# Prairie Centre Traffic Impact Study 

August 17, 2016

## Prepared for:

## City of St. Charles

## Prepared By:



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## I. Executive Summary

Shodeen Group has proposed redevelopment the former St. Charles Mall site on IL Route 38 just east of Randall Road in St. Charles, Illinois. Shodeen Group has previously proposed a number of redevelopment concepts, none of which has been executed. Hampton, Lenzini and Renwick, Inc. (HLR) provided traffic engineering services to the City of St. Charles on most of those previous concepts, the most recent of which was in the form of a Traffic Impact Study in 2010. At that time, the development was known as the St. Charles Towne Centre.

The current concept is a mixed-use development called the St. Charles Prairie Centre. The Prairie Centre property is a $26 \pm$ acre site located on the block bounded by IL Route 38 (Lincoln Highway), Randall Road, Prairie Street, and $14^{\text {th }}$ Street in the City of St. Charles. See the Appendix for a general location map of the study area, an aerial photo overview and a preliminary plan of the proposed development. The concept plan consists of a mix of restaurant, retail, and residential spaces. The current concept plan is similar to the 2010 concept but includes shorter buildings and fewer units. As currently proposed, the Prairie Centre will utilize and improve existing access locations onto IL Route 38 and Prairie Street. No new access points to the surrounding street system are proposed.

## Existing Traffic Conditions

Peak period turning movement traffic counts were conducted on weekdays from 6:00 - 9:00 AM and from 3:30-6:30 PM and on Saturdays from 11:00 AM - 2:00 PM in May 2016 at the following 16 intersections:

- IL Route 38 \& Randall Road
- IL Route 38 \& Jewel Driveway
- IL Route 38 \& West Mall Entrance
- IL Route 38 \& East Mall Entrance/Vanderbilt Drive
- IL Route 38 \& 14 ${ }^{\text {th }}$ Street/Bricher Road
- Randall Road \& Prairie Street
- Prairie Street \& Jewel Driveway
- Prairie Street \& West Mall Entrance
- Prairie Street \& East Mall Entrance
- Prairie Street \& Covington Court/Wessel Court
- Prairie Street \& $16^{\text {th }}$ Street
- Prairie Street \& $14^{\text {th }}$ Street
- Prairie Street \& $7^{\text {th }}$ Street
- Prairie Street \& 3 ${ }^{\text {rd }}$ Street
- $14^{\text {th }}$ Street $\&$ Vanderbilt Drive
- $14^{\text {th }}$ Street \& Covington Court/Horne Street

Thirteen-hour weekday counts were also collected from 6:00 AM - 7:00 PM at three intersections on Prairie Street: at $14^{\text {th }}$ Street, $7^{\text {th }}$ Street and $3^{\text {rd }}$ Street.

## Traffic Projections

To account for increases in overall traffic growth beyond what the proposed developments will generate, the existing traffic volumes were increased using growth rates provided by the Chicago Metropolitan Agency for Planning (CMAP). A copy of the memo from CMAP regarding these growth rates is included in the Appendix.

## Estimated Site-Generated Traffic

Site-generated traffic was estimated using the ITE's Trip Generation Manual, $9^{\text {th }}$ Edition. Land use assumptions were based on information provided by the developer. The residential units were modeled with ITE Code 220, Apartment. The restaurant units were modeled as a combination of ITE Code 932, High-turnover (Sit-down) Restaurant, and ITE Code 934, Fastfood Restaurant with Drive-through Window. The retail units were modeled with ITE Code 826, Specialty Retail Center, except for the weekday AM peak hour, for which ITE Code 820, Shopping Center, was determined to be more reasonable. Adjustments were made to the generated trip amounts for a combined $20 \%$ internal capture and pass-by reduction applied to the appropriate uses. The table below summarizes the resulting generated traffic volumes.

Total New Trips after all Adjustments in veh/hr

| Peak Hour | In | Out | Total |
| :---: | :---: | :---: | :---: |
| Weekday AM | 377 | 476 | 853 |
| Weekday PM | 493 | 377 | 870 |
| Saturday | 477 | 450 | 927 |

Source: ITE Trip Generation Manual, ${ }^{\text {th }}$ Edition

## Analyses and Recommendations

Capacity analyses were performed for the study area intersections using Synchro and Highway Capacity Software. The analyses were run for the weekday AM and PM peak hours and the Saturday peak. Analyses were performed for the 2016 Existing Traffic, 2026 Base Year Traffic, and the 2026 Total Traffic. Signal warrants were also analyzed for three intersections on Prairie Street. The resulting recommendations are summarized below. Where a study area intersection is not listed below, no improvements are recommended, and no significant issues were discovered.

IL Route 38 \& Randall Road: This intersection and its turn lanes have sufficient capacity to accommodate the projected 2026 Total Site traffic. Individual movements may operate at LOS E or F, though this is not entirely attributed to the Prairie Centre Development as this occurs in the 2026 Base Traffic scenario also. The proposed future Kane County improvement to add through-traffic lanes to Randall Road will further enhance the capacity of this intersection. No improvements to this intersection are recommended as a result of the proposed development.

IL Route 38 \& West Mall Entrance: The site plans show conversion of the southbound approach to consist of an exclusive left turn lane, an exclusive through lane, and an exclusive right turn lane. Additionally, the northbound and southbound left turn phases should be converted to protected-permissive phasing.

IL Route 38 \& $14^{\text {th }}$ Street: This intersection has the overall capacity to accommodate the projected 2026 Total Traffic. Consideration should be made for adding a northbound right turn lane to reduce existing delays and queues on that approach. No improvements are recommended in connection with the development.

Randall Road \& Prairie Street: This intersection has the overall capacity to accommodate the projected 2026 Total Traffic. However, there are existing issues with the westbound left turn queues and with the interaction between this intersection and the Randall Road and IL

Route 38 intersection. Reconfiguration of the westbound approach and the driveways on that leg could help address this problem. Further, the planned widening of Randall Road will help alleviate these issues. However, since these are existing issues, no specific improvements are recommended as part of this development.

Prairie Street \& 14 th Street: Under 2026 traffic, the northbound left will enter LOS F range during the PM peak hour. However, this intersection has the overall capacity to accommodate the 2026 Total Traffic volumes, with queues reaching four vehicles at most. Although traffic signal warrants are met under the existing traffic at the intersection, installation of traffic signal control is not recommended at this time.

Prairie Street \& $7^{\text {th }}$ Street: Base Year average delays for this intersection reach LOS F during the PM peak hour. Traffic signal control is warranted under existing traffic. A number of improvements could alleviate delays, including signalization or conversion to a roundabout. No improvements are recommended as a part of the development, as the issues here are primarily existing.

Prairie Street \& 3 ${ }^{\text {rd }}$ Street: Base Year average delays for this intersection reach LOS F during the PM peak hour. Traffic signal control is warranted under existing traffic. A number of improvements could alleviate delays, including signalization or conversion to a roundabout. No improvements are recommended as a part of the development, as the issues here are primarily existing.

In summary, few improvements are recommended with this development. The analyses revealed existing concerns within the study area, particularly along Randall Road and along the east end of Prairie Street. While the Prairie Centre development related traffic is expected to contribute to these concerns, the analyses show that the problems will not be avoided by preventing the development.

Kane County has plans to widen Randall Road through the study area limits, which is expected to address the vast majority of issues at the Randall Road intersections. The Prairie Street intersections at $14^{\text {th }}$ Street, $7^{\text {th }}$ Street, and $3^{\text {rd }}$ Street could be improved with signalization, conversion to modern roundabouts, or other possible solutions, as determined by the City of St Charles.

## II. Introduction

Shodeen Group has proposed redevelopment of their property on IL Route 38 just east of Randall Road in St. Charles, Illinois. The St. Charles Mall previously occupied this property before closing in 1996 and later being demolished. Since then, Shodeen Group has proposed a number of redevelopment concepts, none of which has been executed. Hampton, Lenzini and Renwick, Inc. (HLR) provided traffic engineering services to the City of St. Charles on most of those previous concepts, the most recent of which was in the form of a Traffic Impact Study in 2010. At that time, the development was known as the St. Charles Towne Centre.

The current concept is a mixed-use development called the St. Charles Prairie Centre. The Prairie Centre property is a $26 \pm$ acre site located on the block bounded by IL Route 38 (Lincoln Highway), Randall Road, Prairie Street, and $14^{\text {th }}$ Street in the City of St. Charles. See Exhibit 1 A at the end of the report for a general location map of the study area. Exhibit 1B displays an aerial photo overview of the existing development parcel and adjoining land uses. A preliminary plan of the proposed development is included as Exhibit 2.

The Prairie Centre concept plan consists of a mix of restaurant, retail, and residential spaces. The current concept plan is similar to the 2010 concept but includes shorter buildings and fewer units. As currently proposed, the Prairie Centre will utilize and improve existing access locations onto IL Route 38 and Prairie Street. No new access points to the surrounding street system are proposed.

## III. Existing Conditions

A field reconnaissance of the site was conducted to inventory information on surrounding land uses and the area roadway network. In addition, traffic counts were conducted during the morning, evening and Saturday peak periods at 16 critical intersections.

## Surrounding Land Uses

Land uses surrounding the site are predominantly residential to the north and east and predominantly commercial to the south and west. A Jewel supermarket adjoins the west edge of the site, while a series of retail stores adjoin the southeast edge.

## Surrounding Roadway Network

The major roadways servicing the study area are Randall Road and IL Route 38. Prairie Street is a collector route that also provides access to this area. $14^{\text {th }}$ Street is a minor collector street to which the site will also have indirect access. A brief description of these roadways is provided below:

- Illinois Route 38, along the frontage of the proposed site, is a four-lane, two-way, eastwest major arterial roadway. There is a flush median along the frontage of the site with pavement striping providing left turn lanes at both existing full-access entrances into the site. The western entrance is controlled by traffic signals. The eastern entrance is controlled by a stop sign on the entrance approach. Approaching Randall Road, the IL Route 38 median is raised and widens to provide dual left turn lanes at the Randall Road intersection. IL Route 38 is under the jurisdiction of IDOT and is posted with a 45 miles-
per-hour (mph) speed limit. It has not been designated as a Strategic Regional Arterial (SRA) by IDOT.
- Randall Road is a major north-south arterial roadway. Through this area, Randall Road provides two through lanes in each direction with left turn lanes at intersections. Randall Road is under the jurisdiction of the Kane County Division of Transportation (KDOT) and has been designated as an SRA. Randall Road is posted with a 45 mph speed limit. The Randall Road intersections with IL Route 38 and with Prairie Street are controlled by traffic signals, which are interconnected with signalized intersections to the north and south along Randall Road, as well as to the east and west along IL Route 38.
- Prairie Street, along the frontage of the proposed site, is a three-lane, two-way collector street. Prairie Street extends from Randall Road east over the Fox River to Riverside Avenue. Its intersection with Randall Road is controlled by traffic signals. Its intersections with $7^{\text {th }}$ Street and with $3^{\text {rd }}$ Street are controlled by all-way stop signs. Prairie Street is under the jurisdiction of the City of St. Charles and is posted with a 30 mph speed limit ( 25 mph east of $3^{\text {rd }}$ Street).
- $14^{\text {th }}$ Street is a two-lane, two-way, north-south collector street that extends from IL Route 38 north to Prairie Street. $14^{\text {th }}$ Street is under the jurisdiction of the City of St. Charles and is posted with a 25 mph speed limit.

Two other nearby streets of interest include $16^{\text {th }}$ Street and Horne Street. $16^{\text {th }}$ Street is a local street that extends north from Prairie Street and provides a connection to the neighborhood north of Prairie Street and east of Randall Road. Horne Street is a collector street that extends from IL Route 31 west to $14^{\text {th }}$ Street. The westernmost three blocks of this route consist of several turns before Horne Street intersects $14^{\text {th }}$ Street.

## Existing Traffic Conditions

Peak period turning movement traffic counts were conducted on weekdays from 6:00 - 9:00 AM and from 3:30-6:30 PM and on Saturdays from 11:00 AM - 2:00 PM in May 2016 at the following 16 intersections:

- IL Route 38 \& Randall Road
- IL Route 38 \& Jewel Driveway
- IL Route 38 \& West Mall Entrance
- IL Route 38 \& East Mall Entrance/Vanderbilt Drive
- IL Route 38 \& 14 $4^{\text {th }}$ Street/Bricher Road
- Randall Road \& Prairie Street
- Prairie Street \& Jewel Driveway
- Prairie Street \& West Mall Entrance
- Prairie Street \& East Mall Entrance
- Prairie Street \& Covington Court/Wessel Court
- Prairie Street \& $16^{\text {th }}$ Street
- Prairie Street \& $14^{\text {th }}$ Street
- Prairie Street \& $7^{\text {th }}$ Street
- Prairie Street \& $3^{\text {rd }}$ Street
- $14^{\text {th }}$ Street \& Vanderbilt Drive
- $14^{\text {th }}$ Street \& Covington Court/Horne Street

Exhibit 3 in Appendix A presents the existing peak hour volumes at these intersections. Thirteen-hour weekday counts were also collected from 6:00 AM - 7:00 PM at three intersections on Prairie Street: at $14^{\text {th }}$ Street, $7^{\text {th }}$ Street and $3^{\text {rd }}$ Street. It should be noted that the St. Charles Mall was demolished in 2002, well before the traffic counts were performed.

## Traffic Projections

To account for increases in overall traffic growth beyond what the proposed developments will generate, the existing traffic volumes depicted on Exhibit 3 were increased at a rate of 1.04 percent per year on IL Route 38, 0.63 percent per year on Randall Road, 2.56 percent per year on Prairie Street, and 1.17 percent per year on 14th Street for 12 years. These growth rates were provided by the Chicago Metropolitan Agency for Planning (CMAP). A copy of the memo from CMAP regarding these growth rates is included in Appendix J. These increased traffic volumes were used to develop the 2026 Base Year Traffic.

Traffic generated by the new CVS Pharmacy and its outlots was added to the 2026 Base Year Traffic. This development was under construction at the time this study was executed. Table 1 below shows the traffic volumes added to the 2026 Base Year Traffic volumes. The traffic generated by this site was estimated using the Institute of Transportation Engineers' (ITE) Trip Generation Manual, $9^{\text {th }}$ Edition. The ITE numbers referenced in Table 1 refer to the land use codes from the Trip Generation Manual. The final 2026 Base Year Traffic (no development) volumes can be seen in Exhibit 4.

Table 1
2026 Base Traffic Adjustments

| Land Use |  | Weekday AM |  | Weekday PM |  | Saturday |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Out | In | Out | In | Out |  |
| CVS Pharmacy (ITE 934) | 22 | 20 | 59 | 59 | 48 | 49 |  |
| Outlot (ITE 826) | 13 | 15 | 14 | 16 | 1 | 1 |  |
| Total Adjustment | 35 | 35 | 73 | 75 | 49 | 50 |  |

Source: ITE Trip Generation Manual, $9^{\text {th }}$ Edition
Capacity Analysis - Existing Traffic and 2026 Base Traffic
Capacity analyses of existing and 2026 Base Year (estimated short-term, no development) weekday AM and PM and Saturday mid-day peak hour traffic conditions were conducted at the aforementioned intersections.

Level of Service (LOS) criteria for signalized and stop sign controlled intersections are based on the methodologies presented in the Highway Capacity Manual(HCM) published by the Transportation Research Board (TRB). LOS criteria range from "A" (good) to "F" (poor) and are based on average delay. It should be noted that the LOS thresholds are different for signalized and stop sign controlled intersections. At two-way stop intersections, LOS criteria for stop sign controlled intersections are defined for each minor movement and are not defined for the intersection as a whole. The LOS delay thresholds for stop sign controlled intersections are also lower than for signalized intersections since driver expectation at a signalized intersection is for a greater delay. The LOS criteria for signalized and stop sign controlled intersections are presented in Table 2.

Table 2
Level of Service Criteria for Signalized and Stop Sign Controlled Intersections
Signalized Intersections

| Level of <br> Service | Type of Operating Condition | Average Vehicle <br> Delay (seconds) |
| :---: | :--- | :---: |
| A | Very low delay, most vehicles arrive during the green and <br> do not stop at all. | $\leq 10.0$ |
| B | More vehicles stop at the traffic signal than LOS "A", but <br> otherwise good progression of traffic through the <br> intersection. | $10.1-20.0$ |
| C | Congestion starts to occur; number of vehicles stopping at <br> the intersection is significant. | $20.1-35.0$ |
| D | Congestion is more noticeable, longer delays; some <br> vehicles may not clear on a single cycle. | $35.1-55.0$ |
| E | High delays, poor progression through intersection. Most <br> vehicles do not clear the intersection on a single cycle. | $55.1-80.0$ |
| F | Unacceptable high delay to drivers, demand exceeds <br> capacity, increasing queue lengths. | $>80.0$ |

Stop Sign Controlled Intersections

| Level of Service | Average Control Delay (sec/veh.) |
| :---: | :---: |
| A | $0-10$ |
| B | $>10-15$ |
| C | $>15-25$ |
| D | $>25-35$ |
| E | $>35-50$ |
| F | $>50$ |

Source: Highway Capacity Manual 2000, Transportation Research Board, National Research Council, Washington, D.C.

Table 3 presents the existing and 2026 Base Year (projected short-term) traffic operations at the signalized intersections studied. Analysis of 2016 existing traffic was conducted using existing signal controller settings and existing intersection geometry. Analysis of 2026 Base Year Traffic retained existing intersection geometry but assumed that the traffic signal timings would be reoptimized. Copies of the capacity analysis summaries conducted for the existing critical intersections are included in Appendices E and F.

Table 3
Summary of Existing and Base Year (no development) Traffic Conditions Intersection Level of Service (LOS) and Delay (seconds)

Signalized Intersections

| Intersection | Existing 2016 Traffic |  |  | Base Year 2026 Traffic (no site) |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AM Peak | PM Peak | Sat Peak | AM Peak | PM Peak | Sat Peak |
| IL 38 \& Randall Rd | D (39) | D (43) | D (43) | D (40) | D (49) | D (45) |
| IL 38 \& | A (6) | A (6) | A (5) | A (6) | A (8) | A (7) |
| West Mall Entrance | C (28) | C (32) | C (34) | C (24) | D (37) | C (31) |
| IL 38 \& 14 St St | B (11) | C (23) | C (28) | B (12) | D (42) | D (44) |
| Prairie St \& Randall Rd | B |  |  |  |  |  |

It should be noted that some individual movements operate at LOS E or F. Table 4 gives a detailed breakdown of the 2026 Base Year Traffic, showing each individual movement's Level of Service.

Table 4
LOS \& Delay by Movement for 2026 Base Traffic

| Intersection | Peak Hour | Overall LOS \& (delay) | LOS \& (delay) by Movement |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Eastbound |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
|  |  |  | L | T $\quad$ R | L | T | R | L | T | R | L | T | R |
|  <br> Randall Rd | AM | D (40) | $\begin{gathered} \hline \mathrm{E} \\ (72) \\ \hline \end{gathered}$ | D A <br> $(54)$ $(0)$ | $\begin{gathered} \hline \mathrm{E} \\ (67) \end{gathered}$ | $\begin{gathered} \hline \text { D } \\ (53) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{B} \\ (15) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{E} \\ (74) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{D} \\ (38) \end{gathered}$ | A <br> (0) | $\begin{gathered} \hline \text { F } \\ (81) \end{gathered}$ | $\begin{gathered} C \\ (22) \\ \hline \end{gathered}$ | A <br> (3) |
|  | PM | D (49) | $\begin{gathered} F \\ (84) \end{gathered}$ | D A <br> $(49)$ $(6)$ | $\begin{gathered} \mathrm{E} \\ (74) \end{gathered}$ | $\begin{gathered} \text { E } \\ (73) \end{gathered}$ | $\begin{gathered} \text { D } \\ (38) \end{gathered}$ | $\begin{gathered} \mathrm{E} \\ (77) \end{gathered}$ | $\begin{gathered} E \\ (57) \end{gathered}$ | $\begin{gathered} \hline \text { A } \\ (0) \end{gathered}$ | $\begin{gathered} \text { F } \\ (87) \\ \hline \end{gathered}$ | $\begin{gathered} \text { C } \\ (28) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{A} \\ (7) \end{gathered}$ |
|  | Sat | D (45) | $\begin{gathered} \text { F } \\ (87) \\ \hline \end{gathered}$ | $E$ $A$ <br> $(61)$ $(9)$ | $\begin{gathered} \text { F } \\ (87) \\ \hline \end{gathered}$ | $\begin{gathered} \text { F } \\ (87) \\ \hline \end{gathered}$ | $\begin{gathered} \text { D } \\ \text { (32) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { E } \\ \text { (63) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { D } \\ (40) \\ \hline \end{gathered}$ | $\begin{gathered} \text { A } \\ (1) \\ \hline \end{gathered}$ | $\begin{gathered} E \\ \text { E } \\ \hline \end{gathered}$ | $\begin{gathered} \text { C } \\ (26) \\ \hline \end{gathered}$ | $\begin{gathered} \text { A } \\ (2) \\ \hline \end{gathered}$ |
| IL 38 \& W Mall Entrance | AM | A (6) | A <br> (2) | A (6) | A (2) | A (2) |  | $\begin{gathered} \mathrm{E} \\ (57) \end{gathered}$ | B (17) |  | $\begin{gathered} \hline \mathrm{D} \\ \hline(49) \end{gathered}$ | $\begin{gathered} \hline D \\ (48) \end{gathered}$ | A <br> (0) |
|  | PM | A (8) | $\begin{gathered} \text { A } \\ \text { (3) } \\ \hline \end{gathered}$ | A (6) | A (1) | A (5) |  | $\begin{gathered} \text { E } \\ \text { (58) } \\ \hline \end{gathered}$ | B (15) |  | $\begin{gathered} \text { D } \\ (51) \\ \hline \end{gathered}$ | $\begin{gathered} \text { D } \\ (46) \\ \hline \end{gathered}$ | $\begin{gathered} \text { A } \\ (0) \\ \hline \end{gathered}$ |
|  | Sat | A (7) | $\begin{gathered} \text { A } \\ \text { (3) } \\ \hline \end{gathered}$ | A (7) | A (1) | A (3) |  | $\begin{array}{\|c} \hline \text { D } \\ (49) \\ \hline \end{array}$ | B (13) |  | $\begin{gathered} \text { D } \\ (43) \\ \hline \end{gathered}$ | $\begin{gathered} \text { D } \\ (40) \\ \hline \end{gathered}$ | $\begin{gathered} \text { A } \\ (0) \\ \hline \end{gathered}$ |
| $\begin{aligned} & \text { IL } 38 \& \\ & 14^{\text {th }} \mathrm{St} \end{aligned}$ | AM | C (24) | $\begin{gathered} \hline \mathrm{B} \\ (11) \\ \hline \end{gathered}$ | B (19) | $\begin{gathered} \hline \mathrm{B} \\ (14) \\ \hline \end{gathered}$ | B (19) |  | $\begin{gathered} \hline \mathrm{C} \\ (25) \end{gathered}$ | D (43) |  | $\begin{gathered} C \\ \text { (33) } \end{gathered}$ | D (35) |  |
|  | PM | D (37) | $\begin{gathered} \text { C } \\ (27) \end{gathered}$ | C (38) | $\begin{gathered} \text { C } \\ (23) \end{gathered}$ | C (34) |  | $\begin{gathered} C \\ (24) \\ \hline \end{gathered}$ | E (56) |  | $\begin{gathered} C \\ \text { C } \\ \hline \end{gathered}$ | D (40) |  |
|  | Sat | C (31) | $\begin{gathered} B \\ \text { B } \\ \hline \end{gathered}$ | C (31) | $\begin{gathered} \hline \text { D } \\ (37) \\ \hline \end{gathered}$ | C (28) |  | $\begin{gathered} \mathrm{B} \\ (18) \\ \hline \end{gathered}$ | D (39) |  | $\begin{gathered} \text { C } \\ (24) \\ \hline \end{gathered}$ | C (30) |  |
| Prairie St \& Randall Rd | AM | B (12) | $\begin{gathered} \hline \mathrm{D} \\ (45) \end{gathered}$ | D (36) | $\begin{gathered} \hline E \\ (68) \\ \hline \end{gathered}$ | $\begin{gathered} \hline D \\ (51) \end{gathered}$ | $\begin{gathered} \text { A } \\ (10) \end{gathered}$ | $\begin{gathered} \hline \text { A } \\ (2) \\ \hline \end{gathered}$ | A (5) |  | $\begin{gathered} \hline \text { B } \\ (15) \end{gathered}$ | B (12) |  |
|  | PM | D (42) | $\begin{gathered} \text { D } \\ \text { (50) } \\ \hline \end{gathered}$ | D (37) | $\begin{gathered} F \\ (292) \\ \hline \end{gathered}$ | $\begin{gathered} E \\ (61) \end{gathered}$ | $\begin{gathered} c \\ C \\ \text { (33) } \end{gathered}$ | $\begin{gathered} \hline \mathrm{B} \\ (11) \end{gathered}$ | C (30) |  | $\begin{gathered} \text { D } \\ (43) \\ \hline \end{gathered}$ | B (19) |  |
|  | Sat | D (44) | $\begin{gathered} \text { D } \\ (41) \end{gathered}$ | D (53) | $\begin{aligned} & \frac{\mathrm{F}}{\mathrm{~F}} \\ & (103) \end{aligned}$ | $\begin{gathered} \text { D) } \\ \hline \text { (53) } \end{gathered}$ | $\begin{gathered} \text { D } \\ \text { (38) } \end{gathered}$ | $\begin{gathered} c \\ \hline \text { B } \\ (16) \end{gathered}$ | D (47) |  | $\begin{gathered} E \\ E \\ (75) \end{gathered}$ | C (32) |  |

Analysis results show the following:
IL Route 38 and Randall Road: Under existing conditions and signal timings, this intersection operates at an over-all Level of Service D, although some individual movements currently operate at LOS E or F during peak times. With background traffic growth projected to 2026, and signal timings re-optimized, there will be both slight increases and slight decreases in delay depending on the particular movement and peak hour. Some vehicle queues do exceed turn lane storage lengths in both analyses, including some through-lane queues that extend beyond the turn lane tapers, periodically blocking entry by turning vehicles until through traffic moves up. Traffic volumes currently approach the capacity of the intersection and are expected to remain so under 2026 Base Year Traffic conditions.

IL Route 38 and the West Mall Entrance: Analyses of existing traffic using existing signal timing settings show minimal delays to IL Route 38 traffic through this intersection. Drivers on the shopping center driveway approaches experience delays in the range of LOS D. Currently, IL Route 38 traffic is assigned about $75 \%$ of the available green time, which is an appropriate balance based on traffic volumes and the need to move traffic on the arterial route. Traffic volumes currently do not exceed the capacity of the intersection and are not expected to do so under 2026 Base Year Traffic conditions.

IL Route 38 and $14^{\text {th }}$ Street: Traffic on IL Route 38 currently operates at an acceptable LOS C with small increases to delay in the projected 2026 base year. Vehicle queues do not exceed turn lane storage lengths in either analysis, although through-lane queues will extend beyond the turn lane tapers, periodically blocking entry by turning vehicles until through traffic moves up. As the volume of through traffic increases on IL Route 38, delays for all left turns and cross street movements will increase. However, analyses of existing and the 2026 base year show that traffic will not exceed the capacity of the intersection. Note that there are also lengthy delays on the northbound approach. Addition of a northbound right turn lane could reduce these delays. However, as mitigation for an existing issue, this improvement is not recommended as part of this study.

Prairie Street and Randall Road: Analysis of existing traffic shows that the overall Level of Service at this intersection is B to C. However, during PM and Saturday peak periods, the queue of westbound left turning traffic significantly overflows the westbound left turn lane. This movement operates at LOS E or F during these times. Green time for Prairie Street is limited by the need to provide adequate time for the large volume of traffic flowing on Randall Road and to keep these movements coordinated with the nearby IL Route 38/Randall Road intersection to the south. The existing peak period overflow of the westbound left turn lane will frequently block the Jewel Supermarket driveway, as well as the entrance to the retail strip mall on the south side of the street west of the Jewel store. These entrances could be converted to allow only right turns to reduce potential conflicts for left turning driveway traffic with the westbound queues from Randall Road. The retail development on the north side of Prairie Street has access to another Prairie Street driveway 200 feet to the east, and the Jewel supermarket has access to another Prairie Street driveway about 500 feet to the east. Additionally, the full access entry to the Jewel site could be relocated to align with the drive aisle at the front of the Jewel store to maintain Jewel's current access. Cross access from the Jewel site would need to be provided to the retail strip mall in the southeast corner of Randall Road and Prairie Street, since that site's full access on Prairie Street is currently its only access. This driveway conversion would enable the Prairie Street westbound left turn

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lane at Randall Road to be lengthened, allowing traffic to clear more efficiently during the westbound left turn arrow. In order to reinforce the right-turn-only operation of these driveways and prevent illegal left turns, the flush median on Prairie Street would need to be converted to a raised median from Randall Road at least along the length of the extended westbound left turn lane and preferably to the relocated full access Jewel driveway.

In the 2026 PM and Saturday peak hours, northbound through queues are expected to spill back into the Randall Road/IL Route 38 intersection. The best way to alleviate this concern is to construct the planned widening of Randall Road to six lanes, which is discussed in detail later in this report.

Table 5 shows a summary of analysis results for stop sign controlled intersections. Capacity analyses of stop sign controlled intersections provide Levels of Service and delays for individual intersection movements, but not the intersection as a whole. Results for the most critical movement at each intersection are shown in the table below.

