |
| Project Description | Prairie Winds |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 1 | 1 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 1 | 1 | 0 |
| Configuration |  | L | T | R |  | L | T | R |  |  | LTR |  |  | L |  | TR |
| Volume, V (veh/h) |  | 20 | 305 | 11 |  | 18 | 371 | 41 |  | 3 | 0 | 10 |  | 21 | 0 | 11 |
| Percent Heavy Vehicles (\%) |  | 0 |  |  |  | 0 |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) |  |  |  |  |  |  |  |  | 0 |  |  |  | 0 |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


## HCS 2010 Two-Way Stop-Control Report

General Information

| Analyst | V3 Co. | Intersection | Bricher Rd and West Drive |
| :--- | :--- | :--- | :--- |
| Agency/Co. | Village of St. Charles | Jurisdiction | St. Charles Township |
| Date Performed | $12 / 8 / 2016$ | East/West Street | Bricher Rd |
| Analysis Year | 2022 | North/South Street | West Drive |
| Time Analyzed | Future w/ Proj - PM PH | Peak Hour Factor | 0.95 |
| Intersection Orientation | East-West | Analysis Time Period (hrs) | 0.25 |
| Project Description | Prairie Winds |  |  |

Lanes

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | L | T |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume, V (veh/h) |  | 25 | 328 |  |  |  | 370 | 15 |  |  |  |  |  | 8 |  | 14 |
| Percent Heavy Vehicles (\%) |  | 0 |  |  |  |  |  |  |  |  |  |  |  | 0 |  | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Percent Grade (\%) |  |  |  |  |  |  |  |  |  |  |  |  | 0 |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type/Storage | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Critical and Follow-up Headways

| Base Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Critical Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Follow-Up Headway (sec) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


## APPENDIX E

## TRAFFIC SIGNAL WARRANT ANALYSIS



REVIEW INFORMATION

| COUNTS USED: | V3 Counts |
| :--- | :---: |
| COUNT DATE(S): | $11 / 30 / 2016$ |
| DATA REVIEWED: | $3 / 28 / 2017$ |
| REVIEWED BY: | CAS |

LANE CONFIGURATIONS

## RIGHT TURN FACTORIZATION SHEET (CONT.)

$\xrightarrow[\sim]{4}$
Any configuration with an exclusive right turn lane (usually up to

| LEFT | THROUGH | RIGHT | TOTAL (A) | 0.7A | 0.35A | 3 T | T/3 | (T+L) | (T+R) | 3R | 3L | T/2 | T/4 | REDUCTION |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 51 | 0 | 10 | 61 | 43 | 21 | 0 | 0 | 51 | 10 | 30 | 153 | 0 | 0 | 20\% |
| 21 | 0 | 11 | 32 | 22 | 11 | 0 | 0 | 21 | 11 | 33 | 63 | 0 | 0 | 20\% |
| 12 | 0 | 6 | 18 | 13 | 6 | 0 | 0 | 12 | 6 | 18 | 36 | 0 | 0 | 20\% |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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RIGHT TURN FACTORIZATION SHEET

INTERSECTION: Bricher Rd and Camden St./Primary Driveway MUNCIPALITY: St. Charles, IL

COUNTY: Kane

|  | MINOR STREET |  |  |  |  | CRITICAL | BASE RIGHTTURN | MAINLINE CONGESTION | ADJUSTED RIGHT TURN | ADJUSTED RIGHT | ADJUSTED MINOR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | STREET NAME |  | Camden St/Primary Driveway |  |  |  |  |  |  |  |
| DIR | HOUR BEGIN | CONFIG.\# |  | 1 |  | MAINLINE APPROACH VOLUME PER LANE |  |  |  |  |  |
|  |  | VOLUMES |  |  |  |  |  |  |  |  |  |
|  |  | $\begin{gathered} \mathrm{L} \\ \mathrm{LEFT} \end{gathered}$ | $\begin{gathered} \mathrm{T} \\ \text { THROUGH } \end{gathered}$ | $\begin{gathered} \hline \text { R } \\ \text { RIGHT } \end{gathered}$ | $\frac{\mathrm{A}}{\mathrm{TOTAL}}$ |  | $\begin{gathered} \text { REDUCTION } \\ \% \end{gathered}$ | $\begin{gathered} \text { FACTOR } \\ \% \end{gathered}$ | $\begin{gathered} \text { REDUCTION } \\ \% \end{gathered}$ | TURNS | STREET <br> VOLUME |
|  |  |  |  |  |  |  |  |  |  |  |  |
| SB | 7:00 | 51 | 0 | 10 | 61 | 118 | 20\% | 0\% | 20\% | 8 | 59 |
|  |  |  |  |  |  |  |  |  |  |  |  |
| SB | 5:00 | 21 | 0 | 11 | 32 | 371 | 20\% | 0\% | 20\% | 9 | 30 |
|  |  |  |  |  |  |  |  |  |  |  |  |
| SB | 8th Hr | 12 | 0 | 6 | 18 | 204 | 20\% | 0\% | 20\% | 5 | 17 |
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| MAINLINE CONGESTION FACTORS |  |
| :---: | :---: |
| VOLUMES | FACTOR \% |
| $0-399$ | $0 \%$ |
| $400-499$ | $5 \%$ |
| $500-599$ | $10 \%$ |
| $600-699$ | $15 \%$ |
| $700-799$ | $20 \%$ |
| $800-899$ | $25 \%$ |
| $900-999$ | $30 \%$ |
| $1000-1099$ | $35 \%$ |
| $1100-1199$ | $40 \%$ |
| $1200-1299$ | $45 \%$ |
| $1300-1399$ | $50 \%$ |
| $1400-1499$ | $55 \%$ |

REVIEW INFORMATION
COUNTS USED: V3 Counts
COUNT DATE(S): 11/30/2016
DATE REVIEWE[ 3/28/2017
REVIEWED BY: CAS

## SIGNAL WARRANT REVIEW SHEET



## TRAFFIC SIGNAL WARRANT SUMMARY




Plot four volume combinations on the applicable figure below.

## APPENDIX F

## RIGHT TURN LANE WARRANT ANALYSIS EXHIBITS