Table 5
Summary of Existing and Base Year (no development) Traffic Conditions Level of Service (LOS) and Delay (seconds)

Stop Sign Controlled Intersections

| Critical Movement | Existing 2016 Traffic |  |  | Base Year 2026 Traffic (no site) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AM Peak | PM Peak | Sat Peak | AM Peak | PM Peak | Sat Peak |
| East Mall Entrance at IL 38 | $\begin{gathered} \hline \text { SB } \\ D(25) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { SB } \\ \mathrm{D}(28) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathrm{NB} \\ \mathrm{E}(45) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { NB } \\ C(23) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { SB } \\ F(62) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { NB } \\ F(85) \end{gathered}$ |
| Prairie St at West Mall Ent. | $\begin{gathered} \mathrm{NB} \\ \mathrm{~A}(9) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{~B}(12) \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{~B}(10) \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{~B}(11) \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{~B}(13) \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{~B}(11) \end{gathered}$ |
| Prairie St at East Mall Entrance | $\begin{gathered} \mathrm{NBL} \\ \mathrm{~B}(13) \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{C}(15) \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{~B}(13) \end{gathered}$ | $\begin{gathered} \hline \text { NBL } \\ \text { B (15) } \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{C}(19) \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{C}(16) \end{gathered}$ |
| $\begin{aligned} & \text { Prairie St at } \\ & 16^{\text {th }} \mathrm{St} \\ & \hline \end{aligned}$ | $\begin{gathered} \text { SB } \\ \text { B (12) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { SB } \\ \text { B (15) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { SB } \\ \mathrm{B}(13) \\ \hline \end{gathered}$ | $\begin{gathered} \text { SB } \\ \text { B (14) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { SB } \\ \text { C (19) } \end{gathered}$ | $\begin{gathered} \text { SB } \\ C(17) \end{gathered}$ |
| Prairie St at $14^{\text {th }} \mathrm{St}$ | $\begin{gathered} \hline \text { NBL } \\ \mathrm{C}(16) \end{gathered}$ | $\begin{gathered} \text { NBL } \\ \text { D (31) } \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{C}(19) \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{C}(20) \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{~F}(73) \end{gathered}$ | $\begin{gathered} \hline \mathrm{NBL} \\ \mathrm{D}(33) \end{gathered}$ |
| $\begin{aligned} & \text { Prairie St at } \\ & 7^{\text {th }} \mathrm{St} \\ & \hline \end{aligned}$ | $\begin{gathered} E B \\ D(26) \\ \hline \end{gathered}$ | $\begin{gathered} \text { WB } \\ \mathrm{D}(27) \end{gathered}$ | $\begin{gathered} \text { EB } \\ \mathrm{B}(13) \\ \hline \end{gathered}$ | $\begin{gathered} E B \\ E(39) \\ \hline \end{gathered}$ | $\begin{gathered} \text { WB } \\ F(82) \end{gathered}$ | $\begin{gathered} \text { EB } \\ C(16) \\ \hline \end{gathered}$ |
| $\begin{aligned} & \text { Prairie St at } \\ & 3^{\text {rd }} \mathrm{St} \end{aligned}$ | $\begin{gathered} \text { EB } \\ \mathrm{D}(27) \\ \hline \end{gathered}$ | $\begin{gathered} \text { WB } \\ \text { E (36) } \end{gathered}$ | $\begin{gathered} \text { WB } \\ \text { B (14) } \end{gathered}$ | $\begin{gathered} \text { EB } \\ \text { E (50) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { WB } \\ F(197) \end{gathered}$ | $\begin{gathered} \text { WB } \\ C(21) \end{gathered}$ |
| $14^{\text {th }}$ St at Covington Ct | $\begin{gathered} \text { WB } \\ \text { B (12) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { WB } \\ \text { C (17) } \end{gathered}$ | $\begin{gathered} \hline \text { WB } \\ \text { B (12) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { WB } \\ B(13) \\ \hline \end{gathered}$ | $\begin{gathered} \text { WB } \\ C(19) \end{gathered}$ | $\begin{gathered} \text { WB } \\ B(14) \end{gathered}$ |
| $14^{\text {th }}$ St at Vanderbilt Dr | $\begin{gathered} \text { EB } \\ B(11) \\ \hline \hline \end{gathered}$ | $\begin{gathered} \text { EB } \\ \mathrm{B}(12) \end{gathered}$ | $\begin{gathered} \text { EB } \\ \text { B (12) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { EB } \\ \text { B (12) } \\ \hline \end{gathered}$ | $\begin{gathered} \hline E B \\ B(15) \\ \hline \hline \end{gathered}$ | $\begin{gathered} \hline \text { EB } \\ B(14) \\ \hline \hline \end{gathered}$ |

Analysis of existing conditions and 2026 Base Year (no development) Traffic shows that the critical movements at the majority of the stop-controlled intersections included in the analysis operate at acceptable LOS D. There are some exceptions described below, which operate below LOS D.

East Mall Entrance at IL Route 38: The northbound approach during the Saturday peak hour currently has an LOS E. Both northbound and southbound are projected to have LOS F in

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2026, with delays reaching 85 seconds for northbound traffic in the Saturday peak. The delays to the Entrance can be attributed to the large volume of east- and westbound through traffic conflicting with the left turning traffic from the Entrance. The expected 95\% queue approaches 70 feet (almost three vehicles) in 2026.

Prairie Street at 14 ${ }^{\text {th }}$ Street: The northbound left turn movement during the 2026 Base Year PM peak hour is expected to reach LOS F, with delays up to 73 seconds. This delay can be attributed to the northbound left turning traffic having to wait for a sufficient gap in the eastand westbound through traffic. This approach is not expected to have any queuing problems, as the expected $95 \%$ queue is only 75 feet (three vehicles) which is well within the provided storage length. Although traffic signal warrants are met under the existing traffic at the intersection, installation of traffic signal control is not recommended at this time. The intersection currently operates satisfactorily under stop sign control. Further, signalization of the intersection could contribute to or create operational problems at Prairie Street and $16^{\text {th }}$ Street, likely due to the resulting eastbound queues. Conversely, it could create gaps for turning traffic to enter Prairie Street by creating platoons and gaps of vehicles on Prairie Street. It is therefore recommended that there be no immediate change made to the intersection traffic control or geometry and that the intersection be monitored periodically as the site is developed, for example after each development phase, to determine the need for any changes in traffic control or geometry. If traffic control changes are to be made in the future, careful attention should be paid to the impacts on the adjacent $16{ }^{\text {th }}$ Street intersection.

Prairie Street at $7^{\text {th }}$ Street: The eastbound and westbound movements are expected to experience LOS E or F during the weekday peaks in 2026, with the highest delay at 82 seconds. This delay can be attributed primarily to the large volume of traffic using Prairie Street during these peak periods. Despite these delays, the analysis results show the intersection approaching capacity in this scenario.

Prairie Street at $3^{\text {rd }}$ Street: The westbound movement during the PM peak hour has an LOS E. In the base year 2026 scenario, that LOS drops to $F$, while the eastbound approach drops to $E$ in the AM peak. In the 2026 PM peak, the eastbound and southbound movements also drop to LOS F. Delays up to 197 seconds are anticipated. This delay can be attributed primarily to the large volume of traffic using this intersection. The analysis results show the intersection approaching capacity. Even with some capacity at this intersection, attention should be paid to the interaction of traffic here with the intersection of IL Route 31 and Prairie Street. Based on the data, this will be of primary concern for westbound traffic during the PM peak. Queues at this intersection are likely to back into the IL Route 31 intersection. Also, eastbound queues from the IL Route 31 intersection could back into this intersection.

## IV. Site Traffic Characteristics of Proposed Development

## Proposed Land Uses

The concept plan for the proposed development includes the following uses:

- Residential
- Restaurant
- Retail

The current site plan has a number of restaurants along the IL Route 38 frontage, with some sitdown type restaurants and some fast food. The remainder of the site consists of 16 four-story
buildings. The central ones are planned to be residential consisting of apartments. Those closer to IL Route 38 and to Prairie Street will have apartments on the upper floors, while the ground floor will be retail.

The proposed site will also include the necessary access lanes, parking lots, and landscaped areas required by City ordinances. Access to the site is proposed via the following locations:

- IL Route 38 \& Jewel right-turns-only driveway (existing, near west edge of site)
- IL Route 38 \& West Mall Entrance (existing)
- IL Route 38 \& East Mall Entrance (existing, west of JiffyLube)
- Prairie Street \& West Mall Entrance (existing, behind Jewel)
- Prairie Street \& East Mall Entrance (existing)
- $14^{\text {th }}$ Street \& Vanderbilt Drive (existing indirect access, south of Binny's)

The locations of proposed points of access can be seen on the aerial photo in Exhibit 1B and on the concept plan in Exhibit 2. For traffic projection purposes, it is assumed that traffic signal control will remain at the IL Route 38/West Mall Entrance intersection. All other entrances to the proposed site will be controlled by stop signs on the minor street approaches.

## Estimated Site-Generated Traffic

Site-generated traffic was estimated using the ITE's Trip Generation Manual, g'h Edition. The residential units were modeled with ITE Code 220, Apartment. The restaurant units were modeled as a combination of ITE Code 932, High-turnover (Sit-down) Restaurant, and ITE Code 934, Fast-food Restaurant with Drive-through Window. The retail units were modeled with ITE Code 826, Specialty Retail Center, which was determined to be the most appropriate use based on the information provided by the developer. However, there is no trip generation data for the weekday AM peak hour of adjacent street traffic for this use. The trip generation based on the weekday AM peak hour of the generator resulted in volumes that were much higher than seemed reasonable. Therefore, for the weekday AM peak hour trip generation estimates, the retail units were based on ITE Code 820, Shopping Center, weekday AM peak hour of adjacent street traffic. With the final site occupants unknown, it was determined that attempting further detail in land uses would be inappropriate at this time. The proposed land uses and sizes provided by the developer were used to estimate morning, evening, and Saturday peak hour trips to and from the site. The resulting generated traffic is shown in Table 6 below. Also refer to Appendix M for a more detailed breakdown.

Table 6
Trip Generation Table

| Land Use | $\begin{aligned} & \text { ITE } \\ & \text { Code } \end{aligned}$ | Units | Qty | AM Peak Hour Volumes (veh/hr) |  |  | PM Peak Hour Volumes (veh/hr) |  |  | SAT Peak Hour Volumes (veh/hr) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | In | Out | Total | In | Out | Total | In | Out | Total |
| Residential | 220 | DU | 609 | 60 | 242 | 302 | 229 | 124 | 353 | 159 | 158 | 317 |
| Retail | 826 | 1000 SF | 83 | 86 | 53 | 139 | 97 | 124 | 221 | 18 | 17 | 35 |
| Sit-down Restaurants | 932 | 1000 SF | 26 | 156 | 127 | 283 | 155 | 103 | 258 | 196 | 173 | 369 |
| Fast-food | 934 | 1000 SF | 7 | 161 | 155 | 316 | 118 | 109 | 227 | 209 | 201 | 410 |

Source: ITE Trip Generation Manual, g $^{\text {th }}$ Edition
These projected trips have been adjusted for "internal capture" where there is potential for interaction among various uses within the multi-use site, i.e. between residential and restaurant
or between residential and retail. These trips occur entirely within the site and account for a portion of the trips generated by each land use. The capture rate of $15 \%$ percent was calculated using the ITE Internal Capture worksheet provided in Chapter 7 of the ITE Trip Generation Handbook. ITE internal capture estimation methods only pertain to PM peak hour scenarios. In order to determine the most accurate rate for AM and Saturday peaks, an internal capture field study would be required. Without conducting an internal capture field study, it is reasonable to apply the PM peak hour rate to all scenarios. A discussion of multi-use developments and internal capture from the ITE Trip Generation Handbook is included in Appendix N to this report for further reference.

The projected trip generation has also been adjusted for pass-by trips. Pass-by trips are those that already exist on the network but make an intermediate stop at the site location. For example, a driver on their way to work may leave their primary route to buy a cup of coffee and then re-enter their route at the same location. This would add turning movements to the intersection by diverting pre-existing main street through movements. A visual representation of this is provided in Exhibit 5. In the St. Charles Prairie Centre case, pass-by trip generation would not apply to the residential uses. A 5\% pass-by rate was applied to the retail and restaurant uses. A higher pass-by rate would be reasonable given the proposed site uses, but IDOT limits the amount of internal capture and pass-by rates to a combined 20\%. The resulting trip generation, in terms of its impact to the external intersections, is therefore conservatively high. It was determined that pass-by trips would most likely utilize primarily the West Mall Entrance on IL Route 38. The pass-by trip adjustment is provided in Exhibit 6. In Table 7, a summary is given of the total new trips generated after the internal capture and pass-by adjustments.

Table 7
Total New Trips after all Adjustments in veh/hr

| Peak Hour | In | Out | Total |
| :---: | :---: | :---: | :---: |
| Weekday AM | 377 | 476 | 853 |
| Weekday PM | 493 | 377 | 870 |
| Saturday | 477 | 450 | 927 |

Source: ITE Trip Generation Manual, ght $^{\text {th }}$ Edition

## Estimated Trip Distribution

The direction by which traffic will approach and depart the site is dependent on a variety of factors. These factors include existing travel patterns, characteristics and operating conditions of the surrounding roadways, ease of access, location of population and employment centers, and locations of similar competing uses. Based on these factors and a familiarity with the sites and the environs, trip distribution estimates were developed and are presented on Exhibit 7. Separate distribution factors were used for residential, office, and commercial trips, which are reflected in the exhibit. In that exhibit, the Commercial rates provided apply to the restaurant and retail uses. Note that the Commercial rates also apply to the CVS Pharmacy use and its outlots in the development of the 2026 Base Year Traffic.

## Site Traffic Assignments

The estimated site-generated traffic volumes from the proposed development were assigned to the area roadway system based on the directional distribution identified above and on Exhibit 7. These volumes are the adjusted volumes, having removed internally captured trips and pass-by
trips, only leaving primary trips to and from the St. Charles Prairie Centre. The primary trip assignments for the proposed Prairie Centre development are illustrated on Exhibit 8 in Appendix A. The trip assignments presented on Exhibit 8 do not reflect assignment of the offsite development generated traffic from the CVS Pharmacy and its outlots described previously. These trips were included in the 2026 Base Year Traffic.

## Total Traffic Assignments

The development's generated site traffic assignment was then combined with the pass-by traffic and the 2026 Base Year projected traffic to develop a Total Traffic Assignment, shown on Exhibit 9 in Appendix A.

## V. Future Traffic Operations

Traffic Operations
Capacity analyses were conducted based on the traffic volumes estimated for the projected traffic assignments at the 16 intersections included in this study. Traffic signal warrant evaluation is discussed later in the report. There are several geometric improvements already included in these analyses. These improvements are those proposed at the entrances as shown on the developer's current concept plan. These improvements include reconstruction of the exit approaches to add or better demarcate turn lanes. The West Mall Entrance at IL Route 38 will have an exclusive left turn lane, an exclusive through lane, and an exclusive right turn lane on the exiting approach. At the East Mall Entrance on IL Route 38, the exiting approach will consist of an exclusive left turn lane and a shared through/right turn lane. The exiting approach of the East Mall Entrance on Prairie Street will have an exclusive left turn lane and an exclusive right turn lane. The lane configurations used in the 2026 Total Traffic analysis can be seen in Exhibit 10 in Appendix A.

For the analysis of 2026 Total Traffic conditions, no other geometric improvements were assumed. However, the signal timing plans were optimized for the anticipated traffic levels. There was one significant change recommended for this scenario. The existing northbound and southbound left turn phasing at the West Mall Entrance on IL Route 38 is permissive only. That is, left turns from the entrance approaches are currently only permitted with the green ball. Under the 2026 Total Traffic, it is recommended that exclusive left turn phase be added to these movements, converting them to protected/permitted left turns. Under protected/permitted operations, left turns are protected by left turn arrows for a portion of the cycle and then are permitted concurrently with the adjacent green ball during another portion of the cycle. Additionally, the analyses allowed for various lead/lag operations at Randall Road and Prairie Street. This intersection is owned by Kane County, and the signals have 4-section Flashing Yellow Arrow operation for all of the left turns, which enables a lot of flexibility. This flexibility in operation was not assumed at any of the IL Route 38 intersections because IDOT has been very hesitant to adopt such signal improvements and operations. Table 8 presents the results of the capacity analyses and provides a comparison to the year 2026 Base Year (no development) Traffic discussed earlier in this report. Synchro reports are also included in Appendices G and H for the 2026 Total Traffic with and without changes to the system.

Table 8
Summary of Base Year (no development) and Total (with site) Traffic Conditions Intersection Level of Service (LOS) and Delay (seconds)

Signalized Intersections

| Intersection | Base Year 2026 Traffic (no site) |  |  | 2026 Total Traffic (with site) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AM Peak | PM Peak | Sat Peak | AM Peak | PM Peak | Sat Peak |
| IL 38/ Randall Rd | D (40) | D (49) | D (45) | D (41) | D (52) | D (39) |
| IL 38/ West Mall Entrance | A (6) | A (8) | A (7) | A (8) | B (12) | B (17) |
| IL 38/ 14 ${ }^{\text {th }}$ St | C (24) | D (37) | C (31) | B (19) | C (34) | C (32) |
| Prairie St/ Randall Rd | B (12) | D (42) | D (44) | B (12) | D (46) | E (56) |

Note that when traffic growth occurs and signal timings are re-optimized to favor the highervolume arterial street traffic, the over-all intersection delay may reduce, even though delays for some minor street movements will increase. Some individual movements have LOS E or F. Table 9 shows a detailed breakdown of individual movements for the 2026 Total Traffic.

Table 9
LOS \& Delay by Movement for 2026 Total Traffic with Improvements

| Intersection | Peak <br> Hour |  | LOS \& (delay) by Movement |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
|  |  |  | L | T | R | L | T | R | L | T | R | L | T | R |
|  <br> Randall Rd | AM | D (41) | E <br> (61) | $\begin{gathered} \text { E } \\ (54) \\ \hline \end{gathered}$ | $\begin{gathered} \text { A } \\ (0) \\ \hline \end{gathered}$ | $\begin{gathered} \hline E \\ (62) \end{gathered}$ | $\begin{gathered} \text { D } \\ (51) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { B } \\ (18) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{E} \\ (61) \end{gathered}$ | $\begin{gathered} \hline \text { D } \\ (42) \\ \hline \end{gathered}$ | $\begin{gathered} \text { A } \\ (2) \\ \hline \end{gathered}$ | $\begin{gathered} E \\ (74) \\ \hline \end{gathered}$ | $\begin{gathered} C \\ (26) \\ \hline \end{gathered}$ | A <br> (4) |
|  | PM | D (52) | $\begin{gathered} \text { F } \\ (101) \end{gathered}$ | $\begin{gathered} \hline D \\ (49) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { B } \\ (11) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{E} \\ (62) \end{gathered}$ | $\begin{gathered} \mathrm{E} \\ (61) \end{gathered}$ | $\begin{gathered} \text { D } \\ (38) \\ \hline \end{gathered}$ | $\begin{array}{\|c} \hline E \\ (62) \\ \hline \end{array}$ | $\begin{gathered} \mathrm{E} \\ (79) \end{gathered}$ | A (8) | $\begin{gathered} \mathrm{F} \\ (86) \\ \hline \end{gathered}$ | $\begin{gathered} \text { C } \\ (21) \\ \hline \end{gathered}$ | $\begin{gathered} \text { A } \\ \text { (7) } \\ \hline \end{gathered}$ |
|  | Sat | D (39) | $\begin{gathered} \hline \text { D } \\ (50) \end{gathered}$ | $\begin{gathered} \mathrm{E} \\ (74) \end{gathered}$ | $\begin{gathered} \mathrm{B} \\ (13) \end{gathered}$ | $\begin{gathered} \text { D } \\ (48) \end{gathered}$ | $\begin{gathered} E \\ (55) \end{gathered}$ | $\begin{gathered} \text { D } \\ (48) \end{gathered}$ | $\begin{gathered} \hline \text { B } \\ (18) \end{gathered}$ | $\begin{gathered} \text { D } \\ (40) \end{gathered}$ | A (8) | $\begin{gathered} \text { D } \\ (50) \end{gathered}$ | $\begin{gathered} \text { C } \\ (28) \end{gathered}$ | A (4) |
|  <br> W Mall Entrance | AM | A (8) | A (2) | A (3) |  | A (5) | A (9) |  | $\begin{array}{\|c\|} \hline D \\ (38) \\ \hline \end{array}$ | B (18) |  | $\begin{gathered} \text { D } \\ (43) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { D } \\ (47) \\ \hline \end{gathered}$ | $\begin{gathered} \text { B } \\ (14) \\ \hline \end{gathered}$ |
|  | PM | B (12) | $\begin{gathered} \hline \mathrm{B} \\ (15) \end{gathered}$ | B(15) |  | A (2) | A (7) |  | $\begin{gathered} \hline D \\ (40) \end{gathered}$ | B (17) |  | $\begin{gathered} \hline D \\ (42) \end{gathered}$ | $\begin{gathered} \text { D } \\ (49) \end{gathered}$ | $\begin{gathered} \mathrm{B} \\ (16) \end{gathered}$ |
|  | Sat | B (17) | $\begin{gathered} \text { B } \\ (16) \end{gathered}$ | C (23) |  | A (3) | A (6) |  | $\begin{gathered} \hline \text { D } \\ (50) \end{gathered}$ | B (19) |  | $\begin{gathered} \text { D } \\ (54) \end{gathered}$ | $\begin{gathered} E \\ (59) \end{gathered}$ | $\begin{gathered} B \\ (16) \end{gathered}$ |
| $\begin{aligned} & \text { IL } 38 \text { \& } \\ & 14^{\text {th }} \mathrm{St} \end{aligned}$ | AM | B (19) | A (4) | A (8) |  | $\begin{gathered} \hline \text { B } \\ (15) \\ \hline \end{gathered}$ | B (19) |  | $\begin{array}{\|c\|} \hline C \\ (25) \\ \hline \end{array}$ | D (45) |  | $\begin{gathered} \text { D } \\ (35) \\ \hline \end{gathered}$ | C (35) |  |
|  | PM | C (34) | $\begin{gathered} \hline \text { D } \\ (40) \\ \hline \end{gathered}$ | B (19) |  | $\begin{gathered} \text { C } \\ (23) \end{gathered}$ | C (35) |  | $\begin{array}{\|c} \hline \text { C } \\ (25) \\ \hline \end{array}$ | E (59) |  | $\begin{gathered} \hline D \\ (45) \\ \hline \end{gathered}$ | D (41) |  |
|  | Sat | C (32) | A (9) | B (19) |  | $\begin{gathered} \mathrm{C} \\ (30) \end{gathered}$ | C (29) |  | $\begin{gathered} \mathrm{C} \\ (28) \end{gathered}$ | E (60) |  | $\begin{gathered} \text { D } \\ (46) \end{gathered}$ | D (42) |  |
| Prairie St \& Randall Rd | AM | B (12) | $\begin{gathered} \text { D } \\ (39) \\ \hline \end{gathered}$ | C (30) |  | $\begin{gathered} \mathrm{E} \\ (65) \end{gathered}$ | $\begin{gathered} \hline D \\ (48) \end{gathered}$ | $\begin{gathered} \mathrm{B} \\ (16) \end{gathered}$ | A <br> (2) | A (5) |  | $\begin{gathered} C \\ (29) \end{gathered}$ | B (11) |  |
|  | PM | D (46) | $\begin{gathered} \text { D } \\ (44) \end{gathered}$ | D (43) |  | $\begin{gathered} \text { F } \\ (130) \end{gathered}$ | $\begin{gathered} E \\ (55) \end{gathered}$ | $\begin{gathered} \text { C } \\ (29) \\ \hline \end{gathered}$ | $\begin{gathered} \text { C } \\ (22) \end{gathered}$ | D (55) |  | $\begin{gathered} \text { F } \\ (81) \end{gathered}$ | C (22) |  |
|  | Sat | E (56) | $\begin{gathered} \text { D } \\ (51) \\ \hline \end{gathered}$ | $F(83)$ |  | $\begin{gathered} F \\ (137) \\ \hline \end{gathered}$ | $\begin{gathered} E \\ (63) \\ \hline \end{gathered}$ | $\begin{gathered} \text { E } \\ (58) \\ \hline \end{gathered}$ | $\begin{gathered} C \\ (24) \\ \hline \end{gathered}$ | E (60) |  | $\begin{gathered} \text { F } \\ (117) \\ \hline \end{gathered}$ | C (32) |  |

Analysis of the 2026 Total Traffic shows the following results:
IL Route 38 and Randall Road: With projected 2026 Total Traffic and re-optimized signal timings, this intersection operates at an over-all Level of Service D, although some individual movements will operate at LOS E or F during peak times. There is a general increase in delay when compared to the 2026 Base Year Traffic, which would be expected due to the additional traffic. Vehicle queues do not exceed turn lane storage lengths, although throughlane queues will occasionally extend beyond the turn lane tapers, periodically blocking entry by turning vehicles until through traffic moves up. Traffic volumes will not exceed the capacity of the intersection, though several movements do approach capacity. This analysis is very similar to both the existing conditions and the 2026 Base Year conditions, in which the intersection also approaches capacity. Virtually all of the potential improvements that could be reasonably considered have already been implemented at this intersection. Additional capacity was already recognized by Kane County as a need for Randall Road, hence the projected widening of Randall Road, which is discussed later in this report. Widening of IL Route 38 could similarly have beneficial impacts on operation.

IL Route 38 and the West Mall Entrance: With the projected 2026 Total Traffic and reoptimized signal timings, as well as the additional northbound and southbound left turn phases, this intersection operates at an overall LOS B during the PM and Saturday peaks. Drivers on the proposed site roadway approaches may experience delays in the range of LOS D or even E. Left turn queues are expected to reach six to seven vehicles. Currently, IL Route 38 traffic is assigned about $75 \%$ of the available green time, which can be expected in the future, since these signals are controlled by IDOT. Traffic volumes are not expected to exceed the capacity of the intersection under 2026 Total Traffic conditions.

IL Route 38 and $14^{\text {th }}$ Street: With the projected 2026 Total Traffic and re-optimized signal timings, the intersection is expected to experience similar delays to those in the 2026 Base Year analysis, with different movements showing either slightly higher or slightly lower delay. The overall LOS is expected to be C or better. Analysis of 2026 Total Traffic shows that projected traffic will not exceed the capacity of the intersection.

Prairie Street and Randall Road: Analysis of 2026 Total Traffic with signal timings reoptimized and variable left turn phasing shows that the over-all Level of Service at this intersection will be $B$ to $E$. The queue of westbound left turning traffic will continue to overflow the westbound left turn lane frequently during peak times. This movement will operate at LOS F during the PM and Saturday peaks. The queue of southbound left turning traffic during the PM and Saturday peak hours can also be expected to overflow the left turn lane with vehicle queues of 10 to 15 cars. This movement will operate at LOS F during the AM and Saturday peaks. As previously noted, green time for Prairie Street will continue to be limited by the need to provide adequate time for the large volume of traffic flowing on Randall Road and to keep these movements coordinated with the nearby IL Route 38/Randall Road intersection to the south. Further, the northbound through queue can be expected to spill back into the Randall Road/IL Route 38 intersection in the PM and Saturday peaks. This intersection, similar to the existing and 2026 Base Year Traffic, is expected to operate at or near capacity in peak periods. As mentioned under the Base Year analysis, the best solution is to execute the planned widening of Randall Road. The widening of Randall Road is described in greater detail later in the report. Alternative improvements could include extending turn lanes, adding right turn lanes, or reconfiguring the driveways on

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Prairie Street east of Randall Road to reduce their potential interaction with the intersection, which is also discussed under the Base Year analysis.

In summary, the two Randall Road intersections are currently experiencing capacity issues, which are expected to continue into the future. The addition of traffic from new developments in the area will exacerbate these conditions. Optimizing the traffic signals and changing the order of phase operations can help significantly. Further improvements are needed to address what is an existing issue. Conversely, the other two signalized intersections studied are expected to continue to operate under capacity with relatively minimal changes, namely the addition of the left turn phases at the West Mall Entrance.

Table 10 shows a summary of analysis results for stop sign controlled intersections under 2026 Total Traffic. As noted before, capacity analyses of stop sign controlled intersections provide Levels of Service and delays for individual intersection movements, but not the intersection as a whole. Results for the most critical movement at each intersection are shown in Table 10.

Table 10
Summary of Base Year (no development) and Total (with site) Traffic Conditions Level of Service (LOS) and Delay (seconds)

Stop Sign Controlled Intersections

| Critical Movement | Base Year 2026 Traffic (no site) |  |  | 2026 Total Traffic (with site) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AM Peak | PM Peak | Sat Peak | AM Peak | PM Peak | Sat Peak |
| East Mall Entrance at IL 38 | $\begin{gathered} \mathrm{NB} \\ \mathrm{C}(23) \end{gathered}$ | $\begin{gathered} \hline \text { SB } \\ F(62) \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{~F}(85) \end{gathered}$ | $\begin{gathered} \hline \text { SBL } \\ \text { E (41) } \end{gathered}$ | $\begin{gathered} \hline \text { SBL } \\ F(162) \end{gathered}$ | $\begin{gathered} \hline N B \\ F(162) \end{gathered}$ |
| Prairie St at West Mall Ent | $\begin{gathered} \mathrm{NB} \\ \mathrm{~B}(11) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{~B}(13) \\ \hline \end{gathered}$ | $\begin{gathered} \text { NB } \\ \text { B (11) } \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{~B}(14) \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{C}(18) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{NB} \\ \mathrm{~B}(15) \\ \hline \end{gathered}$ |
| Prairie St at East Mall Entrance | $\begin{gathered} \mathrm{NBL} \\ \mathrm{~B}(15) \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{C}(19) \end{gathered}$ | $\begin{gathered} \text { NBL } \\ \text { C (16) } \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{C}(22) \end{gathered}$ | $\begin{gathered} \text { NBL } \\ \text { D (33) } \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{C}(25) \end{gathered}$ |
| $\begin{aligned} & \text { Prairie St at } \\ & 16^{\text {th }} \mathrm{St} \end{aligned}$ | $\begin{gathered} \text { SB } \\ B(14) \end{gathered}$ | $\begin{gathered} \text { SB } \\ \text { C (19) } \end{gathered}$ | $\begin{gathered} \text { SB } \\ C(17) \end{gathered}$ | $\begin{gathered} \text { SB } \\ C(16) \end{gathered}$ | $\begin{gathered} \text { SB } \\ C(23) \end{gathered}$ | $\begin{gathered} \text { SB } \\ C(21) \end{gathered}$ |
| Prairie St at $14^{\text {th }} \mathrm{St}$ | $\begin{gathered} \hline \text { NBL } \\ C(20) \end{gathered}$ | $\begin{gathered} \hline \text { NBL } \\ \mathrm{F}(73) \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{D}(33) \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{C}(25) \end{gathered}$ | $\begin{gathered} \mathrm{NBL} \\ \mathrm{~F}(111) \end{gathered}$ | $\begin{gathered} \hline \text { NBL } \\ \mathrm{E}(44) \end{gathered}$ |
| Prairie St at $7^{\text {th }}$ St | $\begin{gathered} E B \\ E(39) \\ \hline \end{gathered}$ | $\begin{gathered} \text { WB } \\ F(82) \end{gathered}$ | $\begin{gathered} \text { EB } \\ C(16) \end{gathered}$ | $\begin{gathered} E B \\ F(79) \end{gathered}$ | $\begin{gathered} \text { WB } \\ F(129) \end{gathered}$ | $\begin{gathered} E B \\ C(23) \\ \hline \end{gathered}$ |
| $\begin{array}{\|l} \hline \text { Prairie St at } \\ 3^{\text {rd }} \mathrm{St} \end{array}$ | $\begin{gathered} \text { EB } \\ \text { E (50) } \end{gathered}$ | $\begin{gathered} \text { WB } \\ F(197) \\ \hline \end{gathered}$ | $\begin{aligned} & \text { WB } \\ & \text { C (21) } \end{aligned}$ | $\begin{gathered} \text { EB } \\ F(86) \\ \hline \end{gathered}$ | $\begin{gathered} \text { WB } \\ \text { F (235) } \end{gathered}$ | $\begin{gathered} \text { WB } \\ \text { D (30) } \end{gathered}$ |
| $14^{\text {th }}$ St at Covington Ct | $\begin{gathered} \text { WB } \\ B(13) \end{gathered}$ | $\begin{gathered} \text { WB } \\ C(19) \end{gathered}$ | $\begin{gathered} \text { WB } \\ B(14) \end{gathered}$ | $\begin{gathered} \text { WB } \\ \text { B (13) } \end{gathered}$ | $\begin{gathered} \text { WB } \\ \text { C (20) } \end{gathered}$ | $\begin{gathered} \text { WB } \\ B(14) \end{gathered}$ |
| $14^{\text {th }}$ St at Vanderbilt Dr | $\begin{gathered} \text { EB } \\ \text { B (12) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { EB } \\ B(15) \\ \hline \end{gathered}$ | $\begin{gathered} \text { EB } \\ \text { B (14) } \\ \hline \end{gathered}$ | $\begin{gathered} \text { EB } \\ B(11) \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{EB} \\ \mathrm{~B}(15) \\ \hline \end{gathered}$ | $\begin{gathered} \text { EB } \\ \text { B (14) } \\ \hline \end{gathered}$ |

Analysis of 2026 Total Traffic shows that critical movements at the stop-controlled intersections included in the analysis operate in range of Level of Service from B to F. Below is a description of the intersections not meeting LOS D or better. See also Exhibit 12 for queue comparisons.

East Mall Entrance at IL Route 38: The southbound left turn movement is expected to operate at LOS F during the PM and Saturday peak hours. However, the vehicle queues are only anticipated to reach two cars, and the analyses show the intersection having reached

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only about half its capacity. The large delay is caused by the large volume of through traffic on IL Route 38. There will be very few acceptable gaps for left turning vehicles to utilize. Further geometric improvements will not significantly affect the delay. Signalization is not appropriate to consider, as IDOT would not allow it due to the close spacing of existing signalized intersections nearby. No improvements are recommended.
$14^{\text {th }}$ Street at Prairie Street: The northbound left turn movement is expected to operate at a LOS E during the Saturday peak hour and F during the PM peak hour. Vehicle queues are not expected to grow by more than one vehicle. However, a left turning vehicle may be delayed an extended period of time waiting for a sufficient gap in traffic. The analysis results also show plenty of remaining capacity at this intersection. No improvements are recommended. See the Base Year analysis for a discussion of signalization.

Prairie Street at $7^{\text {th }}$ Street: This intersection is expected to exceed capacity with the addition of 2026 Total Traffic volumes. Both the east- and westbound movements at this intersection are expected to see LOS F during the AM and PM peak hours. This delay is caused by the large volume of vehicles making a stop at this intersection. According to the analysis results, the intersection has not reached capacity. No improvements are recommended.

Prairie Street at 3 ${ }^{\text {rd }}$ Street: This intersection is expected to be near capacity in the PM peak hour with the addition of 2026 Total Traffic volumes. Several movements at this intersection are expected to reach LOS F during the peak hour. This delay is caused by the large volume of vehicles proceeding through this intersection. As described previously, the interaction between this intersection and the nearby signalized intersection of IL Route 31 and Prairie Street may need some attention. No mitigation is recommended.

## Randall Road Widening

Kane County has completed a Phase I engineering study and design to widen Randall Road to provide six through-traffic lanes from north of IL Route 64 to south of Bricher Road. The first stage of this project was constructed in 2007, widening Randall Road from north of IL Route 64 south to Oak Street. The widening of Randall Road from Oak Street through the IL Route 38 intersection is planned as Stage 2 of the improvement. Construction is not currently programmed while the County seeks funding for the work.

If the Randall Road improvement is constructed by 2026, which can be reasonably anticipated, it will benefit traffic flow not only on Randall Road, but also on intersecting routes such as IL Route 38 and Prairie Street. Analysis of the peak hours at Randall Road intersections with six lanes on Randall Road is expected to show that the added capacity will reduce delays and shorten vehicle queues on all approaches at both intersections. Table 11 presents the results of the capacity analyses and provides a comparison to the year 2026 Total Year (with site) Traffic discussed earlier in this report. For the analyses shown, the only improvements assumed were the same as described in the previous section plus the widening of Randall Road from four to six lanes. As before, the phase order at Randall Road and Prairie Street was allowed to change by time of day. Synchro reports are also included in Appendix I.

Note at Randall Road and Prairie Street, the northbound and southbound left turn movements have been evaluated with protected-only phasing, meaning turns are only allowed on a green arrow. This is required because IDOT does not allow permissive phasing across three opposing through lanes.

Table 11
Summary of Total (with site) and Total (with site) with Randall Widening Traffic Conditions Intersection Level of Service (LOS) and Delay (seconds)

Signalized Intersections

| Intersection | 2026 Total $\begin{gathered}\text { Year Traffic (with } \\ \text { site) }\end{gathered}$ |  |  | 2026 Total Traffic (with site) with Randall Road Widening |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | AM Peak | PM Peak | Sat Peak | AM Peak | PM Peak | Sat Peak |
| IL 38/ Randall Rd | D (41) | D (52) | D (39) | C (32) | D (35) | C (27) |
| IL 38/ West Mall Entrance | A (8) | B (12) | B (17) | B (11) | B (12) | B (13) |
| IL 38/ 14 ${ }^{\text {th }}$ St | B (19) | C (34) | C (32) | B (19) | C (35) | C (31) |
| Prairie St/ Randall Rd | B (12) | D (46) | E (56) | B (12) | C (28) | C (30) |

Note that with the widening of Randall Road assumed and signal timings re-optimized to favor the higher-volume arterial street traffic, the over-all intersection delay may reduce, even though delays for some minor street movements will increase. Some individual movements have LOS E or F. Table 12 shows a detailed breakdown of individual movements for the 2026 Total Traffic with Randall Road widening.

Table 12
LOS \& Delay by Movement for 2026 Total Traffic with Improvements and Randall Widening

| Intersection | Peak <br> Hour | Overall LOS \& (delay) | LOS \& (delay) by Movement |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Eastbound |  |  | Westbound |  |  | Northbound |  |  | Southbound |  |  |
|  |  |  | L | T | R | L | T | R | L | T | R | L | T | R |
|  <br> Randall Rd | AM | C (32) | D (51) | $\begin{gathered} \text { D } \\ (42) \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { A } \\ (0) \\ \hline \end{gathered}$ | $\begin{gathered} \text { D } \\ (47) \\ \hline \end{gathered}$ | $\begin{gathered} \text { D } \\ (38) \\ \hline \end{gathered}$ | $\begin{gathered} \text { B } \\ (15) \\ \hline \end{gathered}$ | $\begin{gathered} \text { D } \\ (48) \end{gathered}$ | $\begin{gathered} C \\ (34) \\ \hline \end{gathered}$ | A <br> (4) | $\begin{gathered} \mathrm{E} \\ (63) \end{gathered}$ | $\begin{gathered} \hline B \\ (19) \\ \hline \end{gathered}$ | A <br> (2) |
|  | PM | D (35) | $\begin{gathered} \mathrm{E} \\ (68) \end{gathered}$ | $\begin{gathered} \hline D \\ (45) \end{gathered}$ | $\begin{gathered} \text { A } \\ (4) \end{gathered}$ | $\begin{gathered} \hline D \\ (48) \end{gathered}$ | $\begin{gathered} \hline D \\ (44) \end{gathered}$ | $\begin{gathered} \text { C } \\ (32) \end{gathered}$ | $\begin{gathered} \mathrm{E} \\ (62) \end{gathered}$ | $\begin{gathered} \text { D } \\ (39) \end{gathered}$ | A <br> (7) | $\begin{gathered} \mathrm{E} \\ (62) \end{gathered}$ | $\begin{gathered} \mathrm{B} \\ (18) \end{gathered}$ | A <br> (8) |
|  | Sat | C (27) | $\begin{gathered} \text { C } \\ (34) \end{gathered}$ | $\begin{gathered} \text { D } \\ (53) \end{gathered}$ | $\begin{gathered} \text { A } \\ (8) \end{gathered}$ | $\begin{gathered} \text { C } \\ (27) \end{gathered}$ | $\begin{gathered} \text { D } \\ (42) \\ \hline \end{gathered}$ | $\begin{gathered} \text { C } \\ (34) \end{gathered}$ | $\begin{gathered} \mathrm{B} \\ (16) \end{gathered}$ | $\begin{gathered} \text { C } \\ (33) \end{gathered}$ | $\begin{gathered} \text { A } \\ \text { (8) } \end{gathered}$ | $\begin{gathered} C \\ (30) \\ \hline \end{gathered}$ | $\begin{gathered} B \\ (15) \end{gathered}$ | $\begin{gathered} \text { A } \\ \text { (5) } \end{gathered}$ |
|  <br> W Mall <br> Entrance | AM | B (11) | A (2) | A (6) |  | $\begin{gathered} \text { B } \\ (10) \\ \hline \end{gathered}$ | B (19) |  | $\begin{gathered} \mathrm{C} \\ (29) \end{gathered}$ | B (15) |  | $\begin{gathered} \text { C } \\ (32) \\ \hline \end{gathered}$ | $\begin{gathered} \text { D } \\ (38) \\ \hline \end{gathered}$ | $\begin{gathered} \text { B } \\ (11) \\ \hline \end{gathered}$ |
|  | PM | B (12) | $\begin{gathered} \mathrm{B} \\ (17) \\ \hline \end{gathered}$ | B(15) |  | A (2) | A (6) |  | $\begin{gathered} \hline D \\ (40) \end{gathered}$ | B (17) |  | $\begin{gathered} \hline D \\ (42) \end{gathered}$ | $\begin{gathered} \text { D } \\ (49) \end{gathered}$ | $\begin{gathered} B \\ B \\ (16) \end{gathered}$ |
|  | Sat | B (14) | A (9) | B (14) |  | A (4) | A (8) |  | $\begin{array}{\|c} \hline \mathrm{D} \\ (40) \\ \hline \end{array}$ | B (17) |  | $\begin{gathered} \mathrm{D} \\ (44) \end{gathered}$ | $\begin{gathered} \text { D } \\ (49) \end{gathered}$ | $\begin{gathered} \mathrm{B} \\ (16) \end{gathered}$ |
| $\begin{aligned} & \text { IL } 38 \text { \& } \\ & 14^{\text {th }} \mathrm{St} \end{aligned}$ | AM | B (19) | A (6) | B (12) |  | $\begin{gathered} \text { B } \\ (15) \end{gathered}$ | B (20) |  | $\begin{array}{\|c\|} \hline C \\ (20) \\ \hline \end{array}$ | D (37) |  | $\begin{gathered} C \\ (27) \end{gathered}$ | C (27) |  |
|  | PM | C (35) | $\begin{gathered} \text { D } \\ (40) \end{gathered}$ | C (22) |  | $\begin{gathered} \text { C } \\ (23) \end{gathered}$ | C (35) |  | $\begin{gathered} C \\ (25) \end{gathered}$ | E (59) |  | $\begin{gathered} \text { D } \\ (45) \end{gathered}$ | D (41) |  |
|  | Sat | C (31) | A (9) | C (23) |  | $\begin{gathered} \text { C } \\ (31) \end{gathered}$ | C (27) |  | $\begin{gathered} \text { C } \\ (24) \end{gathered}$ | D (53) |  | $\begin{gathered} \text { D } \\ (39) \end{gathered}$ | D (37) |  |
| Prairie St \& Randall Rd | AM | B (12) | $\begin{gathered} \text { C } \\ (30) \\ \hline \end{gathered}$ | C (25) |  | $\begin{gathered} \text { D } \\ (51) \end{gathered}$ | $\begin{gathered} \hline D \\ (39) \end{gathered}$ | A <br> (6) | $\begin{gathered} \text { C } \\ (29) \\ \hline \end{gathered}$ | A (6) |  | $\begin{gathered} E \\ (55) \end{gathered}$ | B (10) |  |
|  | PM | C (28) | $\begin{gathered} D \\ (38) \\ \hline \end{gathered}$ | D (40) |  | $\begin{gathered} \mathrm{E} \\ (67) \end{gathered}$ | $\begin{gathered} \hline D \\ (47) \end{gathered}$ | $\begin{gathered} \text { C } \\ (20) \end{gathered}$ | $\begin{gathered} \hline D \\ (50) \\ \hline \end{gathered}$ | C (22) |  | $\begin{gathered} \mathrm{E} \\ (77) \end{gathered}$ | B (20) |  |
|  | Sat | C (30) | $\begin{gathered} D \\ (40) \end{gathered}$ | D (52) |  | $\begin{gathered} E \\ (74) \end{gathered}$ | $\begin{gathered} \hline D \\ (50) \\ \hline \end{gathered}$ | $\begin{gathered} \text { C } \\ (26) \\ \hline \end{gathered}$ | $\begin{gathered} \hline D \\ (51) \\ \hline \end{gathered}$ | C (24) |  | $\begin{gathered} \mathrm{F} \\ (87) \end{gathered}$ | C (20) |  |

If the proposed add-lanes improvement to Randall Road is in place by 2026, delays and queues are expected to be significantly reduced. This improvement is particularly necessary to alleviate the spillback concerns on Randall Road between IL Route 38 and Prairie Street. Note however, there will still be queuing concerns for the southbound and westbound left turn lanes at Randall Road and Prairie Street. This is due to the requirement to use protected-only phasing for the northbound and southbound left turns. Permissive phasing would help reduce the expected queues but would need to be studied carefully and would require obtaining concurrence from IDOT.

## Traffic Signal Warrants

Traffic Signal Warrants were analyzed at three intersections with this study. Traffic Signal Warrants are analytical tools outlined in Part 4C of the Manual on Uniform Traffic Control Devices (MUTCD) published by the Federal Highway Administration (FHWA). The MUTCD outlines nine warrants dealing with various factors in the need for traffic signals, such as traffic volumes, pedestrian activity, school crossings, railroad crossings, etc. It is important to understand that meeting at least one traffic signal warrant is necessary before traffic signals should be installed at an intersection. However, meeting a traffic signal warrant does not require an agency to install traffic signals at an intersection. Often, traffic signals are not the best solution to handling traffic, even where warrants are met.

For the analyses provided, 13-hour traffic count data was collected. For analyses of the existing traffic, all count data was able to be used. For future years, projection of traffic volume counts is only reasonable for the peak hour volumes. Therefore, analysis of Warrant 1, Eight-hour Vehicular Volume, is limited to what can be estimated. The eighth-highest hour of traffic volume can be estimated as $55 \%$ of the peak hour volume. This rule of thumb is an industry practice and is used by IDOT in their Signal Warrant analyses (IDOT BDE Manual, 2002 Ed., p. 14-3(3), item 4c. Proposed Volumes). Caution needs to be applied when using the $55 \%$ estimate of eighth-highest hourly volumes to evaluate Warrant 1. Similarly, Warrant 2, Four-hour Vehicular Volume, is even more difficult to analyze. There is no accepted method to estimate the fourthhighest hourly volume in the absence of actual traffic count data. Subsequently, where the estimated eighth-highest hourly volume satisfies Warrant 2, it could be reasonably assumed the warrant is met. However, if the peak hour volumes meet it but the eighth-highest hourly volume does not, no conclusion can be drawn regarding Warrant 2.

Prairie Street \& $14^{\text {th }}$ Street: Traffic Signal Warrant 1A, Minimum Vehicular Volumes, has been met by existing traffic volumes at this intersection. This means that, for at least eight hours of the day, the traffic counted at this intersection exceeded the thresholds outlined in the MUTCD. Traffic Signal Warrant 3, Peak Hour, also has been met with existing traffic volumes. Additional warrants are anticipated to be met by future traffic volumes. However, despite delays to the northbound left turn, the capacity analyses show remaining capacity in the operations and short queue lengths under all scenarios studied. Installation of traffic signal control is not recommended at this time.

Prairie Street \& $7^{\text {th }}$ Street: Traffic Signal Warrant 2, Four-hour Vehicular Volume, has been met by existing traffic volumes at this intersection. This means that, for at least four hours of the day, the traffic counted at this intersection exceeded the thresholds outlined in the MUTCD. Additional warrants are anticipated to be met by future traffic volumes. Traffic operations at this intersection are at or near capacity with or without the site traffic added. Note also the distance of the intersection from the site, which makes it difficult to assign
impact from the site to this intersection relative to existing traffic and surrounding traffic generators. Installation of traffic signals at this intersection could potentially improve operations, although this and other remedies have not been studied in detail. Installation of traffic signal control or other mitigation techniques are not recommended at this time.

Prairie Street \& 3 ${ }^{\text {rd }}$ Street: Traffic Signal Warrant 2, Four-hour Vehicular Volume, has been met by existing traffic volumes at this intersection. This means that, for at least four hours of the day, the traffic counted at this intersection exceeded the thresholds outlined in the MUTCD. Additional warrants are anticipated to be met by future traffic volumes. Traffic operations at this intersection are at or near capacity with or without the site traffic added. Note also the distance of the intersection from the site, which makes it difficult to assign impact from the site to this intersection relative to existing traffic and surrounding traffic generators. Installation of traffic signals at this intersection could potentially improve operations, although this and other remedies have not been studied in detail. Installation of traffic signal control or other mitigation techniques are not recommended at this time.

## VI. Additional Considerations

## Cut Through Routes

Some concerns have been raised about the existing use of neighborhood residential streets by cut-through traffic. These streets include $7^{\text {th }}$ Street, $12^{\text {th }}$ Street, $13^{\text {th }}$ Street, $14^{\text {th }}$ Street, $16^{\text {th }}$ Street and Oak Street. Seventh, $12^{\text {th }}$, and Oak Streets adjoin schools and are school traffic routes. Seventh and Oak Streets, along with Prairie Street, are designated as collector streets in the City's Comprehensive Plan. Concerns have also been raised about the potential for increased use of these streets by cut-through traffic with the development of the Prairie Centre site and the additional traffic it will generate.

A study and analysis that would quantify the existing volumes of cut-through traffic on surrounding west-side neighborhood streets would be an undertaking that requires a significant investment of time and manpower. Due to the variable nature of this driver behavior, a prediction of any future increase in cut-through traffic on individual streets could only be very roughly estimated. Any recommendations to discourage or impede cut-through traffic on an individual street would need to be carefully considered, as this often results in traffic diverting to the nearest adjacent street. Following a discussion of this with City staff, HLR was directed to not undertake a formal engineering study of cut-through traffic, but to instead provide an overview of the potential cause and remedy for this situation.

Drivers typically use side streets as cut-through routes to avoid traffic congestion along an otherwise desired route. If traffic is flowing smoothly, drivers are more likely to stay on the through street. During peak traffic periods, Prairie Street now experiences some congestion at its all-way stop intersections at $7^{\text {th }}$ and $3^{\text {rd }}$ Streets. Traffic volumes at these intersections do meet traffic signal warrants without the addition of the Prairie Centre development.

If congestion is not alleviated on the main collector route (such as Prairie Street), measures taken to discourage or block through traffic on individual side streets often result in traffic rerouting to the next adjacent street. If the $3^{\text {rd }}$ Street and $7^{\text {th }}$ Street intersections remain under all-way stop control, peak hour delays and the lengths of lines of traffic will increase, further encouraging drivers to seek alternate routes down other streets. If both intersections are signalized and are under coordinated control with each other and with Prairie Street at IL Route

31, delays and congestion will be significantly reduced. This will reduce the desirability of using cut-through routes down side streets, drawing those drivers back to Prairie Street, the main collector route.

## Traffic Calming

Traffic should continue to be monitored on the surrounding street network for issues that may require traffic calming. Should measures be required, the City of St. Charles has a traffic calming policy that should be followed.

The developer has many features planned to be incorporated into the site plan that will have traffic calming benefits for the on-site traffic circulation. The following list enumerates some of the planned traffic calming features and other design features that will have some traffic calming effects:

- Central, circular, landscaped island/traffic circle on the main driveway
- Narrow vehicle lanes
- Brick crosswalks
- Corner bump-outs
- On-street parking
- Non-continuous vehicle routes through the site


## On-site Traffic Circulation

A detailed review of the site plan should be conducted by City staff and by the Fire Department to ensure that adequate access is provided for emergency vehicles throughout the site. When geometric plans for the access lanes within the site are finalized, they should be reviewed for access by the largest St. Charles Fire Department truck, which can be approximated with a WB50 turning template. Locations of trash collection areas and standing/parking areas for service and delivery vehicles should also be reviewed to ensure that these operations do not block traffic circulation. Truck access to the Jewel store west of the site will be modified or affected by the development of the Prairie Centre site. Care should also be taken in the site design to ensure that the trucks have a viable route in and out of the site. Finally, the site plan should provide for sufficient lane widths throughout the site, particularly at locations where it can be expected that traffic will need to go around stopped or standing vehicles in loading zones, etc.

Many of the traffic calming features described above also have, or primarily have, benefits for pedestrian traffic. Considering the nature of the development and the surrounding residential areas, the site design should include safe and efficient pathways for pedestrian use. The current preliminary site plan appears to make safe pedestrian access a priority. As the site plan develops further, it should be reviewed by City staff to ensure that safe and efficient pedestrian routing is provided.

Finally, it should be noted that, even with installation of the pedestrian features previously described, and considering the relatively dense nature of the development, it is unlikely that the Prairie Centre site will generate significant pedestrian traffic that would cross IL Route 38. Adjacent to the site, IL Route 38 is a five-lane arterial maintained by IDOT. Due to the width of the street and the volume of traffic on IL Route 38, it would be difficult for pedestrians to cross and feel safe. Further, with this site being located in a northern climate, user habits will tend toward moving the vehicle for each stop, especially when crossing to sites on the opposite side of IL Route 38.

## Alternate Development Ideas

As a point of reference, this section discusses some potential alternatives for development of the Prairie Centre site. Comparisons of estimated generated traffic from two alternatives to the generated traffic presented in this study are summarized in this section. The first comparison was to the estimated volumes from the previous study in 2010. The second comparison was to a development scenario that could potentially be constructed under the existing zoning of the site.

The last time this site was studied was in 2010, and the site was referred to as the Towne Centre. At that time, the general concept for development of the site was similar to the current proposal. The primary difference was in the number of units on the site. The residential use was slightly more intense in the 2010 proposal as compared to the current one. The retail uses, including restaurants, had about three times the square footage. The previous proposal also included a hotel and about 142,000 SF of office space, which are not included in the current proposal. Table 13 summarizes the trips generated in the 2010 study. Note that the previous study was performed prior to the publication of the $9^{\text {th }}$ Edition of Trip Generation Manual. The data presented in Table 13 were taken straight from that study. The 2010 study based trip generation on the $8^{\text {th }}$ Edition of Trip Generation Manual, which was the current edition at that time. The formulas and rates in the $9^{\text {th }}$ Edition are only slightly different for most of the assumed land uses, while many of the uses have no changes. Also note that the data presented in Table 13 account for a combined reduction of $20 \%$ due to internal capture and to pass-by trips. However, pass-by does not apply to all uses, such as residential. Therefore, the values for the combined internal capture and pass-by trips will not equal $20 \%$ of the raw total generated trips.

Table 13
Trip Generation Table from 2010 Towne Centre TIS

| Land Use | ITE Code | Units | Qty | AM Peak Hour Volumes (veh/hr) |  |  | PM Peak Hour Volumes (veh/hr) |  |  | SAT Peak Hour Volumes (veh/hr) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | In | Out | Total | In | Out | Total | In | Out | Total |
| Residential | 233 | DU | 650 | 54 | 182 | 236 | 303 | 179 | 482 | 125 | 106 | 231 |
| Office | 710 | 1000 SF | 142 | 218 | 30 | 248 | 40 | 198 | 238 | 26 | 23 | 49 |
| Commercial | 820 | 1000 SF | 349 | 196 | 126 | 322 | 721 | 750 | 1471 | 1005 | 927 | 1932 |
| Hotel | 310 | Rms | 115 | 30 | 19 | 49 | 36 | 32 | 68 | 47 | 37 | 84 |
| Total Generated Trips |  |  |  | 498 | 357 | 855 | 1100 | 1159 | 2259 | 1203 | 1093 | 2296 |
| Internal Capture + Pass-by (20\%) |  |  |  | (68) | (52) | (120) | (180) | (186) | (366) | (210) | (199) | (409) |
| Total New Trips |  |  |  | 430 | 305 | 735 | 920 | 973 | 1893 | 993 | 894 | 1887 |

Source: Towne Centre Traffic Impact Study, January 26, 2010
For another comparison, City staff recommended generating traffic based on a land use assumption that assumes full development of the site with land uses that conform to the existing zoning for the site. Being constructed per existing zoning, this potential development would not require review by the City's Plan Commission. This scenario was also examined in the 2010 Towne Centre study. At that time, City staff provided recommended land use assumptions for the basis of this analysis via a memo dated January 29, 2009, which is included in Appendix K for reference. The City's recommendation was to model a development consisting of approximately 626,000 SF of gross leasable area of Shopping Center (ITE Land Use 820). Table 14 summarizes the estimated generated trips for such a development with and without an estimated combined 20\% of internal capture and pass-by.

Table 14 - Trip Generation Table for Alternate Development of Prairie Centre Site

| Land Use | ITE Code | Units | Qty | AM Peak Hour Volumes (veh/hr) |  |  | PM Peak Hour Volumes (veh/hr) |  |  | SAT Peak Hour Volumes (veh/hr) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | In | Out | Total | In | Out | Total | In | Out | Total |
| Shopping Center | 820 | 1000 SF | 626 | 296 | 181 | 477 | 983 | 1066 | 2049 | 1498 | 1384 | 2882 |
| Internal Capture + Pass-by (20\%) |  |  |  | (59) | (36) | (95) | (197) | (213) | (410) | (300) | (277) | (577) |
| Total New Trips |  |  |  | 237 | 145 | 382 | 786 | 853 | 1639 | 1198 | 1107 | 2305 |

Source: ITE Trip Generation Manual, gth $^{\text {th }}$ Edition
In general, both of the outlined alternatives are expected to generate much larger volumes of traffic than does the current proposal. Although the weekday AM peak hour volumes are expected to be similar in all three cases, the generated volumes are much higher in the weekday PM and Saturday peak hour cases. Tables 15 and 16 summarize and compare the generated traffic for each development case with a percentage change relative to the traffic generated by the current proposal. Table 15 shows the comparison without any internal capture or pass-by adjustments, while Table 16 includes those adjustments.

Table 15
Total Site Generated Traffic (vph) without Internal Capture or Pass-by Adjustments by Development Scenario and Peak Hour

| Development Scenario | AM Peak | PM Peak | SAT Peak |
| ---: | :---: | :---: | :---: |
|  | Total | Total | Total |
| Prairie Centre | 1040 | 1059 | 1131 |
| Towne Centre 2010 |  |  |  |
| Percentage Difference | $-18 \%$ | $+113 \%$ | $+103 \%$ |
|  |  |  |  |
| Max Development per Current Zoning | 477 | 2049 | 2882 |
| Percentage Difference | $-54 \%$ | $+93 \%$ | $+155 \%$ |

Sources: ITE Trip Generation Manual, ght $^{\text {th }}$ Edition and Towne Centre Traffic Impact Study, January 26, 2010

Table 16
Total Site Generated Traffic (vph) with Internal Capture and Pass-by Adjustments by Development Scenario and Peak Hour

| Development Scenario | AM Peak | PM Peak | SAT Peak |
| :---: | :---: | :---: | :---: |
|  | Total | Total | Total |
| Prairie Centre | 853 | 870 | 927 |
| Towne Centre 2010 |  |  |  |
| Percentage Difference | $-14 \%$ | $+117 \%$ | $+104 \%$ |
|  |  |  |  |
| Max Development per Current Zoning | 382 | 1639 | 2305 |
| Percentage Difference | $-55 \%$ | $+88 \%$ | $+149 \%$ |

Sources: ITE Trip Generation Manual, ght $^{\text {th }}$ Edition and Towne Centre Traffic Impact Study, January 26, 2010

As shown, the 2010 Towne Centre proposal was expected to generate just over twice the traffic of the Prairie Centre proposal for the weekday PM and Saturday peaks, while the Saturday peak is 1.5 times the Prairie Centre traffic for the potential development under current zoning. Although the weekday AM peak volume is expected to be higher for the Prairie Centre as compared to both alternate scenarios, the critical peak hours were still the weekday PM and Saturday peak hours, for which both the generated traffic volumes and the counted traffic volumes were higher than those for the weekday AM peak hour.

The larger volumes anticipated for the alternate development scenarios result in more significant impacts to the surrounding roadway network. To provide satisfactory operation, more improvements are needed. Dual left turn lanes would be needed at the main entrance on IL Route 38. Many turn lanes in the area would need to be extended. The interaction on Randall Road between IL Route 38 and Prairie Street would be exacerbated. Longer delays would be experienced at many of the driveways where further improvements are unrealistic for various reasons.

## VII. Findings and Recommendations

The estimates and analyses discussed in the preceding pages, based on the proposed site layout and access as shown in Exhibit 2, indicate the following:

IL Route 38 \& Randall Road: This intersection and its turn lanes have sufficient capacity to accommodate the projected 2026 Total Site traffic. Individual movements may operate at LOS E or F, though this is not entirely attributed to the Prairie Centre Development as this occurs in the 2026 Base Traffic scenario also. The proposed future Kane County improvement to add through-traffic lanes to Randall Road will further enhance the capacity of this intersection. No improvements to this intersection are recommended as a result of the proposed development.

IL Route 38 \& West Mall Entrance: The site plans show conversion of the southbound approach to consist of an exclusive left turn lane, an exclusive through lane, and an exclusive right turn lane. Additionally, the northbound and southbound left turn phases should be converted to protected-permissive phasing.

IL Route 38 \& East Mall Entrance: This intersection has the overall capacity to accommodate the projected 2026 Total Traffic. The north- and southbound left turn movements will experience long delays and LOS F but with short queues.

IL Route $38 \& 14^{\text {th }}$ Street: This intersection has the overall capacity to accommodate the projected 2026 Total Traffic. The eastbound and westbound left turn queues will periodically approach the full left turn lane length during PM and Saturday peak hours. If needed, the painted median and taper can be adjusted to increase the storage length of the left turn lane. Further, consideration should be made for adding a northbound right turn lane to reduce existing delays and queues on that approach.

Randall Road \& Prairie Street: This intersection has the overall capacity to accommodate the projected 2026 Total Traffic. The peak period queues of southbound left turning traffic are anticipated to be as long as 10 to 15 cars. This is due partly to a projected increase in the left turning volume and partly to anticipated background growth in opposing through-
traffic volumes, limiting the amount of green time available for the left turn movement. An interim improvement can be made by adjusting the median and left turn lane alignment to provide additional storage length in the taper. If the proposed add-lanes improvement to Randall Road is in place by 2026, the southbound left turn queues can be expected to be reduced. There is also the potential for spillback on the northbound approach into the IL Route 38 intersection during the PM and Saturday peak periods. The proposed widening of Randall Road will be necessary to alleviate this concern.

Further, the existing peak period overflow of the westbound left turn lane will be increased with the addition of projected traffic, frequently blocking the Jewel Supermarket driveway, as well as the entrance to the retail strip mall on the south side of the street west of the Jewel store. These entrances could be converted to right turns only, as described in detail in the Base Year analysis. These improvements are not recommended with this development, as it treats what is an existing issue. Additionally, the degree of improvement needed will be impacted by the planned Randall Road widening. Therefore, the specific improvements to the westbound approach should be developed with some accounting for the Randall Road widening and when the two sets of improvements would happen relative to each other.

Prairie Street \& $14^{\text {th }}$ Street: Existing average delays to the northbound left turn movement are in the range of LOS D. With the addition of the background growth and the site development traffic, this movement will enter LOS F range during the PM peak hour. However, this intersection has the overall capacity to accommodate the 2026 Total Traffic volumes, with queues reaching four vehicles at most. Although traffic signal warrants are met under the existing traffic at the intersection, installation of traffic signal control is not recommended at this time. See Base Year analysis for more discussion about signalization.

Prairie Street \& $7^{\text {th }}$ Street: Base Year average delays for this intersection reach LOS F during the PM peak hour. The addition of site development traffic only adds to the delays at this intersection. For the westbound approach, queues will nearly reach the Prairie Street \& $4^{\text {th }}$ Street intersection. Traffic signal control is warranted under existing traffic. A number of improvements could alleviate delays, including signalization or conversion to a roundabout. No improvements are recommended as a part of the development, as the issues here are primarily existing.

Prairie Street \& 3 ${ }^{\text {rd }}$ Street: Base Year average delays for this intersection reach LOS F during the PM peak hour. The addition of site development traffic only adds to the delays at this intersection. For the westbound approach, queues will nearly reach the IL Route 31 and Prairie Street intersection. Traffic signal control is warranted under existing traffic. A number of improvements could alleviate delays, including signalization or conversion to a roundabout. No improvements are recommended as a part of the development, as the issues here are primarily existing $\ldots \ldots \ldots$,
Respectfully Submitte


Hampton, Lenzini and F

## APPENDIX

A. EXHIBITS
B. HCS ANALYSIS REPORTS, 2016 EXISTING TRAFFIC
C. HCS ANALYSIS REPORTS, 2026 BASE TRAFFIC
D. HCS ANALYSIS REPORTS, 2026 TOTAL TRAFFIC
E. SYNCHRO ANALYSIS REPORTS, 2016 EXISTING TRAFFIC
F. SYNCHRO ANALYSIS REPORTS, 2026 BASE TRAFFIC
G. SYNCHRO ANALYSIS REPORTS, 2026 TOTAL TRAFFIC, NO IMPROVEMENTS
H. SYNCHRO ANALYSIS REPORTS, 2026 TOTAL TRAFFIC, WITH IMPROVEMENTS
I. SYNCHRO ANALYSIS REPORTS, 2026 TOTAL TRAFFIC, RANDALL WIDENING
J. CMAP MEMO TO ST. CHARLES, JUNE 3, 2016
K. ST. CHARLES ALTERNATE DEVELOPMENT MEMO, JANUARY 29, 2009
L. ITE LAND USE DIAGRAMS
M. TRIP GENERATION DIAGRAMS
N. ITE TRIP GENERATION HANDBOOK CH. 7, MULTI-USE DEVELOPMENT
O. INTERNAL CAPTURE DIAGRAM
P. TRAFFIC COUNTS
Q. TRAFFIC SIGNAL WARRANT ANALYSIS REPORTS

## APPENDIX A

## Exhibits

1. Location Exhibits
a. Location Map
b. Aerial Photo
2. Preliminary Site Plan
3. 2016 Existing Traffic
4. 2026 Base Traffic
5. Pass-by Traffic Example
6. Pass-by Traffic
7. Trip Distribution Diagram
8. Preliminary Site Traffic
9. 2026 Total Traffic
10. Lane Configurations
11. Intersection Spacing
12. Queue Length Exhibits
a. Prairie Street \& $14^{\text {th }}$ Street
b. Prairie Street at $7^{\text {th }}$ Street and at $3^{\text {rd }}$ Street


## Exhibit 1A

## Location Map

City of Saint Charles
Prairie Centre
Hampton, Lenzini and Renwick, inc.
Civil \& Structural Engineers . Land Surveyors . Environmental Services


## Exhibit 1B

## Aerial Photo Overview

## City of Saint Charles <br> Prairie Centre




(2) PROPOSED
(1) EXISTING

PROPERTY TO
BE DEVELOPED

STREET

(4) LOOKS LIKE THIS...

(6) SO YOUR NEW TOTAL VOLUME IS... (EXISTING + NON-PASS-BY + PASS-BY) INTERSECTION LIKE THIS (WE'LL JUST DO EAST BOUND IN THIS EXAMPLE)

(3) A PASS-BY TRIP...

(5) PASS-BY TRAFFIC IS ADDED TO THE







## APPENDIX B

> HCS Analysis Reports
> 2016 Existing Traffic
> Weekday AM, Weekday PM, \& Saturday Peak Hours

1. IL Route 38 \& Jewel Driveway
2. IL Route 38 \& East Mall Entrance/Vanderbilt Drive
3. Prairie Street \& Jewel Driveway
4. Prairie Street \& West Mall Entrance
5. Prairie Street \& East Mall Entrance
6. Prairie Street \& Covington Court/Wessel Court
7. Prairie Street $\& 16^{\text {th }}$ Street
8. Prairie Street \& $14^{\text {th }}$ Street
9. Prairie Street \& $7^{\text {th }}$ Street
10. Prairie Street \& $3^{\text {rd }}$ Street
11. $14^{\text {th }}$ Street $\&$ Vanderbilt Drive
12. $14^{\text {th }}$ Street $\&$ Covington Court/Horne Street

## HCS 2010 Two-Way Stop Control Summary Report

General Information


Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 1 |  | 0 | 0 | 1 |  | 0 | 0 | 1 |
| Configuration |  |  | T | TR |  |  | T | R |  |  |  | R |  |  |  | R |
| Volume (veh/h) |  |  | 847 | 21 |  |  | 411 | 19 |  |  |  | 5 |  |  |  | 12 |
| Percent Heavy Vehicles |  |  |  |  |  |  |  |  |  |  |  | 0 |  |  |  | 8 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Generated: 7/21/2016 8:48:22 AM

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR_SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 1 |  | 0 | 0 | 1 |  | 0 | 0 | 1 |
| Configuration |  |  | T | TR |  |  | T | R |  |  |  | R |  |  |  | R |
| Volume (veh/h) |  |  | 597 | 31 |  |  | 914 | 72 |  |  |  | 19 |  |  |  | 58 |
| Percent Heavy Vehicles |  |  |  |  |  |  |  |  |  |  |  | 0 |  |  |  | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Dwy and IL 38 |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | IL 38 |
| North/South Street | Jewel Dwy |
| Peak Hour Factor | 1.00 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West ${ }^{\text {analysis Time Period }}$ |  |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach |  |  | und |  |  |  | und |  |  | Nor |  |  |  | Sou | und |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 0 |  | 0 | 0 | 1 |  | 0 | 0 | 1 |
| Configuration |  |  | T | TR |  |  | T | TR |  |  |  | R |  |  |  | R |
| Volume (veh/h) |  |  | 779 | 46 |  |  | 703 | 63 |  |  |  | 27 |  |  |  | 82 |
| Percent Heavy Vehicles |  |  |  |  |  |  |  |  |  |  |  | 0 |  |  |  | 1 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Dwy and IL 38 |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | IL 38 |
| North/South Street | Jewel Dwy |
| Peak Hour Factor | 0.96 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 7:15-8:15 am | Peak Hour Factor |
| Intersection Orientation | East-West ${ }^{\text {analysis Time Period }}$ |  |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 2 | 0 | 0 | 1 | 2 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  | L | T | TR |  | L | T | TR |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 4 | 826 | 8 |  | 25 | 412 | 3 |  | 10 | 3 | 41 |  | 3 | 1 | 4 |
| Percent Heavy Vehicles |  | 0 |  |  |  | 4 |  |  |  | 10 | 0 | 2 |  | 33 | 100 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Praire Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 2 | 0 | 0 | 1 | 2 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  | L | T | TR |  | L | T | TR |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 29 | 570 | 25 |  | 23 | 944 | 14 |  | 18 | 3 | 32 |  | 11 | 7 | 34 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 0 |  |  |  | 0 | 0 | 3 |  | 0 | 0 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West ${ }^{\text {analysis Time Period }}$ |  |
| Project Description | Prairie Center TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 2 | 0 | 0 | 1 | 2 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  | L | T | TR |  | L | T | TR |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 45 | 751 | 17 |  | 18 | 700 | 21 |  | 18 | 8 | 20 |  | 12 | 5 | 44 |
| Percent Heavy Vehicles |  | 0 |  |  |  | 0 |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 7:15-8:15 am | Peak Hour Factor |
| Intersection Orientation | East-West Analysis Time Period |  |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  | L | T | R |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 19 | 194 | 13 |  | 11 | 215 | 13 |  | 16 | 5 | 11 |  | 8 | 2 | 23 |
| Percent Heavy Vehicles |  | 8 |  |  |  | 0 |  |  |  | 6 | 0 | 0 |  | 0 | 0 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Ent and Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Jewel Ent |
| Peak Hour Factor | 0.97 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | East-West Analysis Time Period |  |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  | L | T | R |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 21 | 216 | 129 |  | 35 | 307 | 20 |  | 76 | 12 | 50 |  | 17 | 7 | 38 |
| Percent Heavy Vehicles |  | 0 |  |  |  | 0 |  |  |  | 0 | 8 | 0 |  | 0 | 0 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Ent and Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Jewel Ent |
| Peak Hour Factor | 0.93 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West Analysis Time Period |  |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  | L | T | R |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 38 | 202 | 126 |  | 30 | 241 | 25 |  | 84 | 7 | 34 |  | 17 | 10 | 54 |
| Percent Heavy Vehicles |  | 0 |  |  |  | 0 |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Ent and Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Jewel Ent |
| Peak Hour Factor | 1.00 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 7:15-8:15 am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L |  | R |  |  | LR |  |  |  |  |  |
| Volume (veh/h) |  |  | 217 | 3 |  | 16 |  | 243 |  | 5 |  | 12 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 6 |  |  |  | 40 |  | 0 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume (veh/h) |  |  | 301 | 2 |  | 33 | 329 |  |  | 11 |  | 43 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 0 |  | 0 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume (veh/h) |  |  | 245 | 10 |  | 26 | 293 |  |  | 4 |  | 42 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 0 |  |  |  | 25 |  | 2 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 7:15-8:15 am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 247 | 10 |  | 16 | 248 |  |  | 9 |  | 15 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 5 |  |  |  | 22 |  | 7 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 335 | 11 |  | 9 | 365 |  |  | 7 |  | 12 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 0 |  |  |  | 0 |  | 8 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 278 | 1 |  | 1 | 322 |  |  | 0 |  | 3 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 0 |  |  |  | 0 |  | 0 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 7:15-8:15 am | Peak Hour Factor |
| Intersection Orientation | East-West Analysis Time Period |  |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 3 | 234 | 5 |  | 10 | 221 | 8 |  | 13 | 0 | 23 |  | 14 | 0 | 5 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 10 |  |  |  | 0 | 0 | 0 |  | 14 | 0 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Covington Ct \& Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Covington Ct |
| Peak Hour Factor | 0.89 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | East-West Analysis Time Period |  |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 8 | 364 | 22 |  | 15 | 361 | 11 |  | 12 | 1 | 12 |  | 6 | 0 | 2 |
| Percent Heavy Vehicles |  | 0 |  |  |  | 0 |  |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Covington Ct \& Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Covington Ct |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West ${ }^{\text {analysis Time Period }}$ |  |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 8 | 321 | 6 |  | 6 | 308 | 5 |  | 21 | 0 | 8 |  | 7 | 0 | 10 |
| Percent Heavy Vehicles |  | 0 |  |  |  | 0 |  |  |  | 5 | 0 | 0 |  | 0 | 0 | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Covington Ct \& Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Covington Ct |
| Peak Hour Factor | 1.00 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 7:15-8:15 am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | LT |  |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume (veh/h) |  | 8 | 275 |  |  |  | 223 | 8 |  |  |  |  |  | 35 |  | 29 |
| Percent Heavy Vehicles |  | 13 |  |  |  |  |  |  |  |  |  |  |  | 3 |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | LT |  |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume (veh/h) |  | 29 | 311 |  |  |  | 345 | 26 |  |  |  |  |  | 38 |  | 37 |
| Percent Heavy Vehicles |  | 0 |  |  |  |  |  |  |  |  |  |  |  | 0 |  | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | LT |  |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume (veh/h) |  | 19 | 318 |  |  |  | 294 | 23 |  |  |  |  |  | 38 |  | 29 |
| Percent Heavy Vehicles |  | 0 |  |  |  |  |  |  |  |  |  |  |  | 0 |  | 0 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 7:15-8:15 am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  | $\downarrow$ b |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 239 | 74 |  | 92 | 194 |  |  | 29 |  | 148 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 0 |  |  |  | 7 |  | 0 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  | $\downarrow$ b |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 251 | 93 |  | 190 | 311 |  |  | 58 |  | 188 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 1 |  |  |  | 2 |  | 0 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  | $\downarrow$ b |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 221 | 79 |  | 137 | 254 |  |  | 55 |  | 146 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 1 |  |  |  | 0 |  | 1 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.2 |  | 0.0 |  | 0.2 |  | 0.3 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.0 |  | 0.2 |  | 0.1 |  | 0.4 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.1 |  | 0.3 |  | 0.1 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.1 |  | 0.1 |  | 0.4 |  | 0.0 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |
| :--- | :---: | :---: | :---: | :--- | :--- | :--- | :--- |
| x, initial | 0.44 |  | 0.26 |  | 0.11 |  | 0.18 |
| hd, final value (s) | 5.71 |  | 6.03 |  | 7.19 |  |  |
| x, final value | 0.78 |  | 0.48 |  | 0.24 |  |  |
| Move-up time, m (s) | 2.0 |  |  |  |  |  |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 3.7 |  | 4.0 |  | 0.36 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 617 |  | 537 |  | 372 |  | 450 |  |
| Delay (s/veh) | 26.09 |  | 14.48 |  | 12.50 |  | 13.22 |  |
| LOS | D |  | B |  | B |  | B |  |
| Approach: Delay (s/veh) | 26.09 |  | 14.48 |  | 12.50 |  | 13.22 |  |
| LOS | D |  | B |  | B |  | B |  |
| Intersection Delay (s/veh) | 19.23 |  |  |  |  |  |  |  |
| Intersection LOS | C |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.0 |  | 0.3 |  | 0.2 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.0 |  | 0.1 |  | 0.2 |  | 0.5 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.0 |  | -0.0 |  | -0.0 |  | -0.2 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $x$, initial | 0.37 |  | 0.42 |  | 0.06 |  | 0.26 |  |  |
| hd, final value (s) | 6.08 |  | 5.93 |  | 7.44 |  | 6.44 |  |  |
| $x$ x, final value | 0.70 |  | 0.79 |  | 0.13 |  | 0.52 |  |  |
| Move-up time, m (s) |  | 2.0 |  | 2.0 |  | 2.0 |  | 2.0 |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 4.1 |  | 3.9 |  | 5.4 |  | 4.4 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 570 |  | 592 |  | 315 |  | 508 |  |
| Delay (s/veh) | 22.18 |  | 27.33 |  | 11.59 |  | 16.18 |  |
| LOS | C |  | D |  | B |  | C |  |
| Approach: Delay (s/veh) | 22.18 |  | 27.33 |  | 11.59 |  | 16.18 |  |
| LOS | C |  | D |  | B |  | C |  |
| Intersection Delay (s/veh) | 22.21 |  |  |  |  |  |  |  |
| Intersection LOS | C |  |  |  |  |  |  |  |

## ALL-WAY STOP CONTROL ANALYSIS

## General Information

| Analyst |
| :--- | :--- |
| Agency/Co. |
| Date Performed |
| Analysis Time Period |


| $H L R-S M$ |
| :--- |
|  |
| $11: 3 / 2016$ |

Site Information

| \|ntersection |
| :--- |
| Jurisdiction |
| Analysis Year |

Project ID Prairie Centre TIS
East/West Street: Prairie St

## Volume Adjustments and Site Characteristics



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.2 |  | 0.0 |  | 0.2 |  | 0.2 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.5 |  |
| Prop. Heavy Vehicle | 0.1 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.2 |  | -0.0 |  | -0.0 |  | -0.2 |  |

## Departure Headway and Service Time



Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 582 |  | 553 |  | 305 |  | 398 |  |
| Delay (s/veh) | 12.56 |  | 11.38 |  | 9.34 |  | 9.88 |  |
| Los | B |  | B |  | A |  | A |  |
| Approach: Delay (s/veh) | 12.56 |  | 11.38 |  | 9.34 |  | 9.88 |  |
| LOS | B |  | B |  | A |  | A |  |
| Intersection Delay (s/veh) | 11.45 |  |  |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.2 |  | 0.1 |  | 0.2 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.2 |  |
| Prop. Heavy Vehicle | 0.1 |  | 0.3 |  | 0.1 |  | 0.2 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.2 |  | 0.5 |  | 0.1 |  | 0.3 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |
| :--- | :---: | :---: | :---: | :--- | :--- | :--- | :--- |
| x, initial | 0.39 |  | 0.22 |  | 0.24 |  | 0.14 |
| hd, final value (s) | 6.28 |  | 7.00 |  | 6.81 |  |  |
| x, final value | 0.77 |  | 0.49 |  | 0.51 |  |  |
| Move-up time, m (s) | 2.0 |  |  |  |  |  |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 4.3 |  | 5.0 |  | 0.32 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 556 |  | 473 |  | 482 |  | 411 |  |
| Delay (s/veh) | 27.34 |  | 16.44 |  | 16.76 |  | 13.72 |  |
| LOS | D |  | C |  | C |  | B |  |
| Approach: Delay (s/veh) | 27.34 |  | 16.44 |  | 16.76 |  | 13.72 |  |
| LOS | D |  | C |  | C |  | B |  |
| Intersection Delay (s/veh) | 20.42 |  |  |  |  |  |  |  |
| Intersection LOS | C |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.1 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.2 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | -0.0 |  | -0.0 |  | -0.0 |  | -0.0 |  |

## Departure Headway and Service Time



Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 474 |  | 506 |  | 400 |  | 436 |  |
| Delay (s/veh) | 25.51 |  | 35.94 |  | 17.06 |  | 21.46 |  |
| LOS | D |  | E |  | C |  | C |  |
| Approach: Delay (s/veh) | 25.51 |  | 35.94 |  | 17.06 |  | 21.46 |  |
| LOS | D |  | E |  | C |  | C |  |
| Intersection Delay (s/veh) | 26.70 |  |  |  |  |  |  |  |
| Intersection LOS | D |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.2 |  | 0.2 |  | 0.1 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.2 |  | 0.0 |  | 0.2 |  | 0.2 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.1 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | -0.1 |  | 0.1 |  | 0.0 |  | -0.1 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |
| :--- | :---: | :---: | :---: | :--- | :--- | :--- | :--- |
| x, initial | 0.26 |  | 0.26 |  | 0.19 |  | 0.18 |
| hd, final value (s) | 5.70 |  | 5.84 |  | 6.04 |  |  |
| x, final value | 0.45 |  | 0.47 |  | 0.37 |  |  |
| Move-up time, m (s) | 2.0 |  |  |  |  |  |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 3.7 |  | 3.8 |  | 0.33 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 537 |  | 537 |  | 469 |  | 450 |  |
| Delay (s/veh) | 13.36 |  | 13.82 |  | 12.50 |  | 11.99 |  |
| LOS | B |  | B |  | B |  | B |  |
| Approach: Delay (s/veh) | 13.36 |  | 13.82 |  | 12.50 |  | 11.99 |  |
| LOS | B |  | B |  | B |  | B |  |
| Intersection Delay (s/veh) | 13.03 |  |  |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |  |  |

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 7:15-8:15am | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 7 | 0 | 2 |  | 0 | 0 | 0 |  | 4 | 177 | 5 |  | 2 | 183 | 15 |
| Percent Heavy Vehicles |  | 0 | 0 | 50 |  | 0 | 0 | 0 |  | 25 |  |  |  | 0 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

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| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 23 | 0 | 38 |  | 0 | 0 | 0 |  | 13 | 287 | 0 |  | 1 | 299 | 34 |
| Percent Heavy Vehicles |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 |  |  |  | 0 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 24 | 0 | 32 |  | 0 | 0 | 0 |  | 15 | 264 | 6 |  | 3 | 277 | 15 |
| Percent Heavy Vehicles |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 |  |  |  | 33 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

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| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 7:15-8:15 am | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Peri |
| Project Description | Praire Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 5 | 1 | 20 |  | 24 | 0 | 4 |  | 7 | 167 | 13 |  | 7 | 151 | 0 |
| Percent Heavy Vehicles |  | 0 | 0 | 5 |  | 4 | 0 | 50 |  | 0 |  |  |  | 29 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 4:30-5:30 pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Praire Centre T Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 3 | 2 | 14 |  | 36 | 2 | 5 |  | 31 | 245 | 36 |  | 5 | 282 | 3 |
| Percent Heavy Vehicles |  | 0 | 0 | 0 |  | 0 | 0 | 20 |  | 0 |  |  |  | 0 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/3/2016 | East/West Street |
| Analysis Year | 2016 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Peri |
| Project Description | Prairie Centre TIS Existing |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 4 | 1 | 17 |  | 32 | 1 | 13 |  | 11 | 192 | 38 |  | 5 | 205 | 6 |
| Percent Heavy Vehicles |  | 0 | 100 | 0 |  | 3 | 0 | 0 |  | 0 |  |  |  | 0 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## APPENDIX C

HCS Analysis Reports
2026 Base Traffic
Weekday AM, Weekday PM, \& Saturday Peak Hours

1. IL Route 38 \& Jewel Driveway
2. IL Route 38 \& East Mall Entrance/Vanderbilt Drive
3. Prairie Street \& Jewel Driveway
4. Prairie Street \& West Mall Entrance
5. Prairie Street \& East Mall Entrance
6. Prairie Street \& Covington Court/Wessel Court
7. Prairie Street \& $16^{\text {th }}$ Street
8. Prairie Street \& $14^{\text {th }}$ Street
9. Prairie Street \& $7^{\text {th }}$ Street
10. Prairie Street \& $3^{\text {rd }}$ Street
11. $14^{\text {th }}$ Street $\&$ Vanderbilt Drive
12. $14^{\text {th }}$ Street $\&$ Covington Court/Horne Street

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  | $\downarrow$ b |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 1 |  | 0 | 0 | 1 |  | 0 | 0 | 1 |
| Configuration |  |  | T | TR |  |  | T | R |  |  |  | R |  |  |  | R |
| Volume (veh/h) |  |  | 947 | 23 |  |  | 466 | 21 |  |  |  | 6 |  |  |  | 13 |
| Percent Heavy Vehicles |  |  |  |  |  |  |  |  |  |  |  | 3 |  |  |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West Analysis Time Period |  |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 1 |  | 0 | 0 | 1 |  | 0 | 0 | 1 |
| Configuration |  |  | T | TR |  |  | T | R |  |  |  | R |  |  |  | R |
| Volume (veh/h) |  |  | 685 | 34 |  |  | 1036 | 79 |  |  |  | 21 |  |  |  | 64 |
| Percent Heavy Vehicles |  |  |  |  |  |  |  |  |  |  |  | 3 |  |  |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Dwy and IL 38 |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | IL 38 |
| North/South Street | Jewel Dwy |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 1 |  | 0 | 0 | 1 |  | 0 | 0 | 1 |
| Configuration |  |  | T | TR |  |  | T | R |  |  |  | R |  |  |  | R |
| Volume (veh/h) |  |  | 877 | 51 |  |  | 794 | 70 |  |  |  | 30 |  |  |  | 91 |
| Percent Heavy Vehicles |  |  |  |  |  |  |  |  |  |  |  | 3 |  |  |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Dwy and IL 38 |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | IL 38 |
| North/South Street | Jewel Dwy |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Period |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 2 | 0 | 0 | 1 | 2 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  | L | T | TR |  | L | T | TR |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 4 | 924 | 9 |  | 28 | 467 | 3 |  | 11 | 3 | 45 |  | 3 | 1 | 4 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 2 | 0 | 0 | 1 | 2 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  | L | T | TR |  | L | T | TR |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 32 | 655 | 28 |  | 25 | 1069 | 15 |  | 20 | 3 | 35 |  | 12 | 8 | 38 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 2 | 0 | 0 | 1 | 2 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  | L | T | TR |  | L | T | TR |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 50 | 846 | 19 |  | 20 | 791 | 23 |  | 20 | 9 | 22 |  | 13 | 6 | 49 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service

| Flow Rate (veh/h) | 54 |  |  |  | 22 |  |  |  |  | 56 |  |  |  | 74 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Capacity | 754 |  |  |  | 718 |  |  |  |  | 96 |  |  |  | 169 |  |
| v/c Ratio | 0.07 |  |  |  | 0.03 |  |  |  |  | 0.58 |  |  |  | 0.44 |  |
| 95\% Queue Length | 0.2 |  |  |  | 0.1 |  |  |  |  | 2.7 |  |  |  | 2.0 |  |
| Control Delay (s/veh) | 10.1 |  |  |  | 10.2 |  |  |  |  | 85.4 |  |  |  | 41.9 |  |
| Level of Service (LOS) | B |  |  |  | B |  |  |  |  | F |  |  |  | E |  |
| Approach Delay (s/veh) |  | 0.6 |  |  |  | 0.2 |  |  |  | 85.4 |  |  |  | 41.9 |  |
| Approach LOS |  |  |  |  |  |  |  |  |  | F |  |  |  | E |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  | L | T | R |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 24 | 244 | 44 |  | 14 | 270 | 16 |  | 20 | 6 | 14 |  | 10 | 3 | 29 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  | L | T | R |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 30 | 225 | 138 |  | 44 | 316 | 29 |  | 85 | 21 | 59 |  | 26 | 16 | 47 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


[^0]Site Information

| Intersection | Jewel Ent and Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Jewel Ent |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  | L | T | R |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 48 | 254 | 158 |  | 38 | 303 | 31 |  | 106 | 9 | 43 |  | 21 | 13 | 68 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Ent and Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Jewel Ent |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume (veh/h) |  |  | 273 | 4 |  | 20 | 305 |  |  | 6 |  | 15 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service

Site Information

| Intersection | West Mall Ent\& Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | West Mall Ent |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West


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HCS 2010™ TWSC Version 6.80 West Mall Ent\&Prairie_PR_AM.xtw

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume (veh/h) |  |  | 378 | 3 |  | 41 | 413 |  |  | 14 |  | 54 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume (veh/h) |  |  | 308 | 13 |  | 33 | 368 |  |  | 5 |  | 53 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 310 | 13 |  | 20 | 311 |  |  | 11 |  | 19 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | East Mall Ent\&Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | East Mall Ent |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Site Information

| Intersection | East Mall Ent\&Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | East Mall Ent |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 421 | 14 |  | 11 | 458 |  |  | 9 |  | 15 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 349 | 1 |  | 1 | 404 |  |  | 0 |  | 4 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 4 | 294 | 6 |  | 13 | 278 | 10 |  | 16 | 0 | 29 |  | 18 | 0 | 6 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Covington Ct \& Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Covington Ct |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 10 | 457 | 28 |  | 19 | 453 | 14 |  | 15 | 1 | 15 |  | 8 | 0 | 3 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Covington Ct \& Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Covington Ct |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 10 | 403 | 8 |  | 8 | 387 | 6 |  | 26 | 0 | 10 |  | 9 | 0 | 13 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Covington Ct \& Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Covington Ct |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | LT |  |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume (veh/h) |  | 10 | 345 |  |  |  | 280 | 11 |  |  |  |  |  | 45 |  | 36 |
| Percent Heavy Vehicles |  | 3 |  |  |  |  |  |  |  |  |  |  |  | 3 |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stre |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Per |
| Project Description | Prairie Centre Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | LT |  |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume (veh/h) |  | 36 | 391 |  |  |  | 433 | 34 |  |  |  |  |  | 49 |  | 46 |
| Percent Heavy Vehicles |  | 3 |  |  |  |  |  |  |  |  |  |  |  | 3 |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stre |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Perior |
| Project Description | Prairie Centre Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | LT |  |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume (veh/h) |  | 24 | 399 |  |  |  | 369 | 30 |  |  |  |  |  | 49 |  | 36 |
| Percent Heavy Vehicles |  | 3 |  |  |  |  |  |  |  |  |  |  |  | 3 |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM1 | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 300 | 94 |  | 121 | 244 |  |  | 33 |  | 170 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 315 | 118 |  | 250 | 391 |  |  | 66 |  | 221 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 278 | 100 |  | 179 | 319 |  |  | 62 |  | 171 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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| ALL-WAY STOP CONTROL ANALYSIS |  |  |  |
| :---: | :---: | :---: | :---: |
| General Information |  | Site Information |  |
| Analyst | HLR-SM | Intersection | $7{ }^{7 \text { th St and Prairie St }}$ |
| Agency/Co. |  | Jurisdiction |  |
| Date Performed | 6/6/2016 | Analysis Year | 2026 Projected |
| Analysis Time Period | 7:15am-8:15am |  |  |

Project ID Prairie Centre TIS
East/West Street: Prairie St
Volume Adjustments and Site Characteristics


Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.2 |  | 0.0 |  | 0.2 |  | 0.3 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.0 |  | 0.2 |  | 0.1 |  | 0.4 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.0 |  | -0.1 |  | -0.0 |  | -0.2 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $x$, initial | 0.49 |  | 0.28 |  | 0.12 |  | 0.20 |  |  |
| hd, final value (s) | 5.86 |  | 6.13 |  | 7.13 |  | 6.69 |  |  |
| $x$ x, final value | 0.89 |  | 0.55 |  | 0.27 |  | 0.41 |  |  |
| Move-up time, m (s) |  | . 0 |  | 2.0 |  | . 0 |  | 2.0 |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 3.9 |  | 4.1 |  | 5.1 |  | 4.7 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 607 |  | 550 |  | 386 |  | 472 |  |
| Delay (s/veh) | 38.79 |  | 16.24 |  | 12.74 |  | 14.31 |  |
| LOS | $E$ |  | C |  | B |  | $B$ |  |
| Approach: Delay (s/veh) | 38.79 |  | 16.24 |  | 12.74 |  | 14.31 |  |
| LOS | E |  | C |  | B |  | B |  |
| Intersection Delay (s/veh) | 25.58 |  |  |  |  |  |  |  |
| Intersection LOS | D |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.0 |  | 0.3 |  | 0.2 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.0 |  | 0.1 |  | 0.2 |  | 0.5 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.0 |  | -0.1 |  | -0.0 |  | -0.3 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $x$, initial | 0.44 |  | 0.50 |  | 0.07 |  | 0.31 |  |  |
| hd, final value (s) | 6.90 |  | 6.76 |  | 8.83 |  | 7.28 |  |  |
| $x$ x, final value | 0.95 |  | 1.06 |  | 0.19 |  | 0.70 |  |  |
| Move-up time, m (s) |  | . 0 |  | 2.0 |  | . 0 |  | 2.0 |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 4.9 |  | 4.8 |  | 6.8 |  | 5.3 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 520 |  | 566 |  | 328 |  | 486 |  |
| Delay (s/veh) | 53.50 |  | 82.49 |  | 13.90 |  | 25.38 |  |
| LOS | $F$ |  | $F$ |  | B |  | D |  |
| Approach: Delay (s/veh) | 53.50 |  | 82.49 |  | 13.90 |  | 25.38 |  |
| LOS | $F$ |  | $F$ |  | B |  | D |  |
| Intersection Delay (s/veh) | 55.94 |  |  |  |  |  |  |  |
| Intersection LOS | $F$ |  |  |  |  |  |  |  |

## ALL-WAY STOP CONTROL ANALYSIS

## General Information

| Analyst |
| :--- |
| Agency/Co. |
| Date Performed |
| Analysis Time Period |


| HLR-SM |
| :--- |
|  |
| 6/6/2016 |
| $11: 15 a \mathrm{am}-12: 15 \mathrm{pm}$ |

Site Information

| \|ntersection |
| :--- |
| Jurisdiction |
| Analysis Year |

7th St and Prairie Centre

2026 Projected

Project ID Prairie Centre TIS
East/West Street: Prairie St

## Volume Adjustments and Site Characteristics



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.2 |  | 0.0 |  | 0.2 |  | 0.2 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.5 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.0 |  | -0.0 |  | -0.1 |  | -0.3 |  |

## Departure Headway and Service Time



Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 656 |  | 631 |  | 320 |  | 436 |  |
| Delay (s/veh) | 16.35 |  | 14.91 |  | 10.29 |  | 11.39 |  |
| Los | C |  | B |  | B |  | B |  |
| Approach: Delay (s/veh) | 16.35 |  | 14.91 |  | 10.29 |  | 11.39 |  |
| LOS | C |  | B |  | B |  | B |  |
| Intersection Delay (s/veh) | 14.56 |  |  |  |  |  |  |  |
| Intersection LOS | B |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.2 |  | 0.1 |  | 0.2 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.2 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | -0.0 |  | -0.0 |  | -0.1 |  | -0.1 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |
| :--- | :---: | :---: | :---: | :--- | :--- | :--- | :--- |
| x, initial | 0.46 |  | 0.26 |  | 0.28 |  | 0.17 |
| hd, final value (s) | 6.58 |  | 7.14 |  | 7.21 |  |  |
| x, final value | 0.94 |  | 0.58 |  | 0.63 |  |  |
| Move-up time, m (s) | 2.0 |  |  |  |  |  |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 4.6 |  | 5.1 |  | 0.40 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 541 |  | 470 |  | 471 |  | 422 |  |
| Delay (s/veh) | 49.72 |  | 19.38 |  | 21.74 |  | 15.67 |  |
| LOS | E |  | C |  | C |  | C |  |
| Approach: Delay (s/veh) | 49.72 |  | 19.38 |  | 21.74 |  | 15.67 |  |
| LOS | E |  | C |  | C |  | C |  |
| Intersection Delay (s/veh) | 31.32 |  |  |  |  |  |  |  |
| Intersection LOS | D |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.1 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.2 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | -0.1 |  | -0.0 |  | -0.1 |  | -0.1 |  |

## Departure Headway and Service Time



Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 451 |  | 549 |  | 370 |  | 402 |  |
| Delay (s/veh) | 103.95 |  | 196.71 |  | 32.58 |  | 59.21 |  |
| LOS | $F$ |  | $F$ |  | D |  | $F$ |  |
| Approach: Delay (s/veh) | 103.95 |  | 196.71 |  | 32.58 |  | 59.21 |  |
| LOS | $F$ |  | $F$ |  | D |  | $F$ |  |
| Intersection Delay (s/veh) | 113.14 |  |  |  |  |  |  |  |
| Intersection LOS | $F$ |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.2 |  | 0.2 |  | 0.1 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.2 |  | 0.1 |  | 0.2 |  | 0.2 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | -0.1 |  | 0.0 |  | -0.1 |  | -0.1 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $x$, initial | 0.31 |  | 0.31 |  | 0.24 |  | 0.22 |  |  |
| hd, final value (s) | 6.54 |  | 6.61 |  | 6.86 |  | 6.95 |  |  |
| $x$ x, final value | 0.64 |  | 0.65 |  | 0.51 |  | 0.47 |  |  |
| Move-up time, m (s) |  | . 0 |  | . 0 |  | . 0 |  | 2.0 |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 4.5 |  | 4.6 |  | 4.9 |  | 4.9 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 512 |  | 507 |  | 463 |  | 450 |  |
| Delay (s/veh) | 20.43 |  | 20.92 |  | 16.85 |  | 16.02 |  |
| Los | C |  | C |  | C |  | C |  |
| Approach: Delay (s/veh) | 20.43 |  | 20.92 |  | 16.85 |  | 16.02 |  |
| LOS | C |  | C |  | C |  | C |  |
| Intersection Delay (s/veh) | 18.89 |  |  |  |  |  |  |  |
| Intersection LOS | C |  |  |  |  |  |  |  |

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 15 | 0 | 2 |  | 0 | 0 | 0 |  | 4 | 198 | 6 |  | 2 | 204 | 24 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 40 | 0 | 42 |  | 0 | 0 | 0 |  | 15 | 321 | 0 |  | 1 | 334 | 52 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 37 | 0 | 36 |  | 0 | 0 | 0 |  | 17 | 295 | 7 |  | 3 | 309 | 26 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 6 | 1 | 22 |  | 28 | 0 | 4 |  | 8 | 193 | 16 |  | 8 | 175 | 0 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Peri |
| Project Description | Prairie Centre TIS Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 3 | 2 | 16 |  | 42 | 2 | 6 |  | 35 | 286 | 42 |  | 6 | 327 | 3 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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HCS 2010™ TWSC Version 6.80 14th\&Covington_PR_PM.xtw

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/6/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre Projected |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 4 | 1 | 19 |  | 37 | 1 | 15 |  | 12 | 223 | 43 |  | 6 | 237 | 7 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## APPENDIX D

HCS Analysis Reports
2026 Total Traffic
Weekday AM, Weekday PM, \& Saturday Peak Hours

1. IL Route 38 \& Jewel Driveway
2. IL Route 38 \& East Mall Entrance/Vanderbilt Drive
3. Prairie Street \& Jewel Driveway
4. Prairie Street \& West Mall Entrance
5. Prairie Street \& East Mall Entrance
6. Prairie Street \& Covington Court/Wessel Court
7. Prairie Street \& $16^{\text {th }}$ Street
8. Prairie Street \& $14^{\text {th }}$ Street
9. Prairie Street \& $7^{\text {th }}$ Street
10. Prairie Street \& $3^{\text {rd }}$ Street
11. $14^{\text {th }}$ Street $\&$ Vanderbilt Drive
12. $14^{\text {th }}$ Street $\&$ Covington Court/Horne Street

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  | $\downarrow$ b |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 1 |  | 0 | 0 | 1 |  | 0 | 0 | 1 |
| Configuration |  |  | T | TR |  |  | T | R |  |  |  | R |  |  |  | R |
| Volume (veh/h) |  |  | 1123 | 23 |  |  | 683 | 21 |  |  |  | 6 |  |  |  | 13 |
| Percent Heavy Vehicles |  |  |  |  |  |  |  |  |  |  |  | 3 |  |  |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Dwy and IL 38 |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | IL 38 |
| North/South Street | Jewel Dwy |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  | $\downarrow$ b |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 1 |  | 0 | 0 | 1 |  | 0 | 0 | 1 |
| Configuration |  |  | T | TR |  |  | T | R |  |  |  | R |  |  |  | R |
| Volume (veh/h) |  |  | 891 | 34 |  |  | 1213 | 79 |  |  |  | 21 |  |  |  | 64 |
| Percent Heavy Vehicles |  |  |  |  |  |  |  |  |  |  |  | 3 |  |  |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 2 | 0 | 0 | 0 | 3 | 1 |  | 0 | 0 | 1 |  | 0 | 0 | 1 |
| Configuration |  |  | T | TR |  |  | T | R |  |  |  | R |  |  |  | R |
| Volume (veh/h) |  |  | 1085 | 51 |  |  | 1003 | 70 |  |  |  | 30 |  |  |  | 91 |
| Percent Heavy Vehicles |  |  |  |  |  |  |  |  |  |  |  | 3 |  |  |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 2 | 0 | 0 | 1 | 2 | 0 |  | 0 | 1 | 0 |  | 1 | 1 | 0 |
| Configuration |  | L | T | TR |  | L | T | TR |  |  | LTR |  |  | L |  | TR |
| Volume (veh/h) |  | 22 | 982 | 9 |  | 28 | 502 | 29 |  | 11 | 3 | 45 |  | 13 | 1 | 38 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Period |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 1 | 2 | 0 | 0 | 1 | 2 | 0 |  | 0 | 1 | 0 |  | 1 | 1 | 0 |
| Configuration |  | L | T | TR |  | L | T | TR |  |  | LTR |  |  | L |  | TR |
| Volume (veh/h) |  | 57 | 698 | 28 |  | 25 | 1108 | 69 |  | 20 | 3 | 35 |  | 17 | 8 | 62 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments


Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  | L | T | R |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 24 | 307 | 44 |  | 14 | 356 | 16 |  | 20 | 6 | 14 |  | 10 | 3 | 29 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Jewel Ent and Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Jewel Ent |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  | L | T | R |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 30 | 329 | 138 |  | 44 | 377 | 29 |  | 85 | 21 | 59 |  | 26 | 16 | 47 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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Site Information

| Intersection | Jewel Ent and Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Jewel Ent |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  | L | T | R |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 48 | 346 | 158 |  | 38 | 377 | 31 |  | 106 | 9 | 43 |  | 21 | 13 | 68 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service

| Flow Rate (veh/h) | 52 |  |  |  | 41 |  |  |  |  | 172 |  |  |  | 111 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Capacity | 1110 |  |  |  | 1015 |  |  |  |  | 185 |  |  |  | 326 |  |
| v/c Ratio | 0.05 |  |  |  | 0.04 |  |  |  |  | 0.93 |  |  |  | 0.34 |  |
| 95\% Queue Length | 0.1 |  |  |  | 0.1 |  |  |  |  | 7.3 |  |  |  | 1.5 |  |
| Control Delay (s/veh) | 8.4 |  |  |  | 8.7 |  |  |  |  | 100.0 |  |  |  | 21.7 |  |
| Level of Service (LOS) | A |  |  |  | A |  |  |  |  | F |  |  |  | c |  |
| Approach Delay (s/veh) |  | 1.3 |  |  |  | 0.7 |  |  |  | 100.0 |  |  |  | 21.7 |  |
| Approach LOS |  |  |  |  |  |  |  |  |  | F |  |  |  | C |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume (veh/h) |  |  | 296 | 44 |  | 20 | 368 |  |  | 29 |  | 39 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume (veh/h) |  |  | 423 | 62 |  | 41 | 456 |  |  | 32 |  | 72 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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HCS 2010™ TWSC Version 6.80 West Mall Ent\&Prairie_T_NI_PM.xtw

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  |  | LR |  |  |  |  |  |
| Volume (veh/h) |  |  | 345 | 68 |  | 33 | 419 |  |  | 28 |  | 76 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 334 | 46 |  | 83 | 311 |  |  | 69 |  | 72 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
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| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 439 | 59 |  | 90 | 458 |  |  | 52 |  | 58 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 372 | 38 |  | 79 | 404 |  |  | 51 |  | 54 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 4 | 371 | 6 |  | 13 | 341 | 10 |  | 16 | 0 | 29 |  | 18 | 0 | 6 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 10 | 518 | 28 |  | 19 | 532 | 14 |  | 15 | 1 | 15 |  | 8 | 0 | 3 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 1 | 0 |  | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 10 | 476 | 8 |  | 8 | 465 | 6 |  | 26 | 0 | 10 |  | 9 | 0 | 13 |
| Percent Heavy Vehicles |  | 3 |  |  |  | 3 |  |  |  | 3 | 3 | 3 |  | 3 | 3 | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Site Information

| Intersection | Covington Ct\& Prairie St |
| :--- | :--- |
| Jurisdiction |  |
| East/West Street | Prairie St |
| North/South Street | Covington Ct |
| Peak Hour Factor | 0.92 |
| Analysis Time Period (hrs) | 0.25 |

Major Street: East-West

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stre |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Per |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | LT |  |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume (veh/h) |  | 15 | 417 |  |  |  | 336 | 11 |  |  |  |  |  | 45 |  | 43 |
| Percent Heavy Vehicles |  | 3 |  |  |  |  |  |  |  |  |  |  |  | 3 |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stre |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Perior |
| Project Description | Prairie Centre Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | LT |  |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume (veh/h) |  | 41 | 447 |  |  |  | 506 | 34 |  |  |  |  |  | 49 |  | 52 |
| Percent Heavy Vehicles |  | 3 |  |  |  |  |  |  |  |  |  |  |  | 3 |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stre |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Perior |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Configuration |  | LT |  |  |  |  |  | TR |  |  |  |  |  |  | LR |  |
| Volume (veh/h) |  | 30 | 466 |  |  |  | 440 | 30 |  |  |  |  |  | 49 |  | 43 |
| Percent Heavy Vehicles |  | 3 |  |  |  |  |  |  |  |  |  |  |  | 3 |  | 3 |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 372 | 94 |  | 121 | 300 |  |  | 33 |  | 170 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 371 | 118 |  | 250 | 464 |  |  | 66 |  | 221 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR_SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | East-West | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |  | 7 | 8 | 9 |  | 10 | 11 | 12 |
| Number of Lanes | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 |  | 1 | 0 | 1 |  | 0 | 0 | 0 |
| Configuration |  |  |  | TR |  | L | T |  |  | L |  | R |  |  |  |  |
| Volume (veh/h) |  |  | 345 | 100 |  | 179 | 390 |  |  | 62 |  | 171 |  |  |  |  |
| Percent Heavy Vehicles |  |  |  |  |  | 3 |  |  |  | 3 |  | 3 |  |  |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Delay, Queue Length, and Level of Service


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Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.2 |  | 0.0 |  | 0.2 |  | 0.2 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.0 |  | 0.2 |  | 0.1 |  | 0.4 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.0 |  | -0.1 |  | -0.0 |  | -0.2 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $x$, initial | 0.55 |  | 0.32 |  | 0.12 |  | 0.21 |  |  |
| hd, final value (s) | 6.16 |  | 6.48 |  | 7.66 |  | 7.07 |  |  |
| $x$ x, final value | 1.06 |  | 0.64 |  | 0.29 |  | 0.47 |  |  |
| Move-up time, m (s) |  | . 0 |  | 2.0 |  | 2.0 |  | 2.0 |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 4.2 |  | 4.5 |  | 5.7 |  | 5.1 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 620 |  | 540 |  | 386 |  | 484 |  |
| Delay (s/veh) | 78.83 |  | 20.45 |  | 13.75 |  | 16.21 |  |
| LOS | $F$ |  | C |  | B |  | C |  |
| Approach: Delay (s/veh) | 78.83 |  | 20.45 |  | 13.75 |  | 16.21 |  |
| LOS | F |  | C |  | B |  | C |  |
| Intersection Delay (s/veh) | 45.76 |  |  |  |  |  |  |  |
| Intersection LOS | E |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.2 |  | 0.0 |  | 0.3 |  | 0.1 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.0 |  | 0.1 |  | 0.2 |  | 0.5 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.0 |  | -0.1 |  | -0.0 |  | -0.3 |  |

## Departure Headway and Service Time



Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 550 |  | 615 |  | 328 |  | 487 |  |
| Delay (s/veh) | 88.95 |  | 129.02 |  | 14.32 |  | 29.55 |  |
| LOS | $F$ |  | $F$ |  | B |  | D |  |
| Approach: Delay (s/veh) | 88.95 |  | 129.02 |  | 14.32 |  | 29.55 |  |
| LOS | $F$ |  | $F$ |  | B |  | D |  |
| Intersection Delay (s/veh) | 87.03 |  |  |  |  |  |  |  |
| Intersection LOS | $F$ |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.2 |  | 0.0 |  | 0.2 |  | 0.2 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.0 |  | 0.1 |  | 0.2 |  | 0.6 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | 0.0 |  | -0.0 |  | -0.1 |  | -0.3 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |
| :--- | :---: | :---: | :---: | :--- | :--- | :--- | :--- |
| x, initial | 0.43 |  | 0.38 |  | 0.06 |  | 0.19 |
| hd, final value (s) | 5.58 |  | 5.63 |  | 6.96 |  |  |
| x, final value | 0.75 |  | 0.67 |  | 0.14 |  |  |
| Move-up time, m (s) | 2.0 |  |  |  |  |  |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 3.6 |  | 3.6 |  | 0.36 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 629 |  | 618 |  | 320 |  | 460 |  |
| Delay (s/veh) | 23.45 |  | 19.25 |  | 11.04 |  | 12.79 |  |
| Los | C |  | C |  | B |  | B |  |
| Approach: Delay (s/veh) | 23.45 |  | 19.25 |  | 11.04 |  | 12.79 |  |
| LOS | C |  | C |  | B |  | B |  |
| Intersection Delay (s/veh) | 19.34 |  |  |  |  |  |  |  |
| Intersection LOS | C |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.1 |  | 0.1 |  | 0.2 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.3 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | -0.0 |  | -0.0 |  | -0.1 |  | -0.1 |  |

## Departure Headway and Service Time



Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 560 |  | 461 |  | 457 |  | 417 |  |
| Delay (s/veh) | 86.17 |  | 22.93 |  | 24.33 |  | 17.53 |  |
| LOS | $F$ |  | C |  | C |  | C |  |
| Approach: Delay (s/veh) | 86.17 |  | 22.93 |  | 24.33 |  | 17.53 |  |
| LOS | $F$ |  | C |  | C |  | C |  |
| Intersection Delay (s/veh) | 47.90 |  |  |  |  |  |  |  |
| Intersection LOS | E |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.1 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.1 |  | 0.1 |  | 0.2 |  | 0.3 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | -0.0 |  | -0.0 |  | -0.1 |  | -0.1 |  |

## Departure Headway and Service Time

| hd, initial value (s) | 3.20 |  | 3.20 |  | 3.20 |  | 3.20 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $x$, initial | 0.43 |  | 0.51 |  | 0.24 |  | 0.36 |  |  |
| hd, final value (s) | 8.99 |  | 9.02 |  | 9.71 |  | 8.92 |  |  |
| $x$ x, final value | 1.22 |  | 1.44 |  | 0.72 |  | 0.99 |  |  |
| Move-up time, m (s) |  | . 0 |  | 2.0 |  | 2.0 |  | 2.0 |  |
| Service Time, $\mathrm{t}_{\mathrm{s}}(\mathrm{s})$ | 7.0 |  | 7.0 |  | 7.7 |  | 6.9 |  |  |

Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 489 |  | 573 |  | 370 |  | 404 |  |
| Delay (s/veh) | 147.64 |  | 234.52 |  | 34.05 |  | 73.15 |  |
| LOS | $F$ |  | $F$ |  | D |  | $F$ |  |
| Approach: Delay (s/veh) | 147.64 |  | 234.52 |  | 34.05 |  | 73.15 |  |
| LOS | $F$ |  | $F$ |  | D |  | $F$ |  |
| Intersection Delay (s/veh) | 141.66 |  |  |  |  |  |  |  |
| Intersection LOS | $F$ |  |  |  |  |  |  |  |



Saturation Headway Adjustment Worksheet

| Prop. Left-Turns | 0.1 |  | 0.2 |  | 0.2 |  | 0.1 |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Prop. Right-Turns | 0.1 |  | 0.0 |  | 0.2 |  | 0.3 |  |
| Prop. Heavy Vehicle | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |
| hLT-adj | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 | 0.2 |
| hRT-adj | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 | -0.6 |
| hHV-adj | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 | 1.7 |
| hadj, computed | -0.1 |  | 0.0 |  | -0.1 |  | -0.1 |  |

## Departure Headway and Service Time



Capacity and Level of Service

|  | Eastbound |  | Westbound |  | Northbound |  | Southbound |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L1 | L2 | L1 | L2 | L1 | L2 | L1 | L2 |
| Capacity (veh/h) | 488 |  | 477 |  | 417 |  | 417 |  |
| Delay (s/veh) | 29.74 |  | 27.91 |  | 19.57 |  | 19.39 |  |
| LOS | D |  | D |  | C |  | C |  |
| Approach: Delay (s/veh) | 29.74 |  | 27.91 |  | 19.57 |  | 19.39 |  |
| LOS | D |  | D |  | C |  | C |  |
| Intersection Delay (s/veh) | 25.01 |  |  |  |  |  |  |  |
| Intersection LOS | D |  |  |  |  |  |  |  |

## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 23 | 0 | 25 |  | 0 | 0 | 0 |  | 4 | 198 | 6 |  | 2 | 204 | 34 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 48 | 0 | 61 |  | 0 | 0 | 0 |  | 15 | 321 | 0 |  | 1 | 334 | 61 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 47 | 0 | 59 |  | 0 | 0 | 0 |  | 17 | 295 | 7 |  | 3 | 309 | 36 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 7:15am-8:15am | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 6 | 1 | 22 |  | 38 | 0 | 4 |  | 8 | 193 | 24 |  | 8 | 175 | 0 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 4:30pm-5:30pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Period |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 L | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 3 | 2 | 16 |  | 51 | 2 | 6 |  | 35 | 286 | 50 |  | 6 | 327 | 3 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## HCS 2010 Two-Way Stop Control Summary Report

General Information

| Analyst | HLR-SM | Intersection |
| :---: | :---: | :---: |
| Agency/Co. |  | Jurisdiction |
| Date Performed | 6/10/2016 | East/West Street |
| Analysis Year | 2026 | North/South Stree |
| Time Analyzed | 11:15am-12:15pm | Peak Hour Factor |
| Intersection Orientation | North-South | Analysis Time Peri |
| Project Description | Prairie Centre TIS Total No Improvements |  |
| Lanes |  |  |
|  |  |  |

Vehicle Volumes and Adjustments

| Approach | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | U | L | T | R | U | L | T | R | U | L | T | R | U | L | T | R |
| Priority |  | 10 | 11 | 12 |  | 7 | 8 | 9 | 1 U | 1 | 2 | 3 | 4 U | 4 | 5 | 6 |
| Number of Lanes |  | 0 | 1 | 0 |  | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 |
| Configuration |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |  |  | LTR |  |
| Volume (veh/h) |  | 4 | 1 | 19 |  | 47 | 1 | 15 |  | 12 | 223 | 53 |  | 6 | 237 | 7 |
| Percent Heavy Vehicles |  | 3 | 3 | 3 |  | 3 | 3 | 3 |  | 3 |  |  |  | 3 |  |  |
| Proportion Time Blocked |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Right Turn Channelized | No |  |  |  | No |  |  |  | No |  |  |  | No |  |  |  |
| Median Type | Undivided |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Median Storage |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

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## APPENDIX E

## Synchro Analysis Reports <br> 2016 Existing Traffic

1. Weekday AM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
2. Weekday PM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
3. Saturday Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street

1：Randall Rd \＆IL Route 38

|  | $\rangle$ |  |  |  |  |  | 4 | 4 | 7 |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊＊ | 个 $\uparrow$ | 「 | \％${ }^{*}$ | 个4 | $\overline{7}$ | \％${ }^{*}$ | 个 $\uparrow$ | 「 | \％${ }^{*}$ | 个个 | F |
| Volume（vph） | 219 | 531 | 35 | 35 | 235 | 147 | 92 | 935 | 33 | 290 | 953 | 240 |
| Satd．Flow（prot） | 3155 | 3471 | 1380 | 3213 | 3343 | 1538 | 3335 | 3438 | 1442 | 3335 | 3406 | 1495 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3155 | 3471 | 1363 | 3210 | 3343 | 1538 | 3334 | 3438 | 1423 | 3333 | 3406 | 1476 |
| Satd．Flow（RTOR） |  |  | 97 |  |  | 62 |  |  | 97 |  |  | 174 |
| Lane Group Flow（vph） | 238 | 577 | 38 | 38 | 255 | 160 | 100 | 1016 | 36 | 315 | 1036 | 261 |
| Turn Type | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | $\mathrm{pm}+\mathrm{ov}$ |
| Protected Phases | 5 | 2 |  | 1 | 6 | 7 | ， | 8 | 1 | 7 | 4 | 5 |
| Permitted Phases |  |  | 2 |  |  | 6 |  |  | 8 |  |  | 4 |
| Total Split（s） | 21.0 | 32.5 | 15.0 | 20.0 | 31.5 | 26.0 | 15.0 | 61.5 | 20.0 | 26.0 | 72.5 | 21.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Effct Green（s） | 14.9 | 33.5 | 44.7 | 7.1 | 23.8 | 48.4 | 9.2 | 61.3 | 70.4 | 18.1 | 70.2 | 87.0 |
| Actuated g／C Ratio | 0.11 | 0.24 | 0.32 | 0.05 | 0.17 | 0.35 | 0.07 | 0.44 | 0.50 | 0.13 | 0.50 | 0.62 |
| v／c Ratio | 0.71 | 0.70 | 0.08 | 0.23 | 0.45 | 0.28 | 0.46 | 0.68 | 0.05 | 0.73 | 0.61 | 0.26 |
| Control Delay | 72.5 | 53.7 | 0.3 | 66.7 | 54.6 | 20.1 | 69.6 | 35.5 | 0.1 | 78.5 | 21.9 | 1.5 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 | 0.0 |
| Total Delay | 72.5 | 53.7 | 0.3 | 66.7 | 54.6 | 20.1 | 69.6 | 35.5 | 0.1 | 78.5 | 22.1 | 1.5 |
| LOS | E | D | A | E | D | C | E | D | A | E | C | A |
| Approach Delay |  | 56.6 |  |  | 43.4 |  |  | 37.4 |  |  | 29.8 |  |
| Approach LOS |  | E |  |  | D |  |  | D |  |  | C |  |
| Queue Length 50th（ft） | 108 | 252 | 0 | 17 | 109 | 61 | 45 | 400 | 0 | 128 | 383 | 7 |
| Queue Length 95th（ft） | 155 | 324 | 0 | 36 | 155 | 115 | 76 | 501 | 0 | m200 | 287 | m4 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（tt） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 371 | 830 | 514 | 355 | 608 | 607 | 250 | 1504 | 845 | 512 | 1706 | 1001 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 152 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.64 | 0.70 | 0.07 | 0.11 | 0.42 | 0.26 | 0.40 | 0.68 | 0.04 | 0.62 | 0.67 | 0.26 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 94 （67\％），Referenced to phase 4：SBT and 8：NBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.73 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 39.1 |  |  |  |  | ntersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization $77.1 \% \quad$ ICU Level of Service DAnalysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| $m$ Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


|  | 4 |  |  | $\checkmark$ |  |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个t |  | \% | 个t |  | \% | F |  | ${ }^{7}$ | $\uparrow$ | F |
| Volume (vph) | 8 | 783 | 67 | 29 | 402 | 1 | 33 | 2 | 48 | 5 | 1 | 1 |
| Satd. Flow (prot) | 1444 | 3410 | 0 | 1805 | 3367 | 0 | 1752 | 1596 | 0 | 1805 | 1900 | 1615 |
| Flt Permitted | 0.505 |  |  | 0.298 |  |  | 0.757 |  |  | 0.722 |  |  |
| Satd. Flow (perm) | 768 | 3410 | 0 | 566 | 3367 | 0 | 1396 | 1596 | 0 | 1372 | 1900 | 1615 |
| Satd. Flow (RTOR) |  | 12 |  |  |  |  |  | 51 |  |  |  | 64 |
| Lane Group Flow (vph) | 8 | 895 | 0 | 31 | 424 | 0 | 35 | 53 | 0 | 5 | 1 | 1 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | Perm | NA |  | Perm | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 13.0 | 72.0 |  | 13.0 | 72.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 | 35.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |
| Act Effct Green (s) | 100.4 | 95.3 |  | 102.1 | 99.2 |  | 10.6 | 10.6 |  | 10.6 | 10.6 | 10.6 |
| Actuated g/C Ratio | 0.84 | 0.79 |  | 0.85 | 0.83 |  | 0.09 | 0.09 |  | 0.09 | 0.09 | 0.09 |
| v/c Ratio | 0.01 | 0.33 |  | 0.06 | 0.15 |  | 0.29 | 0.28 |  | 0.04 | 0.01 | 0.01 |
| Control Delay | 2.1 | 5.3 |  | 0.8 | 1.5 |  | 56.3 | 18.2 |  | 49.2 | 48.0 | 0.0 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 2.1 | 5.3 |  | 0.8 | 1.5 |  | 56.3 | 18.2 |  | 49.2 | 48.0 | 0.0 |
| LOS | A | A |  | A | A |  | E | B |  | D | D | A |
| Approach Delay |  | 5.3 |  |  | 1.4 |  |  | 33.4 |  |  | 42.0 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | D |  |
| Queue Length 50th (ft) | 1 | 113 |  | 1 | 5 |  | 26 | 1 |  | 4 | 1 | 0 |
| Queue Length 95th (ft) | 4 | 165 |  | 2 | 71 |  | 58 | 40 |  | 16 | 6 | 0 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length (ft) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 710 | 2710 |  | 582 | 2782 |  | 337 | 424 |  | 331 | 459 | 438 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 |  |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.01 | 0.33 |  | 0.05 | 0.15 |  | 0.10 | 0.13 |  | 0.02 | 0.00 | 0.00 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $95(79 \%)$, Referenced to phase 2:EBTL and $6: W B T L$, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.33
Intersection Signal Delay: $6.0 \quad$ Intersection LOS: A
Intersection Capacity Utilization 43.0\% ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个t |  | 7 | 个 ${ }^{\text {a }}$ |  | 7 | F |  | 7 | F |  |
| Volume (vph) | 66 | 809 | 7 | 86 | 389 | 43 | 12 | 76 | 198 | 69 | 88 | 30 |
| Satd. Flow (prot) | 1770 | 3428 | 0 | 1703 | 3304 | 0 | 1671 | 1671 | 0 | 1787 | 1801 | 0 |
| Flt Permitted | 0.490 |  |  | 0.253 |  |  | 0.679 |  |  | 0.312 |  |  |
| Satd. Flow (perm) | 912 | 3428 | 0 | 453 | 3304 | 0 | 1195 | 1671 | 0 | 587 | 1801 | 0 |
| Satd. Flow (RTOR) |  | 1 |  |  | 14 |  |  | 96 |  |  | 13 |  |
| Lane Group Flow (vph) | 67 | 833 | 0 | 88 | 441 | 0 | 12 | 280 | 0 | 70 | 121 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 65.0 |  | 13.0 | 65.0 |  | 13.0 | 29.0 |  | 13.0 | 29.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Efft Green (s) | 70.4 | 60.2 |  | 72.2 | 62.6 |  | 33.7 | 26.1 |  | 38.2 | 31.9 |  |
| Actuated g/C Ratio | 0.59 | 0.50 |  | 0.60 | 0.52 |  | 0.28 | 0.22 |  | 0.32 | 0.27 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.11 | 0.48 |  | 0.25 | 0.26 |  | 0.03 | 0.64 |  | 0.26 | 0.25 |  |
| Control Delay | 16.4 | 32.6 |  | 10.9 | 16.4 |  | 28.4 | 36.2 |  | 31.5 | 34.2 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 16.4 | 32.6 |  | 10.9 | 16.4 |  | 28.4 | 36.2 |  | 31.5 | 34.2 |  |
| LOS | B | C |  | B | B |  | C | D |  | C | C |  |
| Approach Delay |  | 31.4 |  |  | 15.5 |  |  | 35.9 |  |  | 33.2 |  |
| Approach LOS |  | C |  |  | B |  |  | D |  |  | C |  |
| Queue Length 50th (ft) | 26 | 306 |  | 26 | 95 |  | 6 | 134 |  | 38 | 62 |  |
| Queue Length 95th (ft) | 64 | 405 |  | 47 | 133 |  | 21 | 235 |  | 74 | 128 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 620 | 1719 |  | 375 | 1729 |  | 398 | 438 |  | 281 | 487 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.11 | 0.48 |  | 0.23 | 0.26 |  | 0.03 | 0.64 |  | 0.25 | 0.25 |  |

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: 43 ( $36 \%$ ), Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.64
Intersection Signal Delay: 27.9 Intersection LOS: C
Intersection Capacity Utilization 65.7\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 3: 14th St \& IL Route 38


|  | 4 |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | f |  | \％ | $\uparrow$ | 「 | \％ | 个 ${ }^{\text {a }}$ |  | ${ }^{7}$ | 个 ${ }^{\text {a }}$ |  |
| Volume（vph） | 3 | 5 | 15 | 115 | 9 | 120 | 15 | 1186 | 120 | 123 | 1347 | 11 |
| Satd．Flow（prot） | 1805 | 1582 | 0 | 1687 | 1900 | 1524 | 1805 | 3347 | 0 | 1703 | 3399 | 0 |
| Flt Permitted | 0.751 |  |  | 0.431 |  |  | 0.153 |  |  | 0.143 |  |  |
| Satd．Flow（perm） | 1410 | 1582 | 0 | 764 | 1900 | 1487 | 291 | 3347 | 0 | 256 | 3399 | 0 |
| Satd．Flow（RTOR） |  | 16 |  |  |  | 128 |  | 13 |  |  | 1 |  |
| Lane Group Flow（vph） | 3 | 21 | 0 | 122 | 10 | 128 | 16 | 1390 | 0 | 131 | 1445 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | pm＋ov | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 26.5 |  | 13.0 | 26.5 | 13.0 | 13.0 | 87.5 |  | 13.0 | 87.5 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 11.6 | 7.5 |  | 17.6 | 13.3 | 23.1 | 108.2 | 100.5 |  | 113.8 | 107.9 |  |
| Actuated g／C Ratio | 0.08 | 0.05 |  | 0.13 | 0.10 | 0.16 | 0.77 | 0.72 |  | 0.81 | 0.77 |  |
| v／c Ratio | 0.02 | 0.21 |  | 0.79 | 0.06 | 0.36 | 0.06 | 0.58 |  | 0.46 | 0.55 |  |
| Control Delay | 48.7 | 36.1 |  | 88.5 | 56.1 | 10.2 | 3.2 | 6.6 |  | 8.7 | 9.0 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |  | 0.0 | 0.0 |  |
| Total Delay | 48.7 | 36.1 |  | 88.5 | 56.1 | 10.2 | 3.2 | 6.7 |  | 8.7 | 9.0 |  |
| LOS | D | D |  | F | E | B | A | A |  | A | A |  |
| Approach Delay |  | 37.7 |  |  | 48.7 |  |  | 6.6 |  |  | 9.0 |  |
| Approach LOS |  | D |  |  | D |  |  | A |  |  | A |  |
| Queue Length 50th（ft） | 2 | 4 |  | 102 | 8 | 0 | 2 | 112 |  | 24 | 247 |  |
| Queue Length 95th（ft） | 12 | 33 |  | \＃167 | 27 | 56 | m4 | 152 |  | 43 | 426 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ft） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 180 | 239 |  | 155 | 273 | 371 | 329 | 2406 |  | 301 | 2619 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 170 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.02 | 0.09 |  | 0.79 | 0.04 | 0.35 | 0.05 | 0.62 |  | 0.44 | 0.55 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 101 （72\％），Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.79 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 11.3 |  |  |  |  | Intersection LOS：B |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 73．1\％ |  |  |  |  | ICU Level of Service D |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| $m$ Volume for 95th perc | equeue | metere | by ups | eam sig |  |  |  |  |  |  |  |  |

Splits and Phases：11：Randall Rd \＆Prairie St


Prairie Centre 7：15 am 4／12／2016 Existing 2016

1：Randall Rd \＆IL Route 38

|  |  |  |  |  |  |  | 4 | 4 | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊＊ | 个个 | ${ }^{7}$ | ${ }^{1 *}$ | 个个 | 「 | ${ }^{1 *}$ | 个个 | 「 | ${ }^{1+1}$ | 个个 | F |
| Volume（vph） | 270 | 326 | 79 | 125 | 522 | 344 | 104 | 1152 | 51 | 240 | 1158 | 273 |
| Satd．Flow（prot） | 3367 | 3539 | 1583 | 3467 | 3539 | 1568 | 3367 | 3539 | 1583 | 3433 | 3505 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3365 | 3539 | 1583 | 3467 | 3539 | 1548 | 3367 | 3539 | 1583 | 3433 | 3505 | 1583 |
| Satd．Flow（RTOR） |  |  | 62 |  |  | 62 |  |  | 62 |  |  | 100 |
| Lane Group Flow（vph） | 281 | 340 | 82 | 130 | 544 | 358 | 108 | 1200 | 53 | 250 | 1206 | 284 |
| Turn Type | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov |
| Protected Phases | 5 | 2 | 3 | ， | 6 | 7 |  | ， | ， | 7 | 4 | 5 |
| Permitted Phases |  |  | 2 |  |  | 6 |  |  | 8 |  |  | 4 |
| Total Split（s） | 17.0 | 31.5 | 18.0 | 17.0 | 31.5 | 18.0 | 18.0 | 73.5 | 17.0 | 18.0 | 73.5 | 17.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Effct Green（s） | 12.5 | 27.1 | 43.5 | 10.4 | 25.0 | 40.1 | 9.9 | 67.4 | 84.3 | 13.1 | 70.6 | 89.6 |
| Actuated g／C Ratio | 0.09 | 0.19 | 0.31 | 0.07 | 0.18 | 0.29 | 0.07 | 0.48 | 0.60 | 0.09 | 0.50 | 0.64 |
| v／c Ratio | 0.94 | 0.50 | 0.15 | 0.51 | 0.86 | 0.73 | 0.46 | 0.70 | 0.05 | 0.78 | 0.68 | 0.27 |
| Control Delay | 100.8 | 53.7 | 12.6 | 69.0 | 70.4 | 44.0 | 68.3 | 31.4 | 2.0 | 87.1 | 22.1 | 4.4 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.4 | 0.0 |
| Total Delay | 100.8 | 53.7 | 12.6 | 69.0 | 70.4 | 44.0 | 68.3 | 31.4 | 2.0 | 87.1 | 22.5 | 4.4 |
| LOS | F | D | B | E | E | D | E | C | A | F | C | A |
| Approach Delay |  | 67.7 |  |  | 61.1 |  |  | 33.2 |  |  | 28.8 |  |
| Approach LOS |  | E |  |  | E |  |  | C |  |  | C |  |
| Queue Length 50th（ft） | 133 | 147 | 13 | 59 | 255 | 234 | 49 | 444 | 0 | 106 | 460 | 35 |
| Queue Length 95th（ft） | \＃222 | 203 | 53 | 93 | \＃347 | 350 | 80 | 528 | 14 | m\＃164 | m455 | m31 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ t ） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 300 | 684 | 573 | 309 | 631 | 493 | 324 | 1703 | 1000 | 331 | 1768 | 1049 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 173 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 0 |  | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.94 | 0.50 | 0.14 | 0.42 | 0.86 | 0.73 | 0.33 | 0.71 | 0.05 | 0.76 | 0.76 | 0.27 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 12 （9\％），Referenced to phase 4：SBT and 8：NBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.94 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 42.6 |  |  |  |  | Intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 79．9\％ |  |  |  |  | ICU Level of Service D |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


Prairie Centre 4：30 pm 4／12／2016 Existing 2016

|  | $\rangle$ |  |  | $\dagger$ |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个 ${ }^{\text {a }}$ |  | \% | 个t |  | \% | F |  | ${ }^{7}$ | $\uparrow$ | F |
| Volume (vph) | 19 | 552 | 46 | 57 | 936 | 8 | 48 | 2 | 60 | 20 | 4 | 7 |
| Satd. Flow (prot) | 1805 | 3506 | 0 | 1805 | 3536 | 0 | 1805 | 1623 | 0 | 1719 | 1900 | 1417 |
| Flt Permitted | 0.274 |  |  | 0.392 |  |  | 0.755 |  |  | 0.713 |  |  |
| Satd. Flow (perm) | 520 | 3506 | 0 | 745 | 3536 | 0 | 1433 | 1623 | 0 | 1290 | 1900 | 1398 |
| Satd. Flow (RTOR) |  | 11 |  |  | 1 |  |  | 65 |  |  |  | 64 |
| Lane Group Flow (vph) | 20 | 643 | 0 | 61 | 1015 | 0 | 52 | 67 | 0 | 22 | 4 | 8 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | Perm | NA |  | Perm | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 13.0 | 72.0 |  | 13.0 | 72.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 | 35.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |
| Act Effct Green (s) | 98.2 | 91.9 |  | 100.5 | 96.0 |  | 11.7 | 11.7 |  | 11.7 | 11.7 | 11.7 |
| Actuated g/C Ratio | 0.82 | 0.77 |  | 0.84 | 0.80 |  | 0.10 | 0.10 |  | 0.10 | 0.10 | 0.10 |
| v/c Ratio | 0.04 | 0.24 |  | 0.09 | 0.36 |  | 0.37 | 0.31 |  | 0.17 | 0.02 | 0.04 |
| Control Delay | 2.6 | 5.8 |  | 1.0 | 2.3 |  | 57.2 | 16.0 |  | 51.4 | 46.8 | 0.4 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 2.6 | 5.8 |  | 1.0 | 2.3 |  | 57.2 | 16.0 |  | 51.4 | 46.8 | 0.4 |
| LOS | A | A |  | A | A |  | E | B |  | D | D | A |
| Approach Delay |  | 5.7 |  |  | 2.2 |  |  | 34.0 |  |  | 38.8 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | D |  |
| Queue Length 50th (ft) | 2 | 78 |  | 2 | 21 |  | 38 | 1 |  | 16 | 3 | 0 |
| Queue Length 95th (ft) | 7 | 120 |  | m5 | 39 |  | 77 | 44 |  | 41 | 14 | 0 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length (ft) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 539 | 2687 |  | 714 | 2830 |  | 346 | 441 |  | 311 | 459 | 386 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.04 | 0.24 |  | 0.09 | 0.36 |  | 0.15 | 0.15 |  | 0.07 | 0.01 | 0.02 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $93(78 \%)$, Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.37
Intersection Signal Delay: 6.1 Intersection LOS: A
Intersection Capacity Utilization $55.5 \% \quad$ ICU Level of Service B
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{4}$ | 性 |  | \% | 个 ${ }^{\text {P }}$ |  | ${ }^{7}$ | F |  | \% | $\hat{\beta}$ |  |
| Volume (vph) | 93 | 560 | 14 | 218 | 859 | 73 | 40 | 153 | 185 | 116 | 186 | 50 |
| Satd. Flow (prot) | 1805 | 3527 | 0 | 1805 | 3520 | 0 | 1805 | 1744 | 0 | 1805 | 1826 | 0 |
| Flt Permitted | 0.218 |  |  | 0.373 |  |  | 0.461 |  |  | 0.150 |  |  |
| Satd. Flow (perm) | 414 | 3527 | 0 | 709 | 3520 | 0 | 875 | 1744 | 0 | 285 | 1826 | 0 |
| Satd. Flow (RTOR) |  | 3 |  |  | 10 |  |  | 45 |  |  | 10 |  |
| Lane Group Flow (vph) | 96 | 591 | 0 | 225 | 961 | 0 | 41 | 349 | 0 | 120 | 244 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 65.0 |  | 13.0 | 65.0 |  | 13.0 | 29.0 |  | 13.0 | 29.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Effct Green (s) | 69.8 | 58.9 |  | 72.6 | 60.3 |  | 33.0 | 23.1 |  | 36.8 | 26.7 |  |
| Actuated g/C Ratio | 0.58 | 0.49 |  | 0.60 | 0.50 |  | 0.28 | 0.19 |  | 0.31 | 0.22 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.29 | 0.34 |  | 0.44 | 0.54 |  | 0.14 | 0.94 |  | 0.59 | 0.59 |  |
| Control Delay | 15.3 | 25.3 |  | 12.8 | 21.8 |  | 29.7 | 75.8 |  | 42.3 | 47.9 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 15.3 | 25.3 |  | 12.8 | 21.8 |  | 29.7 | 75.8 |  | 42.3 | 47.9 |  |
| LOS | B | C |  | B | C |  | C | E |  | D | D |  |
| Approach Delay |  | 23.9 |  |  | 20.1 |  |  | 71.0 |  |  | 46.1 |  |
| Approach LOS |  | C |  |  | C |  |  | E |  |  | D |  |
| Queue Length 50th (ft) | 27 | 149 |  | 71 | 257 |  | 22 | 238 |  | 67 | 166 |  |
| Queue Length 95th (ft) | 76 | 258 |  | 109 | 328 |  | 49 | \#423 |  | 116 | 263 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 356 | 1731 |  | 516 | 1773 |  | 329 | 371 |  | 207 | 414 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.27 | 0.34 |  | 0.44 | 0.54 |  | 0.12 | 0.94 |  | 0.58 | 0.59 |  |

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: 47 ( $39 \%$ ), Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated

## Maximum v/c Ratio: 0.94

Intersection Signal Delay: 32.3
Intersection LOS: C
Intersection Capacity Utilization 74.1\%
ICU Level of Service D
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 3: 14th St \& IL Route 38


|  | 4 | $\rightarrow$ |  |  |  |  |  | $\dagger$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% |  |  | \% | $\uparrow$ | F | \% | 个t |  | \% | 中 ${ }^{\text {d }}$ |  |
| Volume (vph) | 56 | 27 | 89 | 192 | 30 | 198 | 64 | 1493 | 185 | 155 | 1404 | 33 |
| Satd. Flow (prot) | 1805 | 1662 | 0 | 1805 | 1900 | 1599 | 1805 | 3456 | 0 | 1770 | 3497 | 0 |
| FIt Permitted | 0.687 |  |  | 0.565 |  |  | 0.126 |  |  | 0.064 |  |  |
| Satd. Flow (perm) | 1305 | 1662 | 0 | 1071 | 1900 | 1599 | 239 | 3456 | 0 | 119 | 3497 | 0 |
| Satd. Flow (RTOR) |  | 92 |  |  |  | 99 |  | 16 |  |  | 3 |  |
| Lane Group Flow (vph) | 58 | 120 | 0 | 198 | 31 | 204 | 66 | 1730 | 0 | 160 | 1481 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA | pm+ov | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split (s) | 13.0 | 26.5 |  | 13.0 | 26.5 | 13.0 | 13.0 | 87.5 |  | 13.0 | 87.5 |  |
| Total Lost Time (s) | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green (s) | 21.2 | 9.7 |  | 20.2 | 12.5 | 25.2 | 100.1 | 91.3 |  | 105.6 | 95.9 |  |
| Actuated g/C Ratio | 0.15 | 0.07 |  | 0.14 | 0.09 | 0.18 | 0.72 | 0.65 |  | 0.75 | 0.68 |  |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.25 | 0.60 |  | 0.99 | 0.18 | 0.55 | 0.27 | 0.77 |  | 0.82 | 0.62 |  |
| Control Delay | 50.2 | 31.2 |  | 115.3 | 62.3 | 32.4 | 5.8 | 13.3 |  | 54.1 | 14.4 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 |  | 0.0 | 0.0 |  |
| Total Delay | 50.2 | 31.2 |  | 115.3 | 62.3 | 32.4 | 5.8 | 13.3 |  | 54.1 | 14.5 |  |
| LOS | D | C |  | F | E | C | A | B |  | D | B |  |
| Approach Delay |  | 37.4 |  |  | 72.5 |  |  | 13.1 |  |  | 18.3 |  |
| Approach LOS |  | D |  |  | E |  |  | B |  |  | B |  |
| Queue Length 50th (ft) | 46 | 25 |  | 169 | 27 | 87 | 12 | 310 |  | 64 | 366 |  |
| Queue Length 95th (ft) | 84 | 87 |  | \#278 | 59 | 164 | m18 | m351 |  | \#199 | 516 |  |
| Internal Link Dist (ft) |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length ( t ) | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity (vph) | 246 | 316 |  | 201 | 271 | 369 | 276 | 2260 |  | 196 | 2395 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 29 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 1 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 39 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.24 | 0.38 |  | 0.99 | 0.11 | 0.55 | 0.24 | 0.78 |  | 0.82 | 0.63 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset: 1 (1\%), Referenced to phase 2:NBTL and 6:SBTL, Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.99 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay: 22.6 |  |  |  |  | Intersection LOS: C |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 87.2\% |  |  |  |  | ICU Level of Service E |  |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer. |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles. |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95th per | queue | meter | by ups | eam sig |  |  |  |  |  |  |  |  |

Splits and Phases: 11: Randall Rd \& Prairie St


Prairie Centre 4:30 pm 4/12/2016 Existing 2016

1：Randall Rd \＆IL Route 38

|  | 4 |  |  |  |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊＊ | 个个 | 「 | 7\％ | 个4 | F | \％${ }^{1}$ | 个个 | 「 | \％${ }^{1 / 1}$ | 个 $\uparrow$ | ¢ |
| Volume（vph） | 357 | 417 | 107 | 176 | 310 | 291 | 109 | 1154 | 84 | 298 | 1417 | 283 |
| Satd．Flow（prot） | 3467 | 3539 | 1568 | 3502 | 3539 | 1583 | 3467 | 3539 | 1599 | 3502 | 3539 | 1599 |
| FIt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3467 | 3539 | 1548 | 3499 | 3539 | 1583 | 3466 | 3539 | 1599 | 3502 | 3539 | 1579 |
| Satd．Flow（RTOR） |  |  | 73 |  |  | 73 |  |  | 73 |  |  | 199 |
| Lane Group Flow（vph） | 368 | 430 | 110 | 181 | 320 | 300 | 112 | 1190 | 87 | 307 | 1461 | 292 |
| Turn Type | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | $\mathrm{pm}+\mathrm{ov}$ |
| Protected Phases | 5 | 2 | 3 | 1 | 6 | 7 | 3 | 8 | 1 | 7 | 4 | 5 |
| Permitted Phases |  |  | 2 |  |  | 6 |  |  | 8 |  |  | 4 |
| Total Split（s） | 17.0 | 31.5 | 18.0 | 17.0 | 31.5 | 18.0 | 18.0 | 53.5 | 17.0 | 18.0 | 53.5 | 17.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Effct Green（s） | 12.5 | 24.1 | 35.4 | 11.0 | 22.7 | 42.5 | 9.2 | 49.5 | 67.0 | 13.4 | 53.6 | 68.1 |
| Actuated g／C Ratio | 0.10 | 0.20 | 0.30 | 0.09 | 0.19 | 0.35 | 0.08 | 0.41 | 0.56 | 0.11 | 0.45 | 0.57 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 1.02 | 0.60 | 0.22 | 0.56 | 0.48 | 0.49 | 0.42 | 0.82 | 0.09 | 0.79 | 0.92 | 0.30 |
| Control Delay | 105.4 | 47.4 | 11.7 | 59.0 | 45.5 | 25.0 | 57.3 | 37.4 | 4.1 | 76.1 | 32.0 | 1.3 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 4.1 | 0.0 | 0.0 | 1.0 | 0.0 |
| Total Delay | 105.4 | 47.4 | 11.7 | 59.0 | 45.5 | 25.0 | 57.3 | 41.6 | 4.1 | 76.1 | 33.0 | 1.3 |
| LOS | F | D | B | E | D | C | E | D | A | E | C | A |
| Approach Delay |  | 66.6 |  |  | 40.9 |  |  | 40.5 |  |  | 35.0 |  |
| Approach LOS |  | E |  |  | D |  |  | D |  |  | C |  |
| Queue Length 50th（ft） | ～155 | 157 | 19 | 70 | 114 | 130 | 43 | 438 | 5 | 131 | 438 | 1 |
| Queue Length 95th（ft） | \＃254 | 214 | 59 | 107 | 160 | 214 | 72 | 535 | 28 | m163 | \＃756 | m5 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ t ） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 361 | 743 | 562 | 364 | 737 | 612 | 390 | 1459 | 943 | 398 | 1581 | 984 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | ， | 0 | 0 | 0 | O | 30 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 196 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | ， | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 1.02 | 0.58 | 0.20 | 0.50 | 0.43 | 0.49 | 0.29 | 0.94 | 0.09 | 0.77 | 0.94 | 0.30 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 0 （0\％），Referenced to phase 4：SBT and 8：NBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 1.02 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 42.9 |  |  |  |  | Intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 83．5\％ |  |  |  |  | ICU Level of Service E |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| ～Volume exceeds capacity，queue is theoretically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


|  | 4 |  |  | $\checkmark$ |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个t |  | \% | 个t |  | \% | $\hat{F}$ |  | ${ }^{7}$ | $\uparrow$ | F |
| Volume (vph) | 18 | 634 | 59 | 35 | 671 | 6 | 48 | 1 | 63 | 18 | 7 | 2 |
| Satd. Flow (prot) | 1805 | 3499 | 0 | 1805 | 3531 | 0 | 1770 | 1619 | 0 | 1805 | 1900 | 1615 |
| FIt Permitted | 0.375 |  |  | 0.354 |  |  | 0.753 |  |  | 0.713 |  |  |
| Satd. Flow (perm) | 712 | 3499 | 0 | 673 | 3531 | 0 | 1403 | 1619 | 0 | 1355 | 1900 | 1615 |
| Satd. Flow (RTOR) |  | 13 |  |  | 1 |  |  | 67 |  |  |  | 94 |
| Lane Group Flow (vph) | 19 | 737 | 0 | 37 | 720 | 0 | 51 | 68 | 0 | 19 | 7 | 2 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | Perm | NA |  | Perm | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 18.0 | 57.0 |  | 13.0 | 52.0 |  | 35.0 | 35.0 |  | 35.0 | 35.0 | 35.0 |
| Total Lost Time (s) | 4.0 | 4.0 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 |
| Act Effct Green (s) | 87.1 | 84.4 |  | 88.2 | 86.5 |  | 9.3 | 9.3 |  | 9.3 | 9.3 | 9.3 |
| Actuated g/C Ratio | 0.83 | 0.80 |  | 0.84 | 0.82 |  | 0.09 | 0.09 |  | 0.09 | 0.09 | 0.09 |
| v/c Ratio | 0.03 | 0.26 |  | 0.06 | 0.25 |  | 0.41 | 0.33 |  | 0.16 | 0.04 | 0.01 |
| Control Delay | 2.1 | 4.1 |  | 0.9 | 1.1 |  | 54.4 | 15.5 |  | 45.7 | 42.1 | 0.0 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 2.1 | 4.1 |  | 0.9 | 1.1 |  | 54.4 | 15.5 |  | 45.7 | 42.1 | 0.0 |
| LOS | A | A |  | A | A |  | D | B |  | D | D | A |
| Approach Delay |  | 4.1 |  |  | 1.1 |  |  | 32.2 |  |  | 41.5 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | D |  |
| Queue Length 50th (ft) | 2 | 70 |  | 1 | 10 |  | 33 | 1 |  | 12 | 4 | 0 |
| Queue Length 95th (ft) | 6 | 111 |  | m3 | 26 |  | 70 | 40 |  | 34 | 18 | 0 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length (ft) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 753 | 2813 |  | 669 | 2907 |  | 414 | 525 |  | 400 | 560 | 543 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.03 | 0.26 |  | 0.06 | 0.25 |  | 0.12 | 0.13 |  | 0.05 | 0.01 | 0.00 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 105

Actuated Cycle Length: 105
Offset: 87 (83\%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.41
Intersection Signal Delay: $5.3 \quad$ Intersection LOS: A
Intersection Capacity Utilization 42.1\% ICU Level of Service A
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.

Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个 ${ }^{\text {a }}$ |  | \% | 个 ${ }^{\text {a }}$ |  | * | ¢ |  | \% | F |  |
| Volume (vph) | 68 | 625 | 25 | 243 | 626 | 54 | 36 | 116 | 227 | 97 | 149 | 29 |
| Satd. Flow (prot) | 1770 | 3518 | 0 | 1770 | 3496 | 0 | 1805 | 1377 | 0 | 1805 | 1820 | 0 |
| Flt Permitted | 0.322 |  |  | 0.271 |  |  | 0.637 |  |  | 0.197 |  |  |
| Satd. Flow (perm) | 599 | 3518 | 0 | 505 | 3496 | 0 | 1209 | 1377 | 0 | 330 | 1820 | 0 |
| Satd. Flow (RTOR) |  | 5 |  |  | 10 |  |  | 86 |  |  | 9 |  |
| Lane Group Flow (vph) | 73 | 699 | 0 | 261 | 731 | 0 | 39 | 369 | 0 | 104 | 191 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | O |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 14.0 | 49.0 |  | 14.0 | 49.0 |  | 13.0 | 29.0 |  | 13.0 | 29.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Effct Green (s) | 53.2 | 42.7 |  | 58.7 | 47.4 |  | 33.1 | 23.6 |  | 37.4 | 29.2 |  |
| Actuated g/C Ratio | 0.51 | 0.41 |  | 0.56 | 0.45 |  | 0.32 | 0.22 |  | 0.36 | 0.28 |  |
| v/c Ratio | 0.19 | 0.49 |  | 0.64 | 0.46 |  | 0.09 | 0.98 |  | 0.43 | 0.37 |  |
| Control Delay | 17.2 | 32.7 |  | 19.9 | 21.6 |  | 22.4 | 75.1 |  | 28.2 | 33.3 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 17.2 | 32.7 |  | 19.9 | 21.6 |  | 22.4 | 75.1 |  | 28.2 | 33.3 |  |
| LOS | B | C |  | B | C |  | C | E |  | C | C |  |
| Approach Delay |  | 31.3 |  |  | 21.1 |  |  | 70.0 |  |  | 31.5 |  |
| Approach LOS |  | C |  |  | C |  |  | E |  |  | C |  |
| Queue Length 50th (ft) | 24 | 223 |  | 84 | 177 |  | 17 | $\sim 200$ |  | 46 | 103 |  |
| Queue Length 95th (ft) | 63 | 292 |  | 131 | 238 |  | 39 | \#396 |  | 85 | 176 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 437 | 1433 |  | 408 | 1583 |  | 463 | 375 |  | 251 | 512 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.17 | 0.49 |  | 0.64 | 0.46 |  | 0.08 | 0.98 |  | 0.41 | 0.37 |  |

## Intersection Summary

## Cycle Length: 105

Actuated Cycle Length: 105
Offset: 48 ( $46 \%$ ), Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated

## Maximum v/c Ratio: 0.98

Intersection Signal Delay: 33.6
Intersection LOS: C
Intersection Capacity Utilization 79.5\%
ICU Level of Service D
Analysis Period (min) 15
~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 3: 14th St \& IL Route 38


|  | $\rangle$ | $\rightarrow$ |  |  | $\downarrow$ |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | $\hat{\square}$ |  | 7 | $\uparrow$ | 「 | \％ | 个 ${ }^{\text {P }}$ |  | \％ | 中 ${ }^{\text {d }}$ |  |
| Volume（vph） | 49 | 30 | 93 | 180 | 33 | 236 | 56 | 1529 | 244 | 180 | 1713 | 26 |
| Satd．Flow（prot） | 1805 | 1668 | 0 | 1770 | 1845 | 1599 | 1805 | 3459 | 0 | 1787 | 3532 | 0 |
| Flt Permitted | 0.735 |  |  | 0.454 |  |  | 0.057 |  |  | 0.054 |  |  |
| Satd．Flow（perm） | 1395 | 1668 | 0 | 845 | 1845 | 1578 | 108 | 3459 | 0 | 102 | 3532 | 0 |
| Satd．Flow（RTOR） |  | 96 |  |  |  | 229 |  | 22 |  |  | 2 |  |
| Lane Group Flow（vph） | 51 | 127 | 0 | 186 | 34 | 243 | 58 | 1828 | 0 | 186 | 1793 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | Perm | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 |  | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 26.5 |  | 13.0 | 26.5 | 26.5 | 13.0 | 67.5 |  | 13.0 | 67.5 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 6.5 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 19.9 | 10.0 |  | 23.1 | 13.4 | 13.4 | 79.6 | 71.0 |  | 85.3 | 75.6 |  |
| Actuated g／C Ratio | 0.17 | 0.08 |  | 0.19 | 0.11 | 0.11 | 0.66 | 0.59 |  | 0.71 | 0.63 |  |
| v／c Ratio | 0.20 | 0.56 |  | 0.80 | 0.17 | 0.64 | 0.37 | 0.89 |  | 0.94 | 0.81 |  |
| Control Delay | 38.6 | 25.5 |  | 67.8 | 50.6 | 16.0 | 19.7 | 22.7 |  | 79.3 | 21.9 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 3.8 |  | 0.0 | 0.6 |  |
| Total Delay | 38.6 | 25.5 |  | 67.8 | 50.6 | 16.0 | 19.7 | 26.4 |  | 79.3 | 22.5 |  |
| LOS | D | C |  | E | D | B | B | C |  | E | C |  |
| Approach Delay |  | 29.3 |  |  | 39.4 |  |  | 26.2 |  |  | 27.8 |  |
| Approach LOS |  | C |  |  | D |  |  | C |  |  | C |  |
| Queue Length 50th（ft） | 33 | 23 |  | 130 | 25 | 10 | 13 | 272 |  | 90 | 515 |  |
| Queue Length 95th（ft） | 62 | 80 |  | \＃193 | 55 | 88 | m19 | m\＃896 |  | \＃248 | \＃845 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ ft ） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 281 | 358 |  | 232 | 307 | 453 | 201 | 2054 |  | 198 | 2226 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 161 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 3 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 144 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.18 | 0.36 |  | 0.80 | 0.11 | 0.54 | 0.29 | 0.97 |  | 0.94 | 0.86 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length： 120

Actuated Cycle Length： 120
Offset： 2 （2\％），Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green
Control Type：Actuated－Coordinated

## Maximum v／c Ratio： 0.94

Intersection Signal Delay： 28.4
Intersection LOS：C
Intersection Capacity Utilization 95．4\％
ICU Level of Service F
Analysis Period（min） 15
\＃95th percentile volume exceeds capacity，queue may be longer．
Queue shown is maximum after two cycles．
$m$ Volume for 95 th percentile queue is metered by upstream signal．
Splits and Phases：11：Randall Rd \＆Prairie St


Prairie Centre 11：15 am 4／16／2016 Existing 2016

## APPENDIX F

Synchro Analysis Reports<br>2026 Base Traffic

1. Weekday AM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
2. Weekday PM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
3. Saturday Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street

1: Randall Rd \& IL Route 38


Splits and Phases: 1: Randall Rd \& IL Route 38


[^1]Synchro 8 Report

|  | 4 |  |  |  |  |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个t |  | \% | 个t |  | \% | F |  | ${ }^{7}$ | $\uparrow$ | F |
| Volume (vph) | 9 | 876 | 74 | 32 | 456 | 1 | 36 | 2 | 53 | 6 | 1 | 1 |
| Satd. Flow (prot) | 1770 | 3497 | 0 | 1770 | 3539 | 0 | 1770 | 1593 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.470 |  |  | 0.252 |  |  | 0.757 |  |  | 0.718 |  |  |
| Satd. Flow (perm) | 875 | 3497 | 0 | 469 | 3539 | 0 | 1410 | 1593 | 0 | 1337 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 14 |  |  |  |  |  | 58 |  |  |  | 64 |
| Lane Group Flow (vph) | 10 | 1032 | 0 | 35 | 497 | 0 | 39 | 60 | 0 | 7 | 1 | 1 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | Perm | NA |  | Perm | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 15.0 | 81.0 |  | 16.0 | 82.0 |  | 23.0 | 23.0 |  | 23.0 | 23.0 | 23.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |
| Act Effct Green (s) | 100.1 | 95.0 |  | 101.9 | 98.9 |  | 10.8 | 10.8 |  | 10.8 | 10.8 | 10.8 |
| Actuated g/C Ratio | 0.83 | 0.79 |  | 0.85 | 0.82 |  | 0.09 | 0.09 |  | 0.09 | 0.09 | 0.09 |
| v/c Ratio | 0.01 | 0.37 |  | 0.08 | 0.17 |  | 0.31 | 0.31 |  | 0.06 | 0.01 | 0.00 |
| Control Delay | 2.2 | 5.7 |  | 1.8 | 2.4 |  | 56.6 | 17.3 |  | 49.3 | 48.0 | 0.0 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 2.2 | 5.7 |  | 1.8 | 2.4 |  | 56.6 | 17.3 |  | 49.3 | 48.0 | 0.0 |
| LOS | A | A |  | A | A |  | E | B |  | D | D | A |
| Approach Delay |  | 5.7 |  |  | 2.3 |  |  | 32.8 |  |  | 43.7 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | D |  |
| Queue Length 50th (ft) | 1 | 139 |  | 2 | 23 |  | 29 | 1 |  | 5 | 1 | 0 |
| Queue Length 95th (ft) | 4 | 199 |  | 6 | 42 |  | 63 | 43 |  | 20 | 6 | 0 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length (ft) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 833 | 2770 |  | 535 | 2916 |  | 199 | 275 |  | 189 | 263 | 279 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 |  |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.01 | 0.37 |  | 0.07 | 0.17 |  | 0.20 | 0.22 |  | 0.04 | 0.00 | 0.00 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $6(5 \%)$, Referenced to phase 2:EBTL and 6 :WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.37
Intersection Signal Delay: 6.4
Intersection LOS: A
Intersection Capacity Utilization 45.7\%
ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | SWR

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $20(17 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.79
Intersection Signal Delay: 23.5 Intersection LOS: C
Intersection Capacity Utilization 70.0\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 3: 14th St \& IL Route 38


11: Randall Rd \& Prairie St

|  | $\stackrel{*}{ }$ | $\rightarrow$ |  | 7 |  |  | 4 | $\dagger$ | $p$ | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{\beta}$ |  | \% | $\uparrow$ | $\overline{7}$ | \% | 个t |  | \% | 性 |  |
| Volume (vph) |  | 6 | 19 | 144 | 11 | 151 | 16 | 1270 | 128 | 131 | 1432 | 12 |
| Satd. Flow (prot) | 1770 | 1652 | 0 | 1770 | 1863 | 1583 | 1770 | 3490 | 0 | 1770 | 3536 | 0 |
| Flt Permitted | 0.755 |  |  | 0.430 |  |  | 0.121 |  |  | 0.103 |  |  |
| Satd. Flow (perm) | 1406 | 1652 | 0 | 801 | 1863 | 1583 | 225 | 3490 | 0 | 192 | 3536 | 0 |
| Satd. Flow (RTOR) |  | 21 |  |  |  | 144 |  | 13 |  |  | 1 |  |
| Lane Group Flow (vph) | 4 | 28 | 0 | 157 | 12 | 164 | 17 | 1519 | 0 | 142 | 1570 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA | pm+ov | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split (s) | 13.0 | 15.0 |  | 19.0 | 21.0 | 19.0 | 13.0 | 87.0 |  | 19.0 | 93.0 |  |
| Total Lost Time (s) | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green (s) | 11.6 | 7.3 |  | 22.7 | 18.2 | 34.0 | 101.4 | 93.5 |  | 109.3 | 102.9 |  |
| Actuated g/C Ratio | 0.08 | 0.05 |  | 0.16 | 0.13 | 0.24 | 0.72 | 0.67 |  | 0.78 | 0.74 |  |
| v/c Ratio | 0.03 | 0.26 |  | 0.69 | 0.05 | 0.33 | 0.08 | 0.65 |  | 0.56 | 0.60 |  |
| Control Delay | 44.8 | 35.8 |  | 68.3 | 51.3 | 9.9 | 2.4 | 4.4 |  | 15.3 | 12.1 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 |  | 0.0 | 0.0 |  |
| Total Delay | 44.8 | 35.8 |  | 68.3 | 51.3 | 9.9 | 2.4 | 4.6 |  | 15.3 | 12.1 |  |
| LOS | D | D |  | E | D | A | A | A |  | B | B |  |
| Approach Delay |  | 37.0 |  |  | 38.9 |  |  | 4.5 |  |  | 12.4 |  |
| Approach LOS |  | D |  |  | D |  |  | A |  |  | B |  |
| Queue Length 50th (ft) | 3 | 6 |  | 126 | 9 | 13 | 1 | 60 |  | 33 | 341 |  |
| Queue Length 95th (ft) | 14 | 39 |  | 197 | 31 | 71 | m2 | 82 |  | 72 | 538 |  |
| Internal Link Dist (ft) |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length ( ft ) | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity (vph) | 176 | 120 |  | 238 | 264 | 552 | 267 | 2335 |  | 319 | 2599 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 185 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.02 | 0.23 |  | 0.66 | 0.05 | 0.30 | 0.06 | 0.71 |  | 0.45 | 0.60 |  |

## Cycle Length: 140

Actuated Cycle Length: 140
Offset: $0(0 \%)$, Referenced to phase 2:NBTL and 6:SBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.69
Intersection Signal Delay: 11.7 Intersection LOS: B
Intersection Capacity Utilization $75.3 \% \quad$ ICU Level of Service D
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 11: Randall Rd \& Prairie St


1：Randall Rd \＆IL Route 38

|  |  |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}{ }^{\text {\％}}$ | 个个 | 「 | \％${ }^{1 / 1}$ | 个4 | 「 | ${ }^{1+1}$ | 个个 | 「 | ${ }^{17}$ | 个个 | 7 |
| Volume（vph） | 298 | 368 | 87 | 138 | 584 | 399 | 111 | 1225 | 54 | 273 | 1231 | 290 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Satd．Flow（RTOR） |  |  | 97 |  |  | 97 |  |  | 132 |  |  | 62 |
| Lane Group Flow（vph） | 324 | 400 | 95 | 150 | 635 | 434 | 121 | 1332 | 59 | 297 | 1338 | 315 |
| Turn Type | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov |
| Protected Phases | 5 | 2 |  | ， | 6 | 7 |  | ， | ， | 7 | 4 | 5 |
| Permitted Phases |  |  | 2 |  |  | 6 |  |  | 8 |  |  | 4 |
| Total Split（s） | 20.0 | 39.0 | 13.0 | 15.0 | 34.0 | 19.0 | 13.0 | 67.0 | 15.0 | 19.0 | 73.0 | 20.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Effct Green（s） | 15.3 | 33.0 | 47.8 | 10.0 | 27.7 | 48.5 | 8.3 | 60.7 | 77.2 | 14.3 | 66.7 | 88.5 |
| Actuated g／C Ratio | 0.11 | 0.24 | 0.34 | 0.07 | 0.20 | 0.35 | 0.06 | 0.43 | 0.55 | 0.10 | 0.48 | 0.63 |
| v／c Ratio | 0.86 | 0.48 | 0.16 | 0.61 | 0.91 | 0.71 | 0.60 | 0.87 | 0.06 | 0.85 | 0.79 | 0.31 |
| Control Delay | 83.7 | 48.5 | 6.2 | 74.4 | 72.5 | 37.7 | 76.8 | 43.4 | 0.1 | 86.8 | 26.2 | 6.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 13.4 | 0.0 | 0.0 | 1.3 | 0.0 |
| Total Delay | 83.7 | 48.5 | 6.2 | 74.4 | 72.5 | 37.8 | 76.8 | 56.8 | 0.1 | 86.8 | 27.5 | 6.6 |
| LOS | F | D | A | E | E | D | E | E | A | F | C | A |
| Approach Delay |  | 57.5 |  |  | 60.4 |  |  | 56.2 |  |  | 33.2 |  |
| Approach LOS |  | E |  |  | E |  |  | E |  |  | C |  |
| Queue Length 50th（ft） | 151 | 167 | 0 | 69 | 301 | 270 | 56 | 573 | 0 | 130 | 565 | 62 |
| Queue Length 95th（ft） | \＃230 | 221 | 38 | 107 | \＃411 | 402 | 91 | 677 | 0 | m\＃195 | m399 | m49 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ t ） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 380 | 835 | 607 | 257 | 700 | 613 | 208 | 1535 | 937 | 355 | 1686 | 1025 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | ， | 0 | 0 | 0 | 0 | 168 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 214 | 0 |  | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.85 | 0.48 | 0.16 | 0.58 | 0.91 | 0.71 | 0.58 | 1.01 | 0.06 | 0.84 | 0.88 | 0.31 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 0 （0\％），Referenced to phase 4：SBT and 8：NBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.91 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 49.2 |  |  |  |  | Intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 84．6\％ |  |  |  |  | ICU Level of Service E |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


Prairie Centre 4：30 pm 4／12／2016 2026 Projected Traffic

|  | $\rangle$ |  |  | 7 |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个 ${ }_{\text {¢ }}$ |  | \% | 性 |  | \% | F |  | ${ }^{7}$ | $\uparrow$ | F |
| Volume (vph) | 21 | 635 | 51 | 63 | 1060 | 9 | 53 | 2 | 66 | 22 | 4 | 8 |
| Satd. Flow (prot) | 1770 | 3500 | 0 | 1770 | 3536 | 0 | 1770 | 1591 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.224 |  |  | 0.352 |  |  | 0.755 |  |  | 0.709 |  |  |
| Satd. Flow (perm) | 417 | 3500 | 0 | 656 | 3536 | 0 | 1406 | 1591 | 0 | 1321 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 13 |  |  | 1 |  |  | 72 |  |  |  | 64 |
| Lane Group Flow (vph) | 23 | 745 | 0 | 68 | 1162 | 0 | 58 | 74 | 0 | 24 | 4 | 9 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | Perm | NA |  | Perm | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 14.0 | 81.0 |  | 15.0 | 82.0 |  | 24.0 | 24.0 |  | 24.0 | 24.0 | 24.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |
| Act Effct Green (s) | 97.7 | 91.3 |  | 99.5 | 93.7 |  | 12.2 | 12.2 |  | 12.1 | 12.1 | 12.1 |
| Actuated g/C Ratio | 0.81 | 0.76 |  | 0.83 | 0.78 |  | 0.10 | 0.10 |  | 0.10 | 0.10 | 0.10 |
| v/c Ratio | 0.06 | 0.28 |  | 0.11 | 0.42 |  | 0.41 | 0.33 |  | 0.18 | 0.02 | 0.04 |
| Control Delay | 2.8 | 6.3 |  | 0.7 | 5.3 |  | 58.0 | 15.4 |  | 50.8 | 46.2 | 0.4 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 2.8 | 6.3 |  | 0.7 | 5.3 |  | 58.0 | 15.4 |  | 50.8 | 46.2 | 0.4 |
| LOS | A | A |  | A | A |  | E | B |  | D | D | A |
| Approach Delay |  | 6.2 |  |  | 5.1 |  |  | 34.1 |  |  | 38.1 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | D |  |
| Queue Length 50th (ft) | 3 | 96 |  | 1 | 223 |  | 43 | 1 |  | 17 | 3 | 0 |
| Queue Length 95th (ft) | 8 | 145 |  | m2 | 435 |  | 84 | 46 |  | 43 | 14 | 0 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length (ft) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 469 | 2666 |  | 659 | 2760 |  | 210 | 299 |  | 198 | 279 | 291 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.05 | 0.28 |  | 0.10 | 0.42 |  | 0.28 | 0.25 |  | 0.12 | 0.01 | 0.03 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $0(0 \%)$, Referenced to phase 2:EBTL and $6: W B T L$, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.42
Intersection Signal Delay: 7.8
Intersection LOS: A
Intersection Capacity Utilization 56.7\%
ICU Level of Service B
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | 中 ${ }^{\text {a }}$ |  | ${ }^{*}$ | 中 ${ }^{\text {W }}$ |  | ${ }^{*}$ | $\uparrow$ |  | ${ }^{*}$ | $\hat{\beta}$ |  |
| Volume (vph) | 103 | 629 | 38 | 241 | 959 | 81 | 67 | 171 | 207 | 130 | 208 | 56 |
| Satd. Flow (prot) | 1770 | 3511 | 0 | 1770 | 3497 | 0 | 1770 | 1710 | 0 | 1770 | 1803 | 0 |
| Flt Permitted | 0.129 |  |  | 0.239 |  |  | 0.446 |  |  | 0.182 |  |  |
| Satd. Flow (perm) | 240 | 3511 | 0 | 445 | 3497 | 0 | 831 | 1710 | 0 | 339 | 1803 | 0 |
| Satd. Flow (RTOR) |  | 5 |  |  | 9 |  |  | 50 |  |  | 11 |  |
| Lane Group Flow (vph) | 112 | 725 | 0 | 262 | 1130 | 0 | 73 | 411 | 0 | 141 | 287 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 44.0 |  | 23.0 | 54.0 |  | 13.0 | 40.0 |  | 13.0 | 40.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.5 |  |
| Act Effct Green (s) | 56.4 | 44.8 |  | 66.2 | 51.2 |  | 42.1 | 30.9 |  | 44.5 | 33.9 |  |
| Actuated g/C Ratio | 0.47 | 0.37 |  | 0.55 | 0.43 |  | 0.35 | 0.26 |  | 0.37 | 0.28 |  |
| v/c Ratio | 0.50 | 0.55 |  | 0.64 | 0.75 |  | 0.21 | 0.86 |  | 0.59 | 0.56 |  |
| Control Delay | 26.9 | 38.4 |  | 22.5 | 33.8 |  | 23.8 | 55.5 |  | 34.5 | 40.1 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 26.9 | 38.4 |  | 22.5 | 33.8 |  | 23.8 | 55.5 |  | 34.5 | 40.1 |  |
| LOS | C | D |  | C | C |  | C | E |  | C | D |  |
| Approach Delay |  | 36.8 |  |  | 31.6 |  |  | 50.7 |  |  | 38.3 |  |
| Approach LOS |  | D |  |  | C |  |  | D |  |  | D |  |
| Queue Length 50th ( ft ) | 55 | 280 |  | 108 | 401 |  | 34 | 263 |  | 69 | 181 |  |
| Queue Length 95th (ft) | 92 | 347 |  | 162 | 493 |  | 65 | \#419 |  | 114 | 275 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 235 | 1314 |  | 460 | 1497 |  | 374 | 513 |  | 239 | 521 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.48 | 0.55 |  | 0.57 | 0.75 |  | 0.20 | 0.80 |  | 0.59 | 0.55 |  |

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: 75 (63\%), Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.86
Intersection Signal Delay: $36.9 \quad$ Intersection LOS: D
Intersection Capacity Utilization $81.2 \% \quad$ ICU Level of Service D
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Splits and Phases: $\quad 3: 14$ th St \& IL Route 38


|  | $\rangle$ |  |  |  |  |  |  | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | $\uparrow$ |  | \％ | $\uparrow$ | 「 | ${ }^{7}$ | 㻢 |  | 7 | 性 |  |
| Volume（vph） | 70 | 34 | 112 | 241 | 38 | 249 | 68 | 1606 | 197 | 165 | 1510 | 35 |
| Satd．Flow（prot） | 1770 | 1649 | 0 | 1770 | 1863 | 1583 | 1770 | 3483 | 0 | 1770 | 3529 | 0 |
| FIt Permitted | 0.730 |  |  | 0.303 |  |  | 0.092 |  |  | 0.048 |  |  |
| Satd．Flow（perm） | 1360 | 1649 | 0 | 564 | 1863 | 1583 | 171 | 3483 | 0 | 89 | 3529 | 0 |
| Satd．Flow（RTOR） |  | 106 |  |  |  | 89 |  | 14 |  |  | 2 |  |
| Lane Group Flow（vph） | 76 | 159 | 0 | 262 | 41 | 271 | 74 | 1960 | 0 | 179 | 1679 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | pm＋ov | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | ， | 8 | ， | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 35.0 |  | 13.0 | 35.0 | 14.0 | 13.0 | 78.0 |  | 14.0 | 79.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 22.5 | 11.7 |  | 24.1 | 14.4 | 38.2 | 90.7 | 81.0 |  | 104.8 | 93.0 |  |
| Actuated g／C Ratio | 0.16 | 0.08 |  | 0.17 | 0.10 | 0.27 | 0.65 | 0.58 |  | 0.75 | 0.66 |  |
| v／c Ratio | 0.31 | 0.68 |  | 1.51 | 0.22 | 0.55 | 0.38 | 0.97 |  | 0.65 | 0.72 |  |
| Control Delay | 49.7 | 36.8 |  | 291.5 | 60.6 | 32.7 | 10.5 | 28.6 |  | 43.3 | 19.0 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 1.4 |  | 0.0 | 0.0 |  |
| Total Delay | 49.7 | 36.8 |  | 291.5 | 60.6 | 32.7 | 10.5 | 30.0 |  | 43.3 | 19.0 |  |
| LOS | D | D |  | F | E | C | B | C |  | D | B |  |
| Approach Delay |  | 40.9 |  |  | 152.8 |  |  | 29.3 |  |  | 21.3 |  |
| Approach LOS |  | D |  |  | F |  |  | C |  |  | C |  |
| Queue Length 50th（ft） | 59 | 47 |  | $\sim 283$ | 36 | 146 | 13 | 361 |  | 101 | 486 |  |
| Queue Length 95th（ft） | 101 | 119 |  | \＃405 | 72 | 219 | m19 | \＃1183 |  | 187 | 727 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（tt） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 251 | 420 |  | 174 | 379 | 496 | 220 | 2020 |  | 275 | 2343 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 21 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 31 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.30 | 0.38 |  | 1.51 | 0.11 | 0.55 | 0.34 | 0.98 |  | 0.65 | 0.73 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 0 （0\％），Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 1.51 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 41.8 |  |  |  |  | Intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 99．3\％Analysis Period（min） 15 |  |  |  |  | ICU Level of Service F |  |  |  |  |  |  |  |
|  |  |  |  |  | Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |
| ～Volume exceeds capacity，queue is theoretically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| $m$ Volume for 95th perc | queue | metere | by ups | ream sig |  |  |  |  |  |  |  |  |

Splits and Phases：11：Randall Rd \＆Prairie St


1：Randall Rd \＆IL Route 38

|  | 4 |  |  | 7 |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊＊ | 个个 | 「 | ${ }^{7 \times 1}$ | 个个 | 「 | ${ }^{1+1}$ | 个4 | 「 | \％${ }^{1 / 4}$ | 个个 | F |
| Volume（vph） | 394 | 465 | 118 | 194 | 347 | 334 | 116 | 1227 | 89 | 329 | 1506 | 301 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Satd．Flow（RTOR） |  |  | 114 |  |  | 114 |  |  | 155 |  |  | 73 |
| Lane Group Flow（vph） | 428 | 505 | 128 | 211 | 377 | 363 | 126 | 1334 | 97 | 358 | 1637 | 327 |
| Turn Type | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | $\mathrm{pm}+\mathrm{ov}$ |
| Protected Phases | 5 | 2 | 3 | 1 | 6 | 7 | 3 | 8 | 1 | 7 | 4 | 5 |
| Permitted Phases |  |  | 2 |  |  | 6 |  |  | 8 |  |  |  |
| Total Split（s） | 20.0 | 27.0 | 13.0 | 13.0 | 20.0 | 20.0 | 13.0 | 60.0 | 13.0 | 20.0 | 67.0 | 20.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Effict Green（s） | 15.5 | 20.5 | 35.2 | 8.5 | 13.5 | 35.1 | 8.2 | 53.9 | 68.9 | 15.1 | 60.8 | 82.8 |
| Actuated g／C Ratio | 0.13 | 0.17 | 0.29 | 0.07 | 0.11 | 0.29 | 0.07 | 0.45 | 0.57 | 0.13 | 0.51 | 0.69 |
| v／c Ratio | 0.97 | 0.84 | 0.23 | 0.87 | 0.95 | 0.67 | 0.54 | 0.84 | 0.10 | 0.83 | 0.91 | 0.29 |
| Control Delay | 87.3 | 61.4 | 8.5 | 87.3 | 86.8 | 31.9 | 62.7 | 35.3 | 0.5 | 71.3 | 20.4 | 1.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 5.0 | 0.0 | 0.0 | 5.2 | 0.0 |
| Total Delay | 87.3 | 61.4 | 8.5 | 87.3 | 86.8 | 31.9 | 62.7 | 40.3 | 0.5 | 71.3 | 25.6 | 1.6 |
| LOS | F | E | A | F | F | C | E | D | A | E | C | A |
| Approach Delay |  | 65.5 |  |  | 65.9 |  |  | 39.7 |  |  | 29.3 |  |
| Approach LOS |  | E |  |  | E |  |  | D |  |  | C |  |
| Queue Length 50th（ ft ） | 172 | 201 | 8 | 85 | 154 | 170 | 49 | 473 | 0 | 140 | 416 | 12 |
| Queue Length 95th（ ft ） | \＃275 | \＃283 | 54 | \＃154 | \＃252 | 282 | 81 | 574 | 4 | m157 | m534 | m12 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ft） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 443 | 604 | 548 | 243 | 398 | 548 | 243 | 1588 | 974 | 443 | 1792 | 1114 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 121 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 198 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | O |
| Reduced v／c Ratio | 0.97 | 0.84 | 0.23 | 0.87 | 0.95 | 0.67 | 0.52 | 0.96 | 0.10 | 0.81 | 0.98 | 0.29 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $12(10 \%)$ ，Referenced to phase 4：SBT and 8：NBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.97 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 44.5 |  |  |  |  | intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 84．1\％ |  |  |  |  | ICU Level of Service E |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


Prairie Centre 11：15 am 4／16／2016 2026 Projected Traffic

|  | 4 |  |  |  |  |  |  | 4 |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 中 ${ }_{\text {d }}$ |  | \％ | 中 ${ }^{\text {b }}$ |  | \％ | $\uparrow$ |  | \％ | $\uparrow$ | 「 |
| Volume（vph） | 20 | 717 | 65 | 39 | 759 | 7 | 53 | 1 | 70 | 20 | 8 | 2 |
| Satd．Flow（prot） | 1770 | 3493 | 0 | 1770 | 3536 | 0 | 1770 | 1587 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.332 |  |  | 0.312 |  |  | 0.752 |  |  | 0.707 |  |  |
| Satd．Flow（perm） | 618 | 3493 | 0 | 581 | 3536 | 0 | 1401 | 1587 | 0 | 1317 | 1863 | 1583 |
| Satd．Flow（RTOR） |  | 14 |  |  | 1 |  |  | 76 |  |  |  | 73 |
| Lane Group Flow（vph） | 22 | 850 | 0 | 42 | 833 | 0 | 58 | 77 | 0 | 22 | 9 | 2 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA |  | Perm | NA |  | Perm | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split（s） | 15.0 | 64.0 |  | 16.0 | 65.0 |  | 25.0 | 25.0 |  | 25.0 | 25.0 | 25.0 |
| Total Lost Time（s） | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |
| Act Effct Green（s） | 84.2 | 79.0 |  | 85.4 | 81.0 |  | 11.6 | 11.6 |  | 11.6 | 11.6 | 11.6 |
| Actuated g／C Ratio | 0.80 | 0.75 |  | 0.81 | 0.77 |  | 0.11 | 0.11 |  | 0.11 | 0.11 | 0.11 |
| v／c Ratio | 0.04 | 0.32 |  | 0.08 | 0.31 |  | 0.37 | 0.32 |  | 0.15 | 0.04 | 0.01 |
| Control Delay | 2.9 | 6.6 |  | 1.4 | 2.9 |  | 49.2 | 13.4 |  | 43.0 | 40.0 | 0.0 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 2.9 | 6.6 |  | 1.4 | 2.9 |  | 49.2 | 13.4 |  | 43.0 | 40.0 | 0.0 |
| LOS | A | A |  | A | A |  | D | B |  | D | D | A |
| Approach Delay |  | 6.5 |  |  | 2.8 |  |  | 28.8 |  |  | 39.6 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | D |  |
| Queue Length 50th（ft） | 2 | 111 |  | 2 | 31 |  | 37 | 1 |  | 14 | 6 | 0 |
| Queue Length 95th（ft） | 8 | 167 |  | m4 | 71 |  | 75 | 42 |  | 36 | 20 | 0 |
| Internal Link Dist（ft） |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length（ ft ） | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity（vph） | 635 | 2630 |  | 620 | 2729 |  | 253 | 349 |  | 238 | 337 | 346 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.03 | 0.32 |  | 0.07 | 0.31 |  | 0.23 | 0.22 |  | 0.09 | 0.03 | 0.01 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 105 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 105 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $57(54 \%)$ ，Referenced to phase 2：EBTL and 6：WBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.37 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 6.9 |  |  |  | Intersection LOS：A |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 48．6\％ |  |  |  | ICU Level of Service A |  |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| $m$ Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：2：IL Route 38 \＆W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个t |  | \% | 个t |  | 7 | F |  | \% | F |  |
| Volume (vph) | 75 | 690 | 43 | 268 | 699 | 60 | 55 | 130 | 254 | 108 | 166 | 32 |
| Satd. Flow (prot) | 1770 | 3507 | 0 | 1770 | 3497 | 0 | 1770 | 1678 | 0 | 1770 | 1818 | 0 |
| Flt Permitted | 0.285 |  |  | 0.168 |  |  | 0.592 |  |  | 0.191 |  |  |
| Satd. Flow (perm) | 531 | 3507 | 0 | 313 | 3497 | 0 | 1103 | 1678 | 0 | 356 | 1818 | 0 |
| Satd. Flow (RTOR) |  | 6 |  |  | 9 |  |  | 99 |  |  | 10 |  |
| Lane Group Flow (vph) | 82 | 797 | 0 | 291 | 825 | 0 | 60 | 417 | 0 | 117 | 215 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 39.0 |  | 13.0 | 39.0 |  | 13.0 | 40.0 |  | 13.0 | 40.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Efft Green (s) | 44.5 | 33.6 |  | 54.0 | 42.7 |  | 37.8 | 27.8 |  | 41.1 | 31.1 |  |
| Actuated g/C Ratio | 0.42 | 0.32 |  | 0.51 | 0.41 |  | 0.36 | 0.26 |  | 0.39 | 0.30 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.26 | 0.71 |  | 0.79 | 0.58 |  | 0.13 | 0.81 |  | 0.45 | 0.40 |  |
| Control Delay | 15.9 | 30.8 |  | 37.2 | 28.4 |  | 17.6 | 39.3 |  | 23.8 | 30.0 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 15.9 | 30.8 |  | 37.2 | 28.4 |  | 17.6 | 39.3 |  | 23.8 | 30.0 |  |
| LOS | B | C |  | D | C |  | B | D |  | C | C |  |
| Approach Delay |  | 29.4 |  |  | 30.7 |  |  | 36.6 |  |  | 27.8 |  |
| Approach LOS |  | C |  |  | C |  |  | D |  |  | C |  |
| Queue Length 50th ( ft ) | 27 | 168 |  | 109 | 233 |  | 24 | 201 |  | 48 | 110 |  |
| Queue Length 95th (ft) | 50 | 210 |  | \#316 | 333 |  | 45 | 300 |  | 77 | 169 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 345 | 1127 |  | 370 | 1428 |  | 478 | 610 |  | 267 | 597 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.24 | 0.71 |  | 0.79 | 0.58 |  | 0.13 | 0.68 |  | 0.44 | 0.36 |  |

## Intersection Summary

## Cycle Length: 105

Actuated Cycle Length: 105
Offset: $60(57 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.81
Intersection Signal Delay: 30.9
Intersection LOS: C
Intersection Capacity Utilization 80.8\%
ICU Level of Service D
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 3: 14th St \& IL Route 38


11：Randall Rd \＆Prairie St

|  | 4 |  |  |  |  |  |  | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个 |  | \％ | $\uparrow$ | 「 | ＊ | 个 ${ }^{\text {a }}$ |  | \％ | 中 ${ }^{\text {b }}$ |  |
| Volume（vph） | 62 | 38 | 117 | 226 | 41 | 296 | 60 | 1625 | 259 | 191 | 1821 | 28 |
| Satd．Flow（prot） | 1770 | 1652 | 0 | 1770 | 1863 | 1583 | 1770 | 3465 | 0 | 1770 | 3532 | 0 |
| FIt Permitted | 0.728 |  |  | 0.305 |  |  | 0.060 |  |  | 0.056 |  |  |
| Satd．Flow（perm） | 1356 | 1652 | 0 | 568 | 1863 | 1583 | 112 | 3465 | 0 | 104 | 3532 | 0 |
| Satd．Flow（RTOR） |  | 100 |  |  |  | 84 |  | 24 |  |  | 2 |  |
| Lane Group Flow（vph） | 67 | 168 | 0 | 246 | 45 | 322 | 65 | 2048 | 0 | 208 | 2009 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | pm＋ov | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 15.0 |  | 16.0 | 18.0 | 15.0 | 13.0 | 74.0 |  | 15.0 | 76.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 18.7 | 8.3 |  | 26.3 | 14.3 | 31.7 | 76.8 | 67.8 |  | 84.8 | 74.0 |  |
| Actuated g／C Ratio | 0.16 | 0.07 |  | 0.22 | 0.12 | 0.26 | 0.64 | 0.56 |  | 0.71 | 0.62 |  |
| v／c Ratio | 0.28 | 0.81 |  | 1.01 | 0.20 | 0.67 | 0.40 | 1.04 |  | 0.93 | 0.92 |  |
| Control Delay | 41.2 | 52.3 |  | 103.3 | 52.7 | 37.6 | 15.6 | 46.4 |  | 74.6 | 29.9 |  |
| Queue Delay | 0.0 | 0.5 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.6 |  | 0.0 | 2.4 |  |
| Total Delay | 41.2 | 52.8 |  | 103.3 | 52.7 | 37.6 | 15.6 | 47.0 |  | 74.6 | 32.3 |  |
| LOS | D | D |  | F | D | D | B | D |  | E | C |  |
| Approach Delay |  | 49.5 |  |  | 65.1 |  |  | 46.1 |  |  | 36.3 |  |
| Approach LOS |  | D |  |  | E |  |  | D |  |  | D |  |
| Queue Length 50th（ft） | 42 | 52 |  | ～173 | 33 | 171 | 13 | $\sim 883$ |  | 108 | 711 |  |
| Queue Length 95th（ft） | 83 | \＃169 |  | \＃310 | 71 | 282 | m22 | 1014 |  | \＃259 | \＃972 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ ft ） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 255 | 209 |  | 244 | 222 | 481 | 198 | 1967 |  | 226 | 2177 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 3 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 2 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 91 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.26 | 0.81 |  | 1.01 | 0.20 | 0.67 | 0.33 | 1.04 |  | 0.92 | 0.96 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 0 （0\％），Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 1.04 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 44.3 |  |  |  |  | Intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 103．0\％ |  |  |  |  | ICU Level of Service G |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| ～Volume exceeds capacity，queue is theoretically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：11：Randall Rd \＆Prairie St


## APPENDIX G

Synchro Analysis Reports<br>2026 Total Traffic<br>No Improvements

1. Weekday AM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
2. Weekday PM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
3. Saturday Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street

1：Randall Rd \＆IL Route 38

|  | $\rangle$ |  |  |  |  | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％${ }^{*}$ | 个个 | F | \％${ }^{1 / 1}$ | 个个 | F | \％${ }^{1+1}$ | 个4 | F | \％${ }^{1+1}$ | 个4 | F |
| Volume（vph） | 242 | 623 | 39 | 181 | 300 | 208 | 98 | 1012 | 129 | 365 | 1013 | 255 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Satd．Flow（RTOR） |  |  | 132 |  |  | 226 |  |  | 132 |  |  | 131 |
| Lane Group Flow（vph） | 263 | 677 | 42 | 197 | 326 | 226 | 107 | 1100 | 140 | 397 | 1101 | 277 |
| Turn Type | Prot | NA | Prot | Prot | NA | Prot | Prot | NA | pt＋ov | Prot | NA | pt＋ov |
| Protected Phases | 5 | 2 | 2 | 1 | 6 | － | 3 | 8 | 81 | 7 | ， | 45 |
| Permitted Phases |  |  |  |  |  |  |  |  |  |  |  |  |
| Total Split（s） | 21.0 | 39.0 | 39.0 | 16.0 | 34.0 | 34.0 | 13.0 | 59.0 |  | 26.0 | 72.0 |  |
| Total Lost Time（s） | 4.5 | 6.5 | 6.5 | 4.5 | 6.5 | 6.5 | 4.5 | 6.5 |  | 4.5 | 6.5 |  |
| Act Efft Green（s） | 15.0 | 32.6 | 32.6 | 11.2 | 28.8 | 28.8 | 8.2 | 54.3 | 70.0 | 19.9 | 66.0 | 85.5 |
| Actuated g／C Ratio | 0.11 | 0.23 | 0.23 | 0.08 | 0.21 | 0.21 | 0.06 | 0.39 | 0.50 | 0.14 | 0.47 | 0.61 |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.72 | 0.82 | 0.09 | 0.72 | 0.45 | 0.45 | 0.53 | 0.80 | 0.16 | 0.81 | 0.66 | 0.27 |
| Control Delay | 71.7 | 60.3 | 0.4 | 78.3 | 51.3 | 8.7 | 74.0 | 43.9 | 4.0 | 80.4 | 24.1 | 3.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 | 0.0 | 0.0 | 0.4 | 0.0 |
| Total Delay | 71.7 | 60.3 | 0.4 | 78.3 | 51.3 | 8.7 | 74.0 | 44.1 | 4.0 | 80.4 | 24.5 | 3.6 |
| LOS | E | E | A | E | D | A | E | D | A | F | C | A |
| Approach Delay |  | 60.8 |  |  | 45.6 |  |  | 42.3 |  |  | 33.8 |  |
| Approach LOS |  | E |  |  | D |  |  | D |  |  | C |  |
| Queue Length 50th（ft） | 120 | 310 | 0 | 91 | 138 | 0 | 49 | 474 | 4 | 183 | 345 | 14 |
| Queue Length 95th（ft） | 168 | 386 | 0 | \＃136 | 190 | 71 | 82 | 567 | 40 | 254 | 253 | m24 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（tt） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 404 | 824 | 469 | 281 | 727 | 504 | 208 | 1372 | 860 | 527 | 1668 | 1033 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 182 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 30 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.65 | 0.82 | 0.09 | 0.70 | 0.45 | 0.45 | 0.51 | 0.82 | 0.16 | 0.75 | 0.74 | 0.27 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $130(93 \%)$ ，Referenced to phase 4：SBT and 8：NBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.82 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 43.4 |  |  |  |  | ntersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 79．1\％ICU Level of Service D |  |  |  |  |  |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95th perc | queue | metere | by upst | am sig |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


Prairie Centre 7：15 am 4／12／2016 2026 Total Traffic without Improvements

|  | 4 |  |  | $\dagger$ |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | * | 个 ${ }^{\text {b }}$ |  | \% | 个 ${ }^{\text {a }}$ |  | \% | F |  | \% | $\uparrow$ | T |
| Volume (vph) | 182 | 879 | 74 | 32 | 480 | 46 | 36 | 2 | 53 | 79 | 1 | 194 |
| Satd. Flow (prot) | 1770 | 3497 | 0 | 1770 | 3493 | 0 | 1770 | 1593 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.410 |  |  | 0.264 |  |  | 0.757 |  |  | 0.718 |  |  |
| Satd. Flow (perm) | 764 | 3497 | 0 | 492 | 3493 | 0 | 1410 | 1593 | 0 | 1337 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 11 |  |  | 12 |  |  | 58 |  |  |  | 211 |
| Lane Group Flow (vph) | 198 | 1035 | 0 | 35 | 572 | 0 | 39 | 60 | 0 | 86 | 1 | 211 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | Perm | NA |  | Perm | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 20.0 | 71.0 |  | 15.0 | 66.0 |  | 34.0 | 34.0 |  | 34.0 | 34.0 | 34.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |
| Act Effct Green (s) | 94.9 | 86.1 |  | 88.7 | 79.7 |  | 15.5 | 15.5 |  | 15.5 | 15.5 | 15.5 |
| Actuated g/C Ratio | 0.79 | 0.72 |  | 0.74 | 0.66 |  | 0.13 | 0.13 |  | 0.13 | 0.13 | 0.13 |
| v/c Ratio | 0.29 | 0.41 |  | 0.08 | 0.25 |  | 0.22 | 0.24 |  | 0.50 | 0.00 | 0.55 |
| Control Delay | 4.6 | 8.5 |  | 2.8 | 5.5 |  | 47.4 | 13.7 |  | 57.4 | 41.0 | 11.3 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 4.6 | 8.5 |  | 2.8 | 5.5 |  | 47.4 | 13.7 |  | 57.4 | 41.0 | 11.3 |
| LOS | A | A |  | A | A |  | D | B |  | E | D | B |
| Approach Delay |  | 7.9 |  |  | 5.3 |  |  | 27.0 |  |  | 24.7 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Queue Length 50th (ft) | 29 | 163 |  | 3 | 47 |  | 28 | 1 |  | 63 | 1 | 0 |
| Queue Length 95th (ft) | 63 | 256 |  | 8 | 64 |  | 58 | 39 |  | 110 | 6 | 66 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length (ft) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 743 | 2512 |  | 507 | 2322 |  | 329 | 416 |  | 311 | 434 | 531 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.27 | 0.41 |  | 0.07 | 0.25 |  | 0.12 | 0.14 |  | 0.28 | 0.00 | 0.40 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $8(7 \%)$, Referenced to phase 2:EBTL and $6: W B T L$, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.55
Intersection Signal Delay: 10.3 Intersection LOS: B
Intersection Capacity Utilization 54.8\% ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 性 |  | \% | 个 ${ }^{\text {P }}$ |  | ${ }^{7}$ | $\hat{\beta}$ |  | \% | $\hat{\square}$ |  |
| Volume (vph) | 73 | 966 | 19 | 95 | 495 | 47 | 24 | 85 | 221 | 100 | 98 | 34 |
| Satd. Flow (prot) | 1770 | 3529 | 0 | 1770 | 3493 | 0 | 1770 | 1662 | 0 | 1770 | 1790 | 0 |
| Flt Permitted | 0.397 |  |  | 0.162 |  |  | 0.665 |  |  | 0.192 |  |  |
| Satd. Flow (perm) | 740 | 3529 | 0 | 302 | 3493 | 0 | 1239 | 1662 | 0 | 358 | 1790 | 0 |
| Satd. Flow (RTOR) |  | 2 |  |  | 10 |  |  | 106 |  |  | 14 |  |
| Lane Group Flow (vph) | 79 | 1071 | 0 | 103 | 589 | 0 | 26 | 332 | 0 | 109 | 144 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 57.0 |  | 13.0 | 57.0 |  | 13.0 | 37.0 |  | 13.0 | 37.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Effct Green (s) | 70.0 | 59.3 |  | 71.6 | 61.6 |  | 33.1 | 23.9 |  | 38.2 | 30.0 |  |
| Actuated g/C Ratio | 0.58 | 0.49 |  | 0.60 | 0.51 |  | 0.28 | 0.20 |  | 0.32 | 0.25 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.16 | 0.61 |  | 0.37 | 0.33 |  | 0.07 | 0.80 |  | 0.49 | 0.31 |  |
| Control Delay | 10.3 | 19.5 |  | 14.7 | 19.3 |  | 25.3 | 44.8 |  | 35.1 | 34.7 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 10.3 | 19.5 |  | 14.7 | 19.3 |  | 25.3 | 44.8 |  | 35.1 | 34.7 |  |
| LOS | B | B |  | B | B |  | C | D |  | D | C |  |
| Approach Delay |  | 18.9 |  |  | 18.6 |  |  | 43.4 |  |  | 34.9 |  |
| Approach LOS |  | B |  |  | B |  |  | D |  |  | C |  |
| Queue Length 50th (ft) | 20 | 198 |  | 31 | 140 |  | 14 | 170 |  | 60 | 85 |  |
| Queue Length 95th (ft) | 42 | 237 |  | 64 | 210 |  | 32 | 262 |  | 96 | 138 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 524 | 1744 |  | 298 | 1798 |  | 412 | 507 |  | 225 | 484 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.15 | 0.61 |  | 0.35 | 0.33 |  | 0.06 | 0.65 |  | 0.48 | 0.30 |  |

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $8(7 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.80
Intersection Signal Delay: 24.0
Intersection LOS: C
Intersection Capacity Utilization 73.3\%
ICU Level of Service D
Analysis Period (min) 15
Splits and Phases: 3: 14th St \& IL Route 38


11：Randall Rd \＆Prairie St

|  | $\Rightarrow$ |  |  |  |  |  |  | $\dagger$ | 7 | ＊ | $\frac{1}{7}$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊ | $\stackrel{\rightharpoonup}{ }$ |  | \％ | $\uparrow$ | 「 | \％ | 中 ${ }^{\text {c }}$ |  | \％ | 性 |  |
| Volume（vph） | 4 | 6 | 19 | 144 | 11 | 237 | 16 | 1307 | 146 | 176 | 1489 | 12 |
| Satd．Flow（prot） | 1770 | 1652 | 0 | 1770 | 1863 | 1583 | 1770 | 3486 | 0 | 1770 | 3536 | 0 |
| Flt Permitted | 0.750 |  |  | 0.431 |  |  | 0.118 |  |  | 0.084 |  |  |
| Satd．Flow（perm） | 1397 | 1652 | 0 | 803 | 1863 | 1583 | 220 | 3486 | 0 | 156 | 3536 | 0 |
| Satd．Flow（RTOR） |  | 21 |  |  |  | 140 |  | 13 |  |  | 1 |  |
| Lane Group Flow（vph） | 4 | 28 | 0 | 157 | 12 | 258 | 17 | 1580 | 0 | 191 | 1631 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | $\mathrm{pt}+\mathrm{v}$ | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 81 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 18.0 |  | 16.0 | 21.0 |  | 13.0 | 83.0 |  | 23.0 | 93.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 |  | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 11.9 | 7.6 |  | 21.3 | 16.8 | 34.8 | 98.0 | 90.2 |  | 110.7 | 104.3 |  |
| Actuated g／C Ratio | 0.08 | 0.05 |  | 0.15 | 0.12 | 0.25 | 0.70 | 0.64 |  | 0.79 | 0.74 |  |
| v／c Ratio | 0.03 | 0.26 |  | 0.75 | 0.05 | 0.52 | 0.08 | 0.70 |  | 0.67 | 0.62 |  |
| Control Delay | 46.2 | 35.1 |  | 76.5 | 53.2 | 22.4 | 1.8 | 4.7 |  | 29.3 | 11.5 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 |  | 0.0 | 0.0 |  |
| Total Delay | 46.2 | 35.1 |  | 76.5 | 53.2 | 22.4 | 1.8 | 5.0 |  | 29.3 | 11.5 |  |
| LOS | D | D |  | E | D | C | A | A |  | C | B |  |
| Approach Delay |  | 36.5 |  |  | 43.2 |  |  | 5.0 |  |  | 13.4 |  |
| Approach LOS |  | D |  |  | D |  |  | A |  |  | B |  |
| Queue Length 50th（ft） | 3 | 6 |  | 130 | 9 | 84 | 0 | 43 |  | 66 | 332 |  |
| Queue Length 95th（ft） | 14 | 39 |  | 200 | 31 | 174 | m1 | 83 |  | 152 | 555 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ ft ） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 178 | 154 |  | 211 | 245 | 540 | 259 | 2249 |  | 342 | 2634 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 169 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.02 | 0.18 |  | 0.74 | 0.05 | 0.48 | 0.07 | 0.76 |  | 0.56 | 0.62 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 0 （0\％），Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.75 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 13.4 |  |  |  | Intersection LOS：B |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 79．3\％ |  |  |  | ICU Level of Service D |  |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：11：Randall Rd \＆Prairie St


1: Randall Rd \& IL Route 38


Splits and Phases: 1: Randall Rd \& IL Route 38


Prairie Centre 4:30 pm 4/12/2016 2026 Total Traffic without Improvements

|  | 4 | $\rightarrow$ |  |  | 4 |  | 4 | $\uparrow$ |  | * | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个t |  | \% | 蚛 |  | \% | $\hat{1}$ |  | \% | $\uparrow$ | F |
| Volume (vph) | 212 | 650 | 51 | 63 | 1078 | 54 | 53 | 2 | 66 | 73 | 4 | 167 |
| Satd. Flow (prot) | 1770 | 3500 | 0 | 1770 | 3514 | 0 | 1770 | 1591 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.173 |  |  | 0.363 |  |  | 0.755 |  |  | 0.709 |  |  |
| Satd. Flow (perm) | 322 | 3500 | 0 | 676 | 3514 | 0 | 1406 | 1591 | 0 | 1321 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 13 |  |  | 6 |  |  | 72 |  |  |  | 182 |
| Lane Group Flow (vph) | 230 | 762 | 0 | 68 | 1231 | 0 | 58 | 74 | 0 | 79 | 4 | 182 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | Perm | NA |  | Perm | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 25.0 | 82.0 |  | 13.0 | 70.0 |  | 25.0 | 25.0 |  | 25.0 | 25.0 | 25.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |
| Act Effct Green (s) | 95.8 | 85.1 |  | 88.7 | 79.2 |  | 14.2 | 14.2 |  | 14.2 | 14.2 | 14.2 |
| Actuated g/C Ratio | 0.80 | 0.71 |  | 0.74 | 0.66 |  | 0.12 | 0.12 |  | 0.12 | 0.12 | 0.12 |
| v/c Ratio | 0.60 | 0.31 |  | 0.12 | 0.53 |  | 0.35 | 0.29 |  | 0.51 | 0.02 | 0.52 |
| Control Delay | 10.3 | 7.5 |  | 1.6 | 3.6 |  | 53.2 | 13.9 |  | 60.0 | 44.0 | 12.3 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 10.3 | 7.5 |  | 1.6 | 3.6 |  | 53.2 | 13.9 |  | 60.0 | 44.0 | 12.3 |
| LOS | B | A |  | A | A |  | D | B |  | E | D | B |
| Approach Delay |  | 8.2 |  |  | 3.5 |  |  | 31.2 |  |  | 27.0 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Queue Length 50th (ft) | 33 | 107 |  | 3 | 61 |  | 42 | 1 |  | 58 | 3 | 0 |
| Queue Length 95th (ft) | 69 | 161 |  | m5 | 82 |  | 82 | 45 |  | 106 | 13 | 64 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length ( ft ) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 517 | 2486 |  | 602 | 2320 |  | 222 | 312 |  | 209 | 294 | 403 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.44 | 0.31 |  | 0.11 | 0.53 |  | 0.26 | 0.24 |  | 0.38 | 0.01 | 0.45 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $13(11 \%)$, Referenced to phase 2:EBTL and $6: W B T L$, Start of 1 st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.60
Intersection Signal Delay: 8.9
Intersection LOS: A
Intersection Capacity Utilization $67.7 \%$ ICU Level of Service C
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个t |  | \% | 个t |  | 7 | $\uparrow$ |  | 7 | $\uparrow$ |  |
| Volume (vph) | 103 | 677 | 38 | 241 | 1052 | 81 | 67 | 171 | 207 | 149 | 208 | 56 |
| Satd. Flow (prot) | 1770 | 3511 | 0 | 1770 | 3500 | 0 | 1770 | 1710 | 0 | 1770 | 1803 | 0 |
| Flt Permitted | 0.099 |  |  | 0.219 |  |  | 0.433 |  |  | 0.163 |  |  |
| Satd. Flow (perm) | 184 | 3511 | 0 | 408 | 3500 | 0 | 807 | 1710 | 0 | 304 | 1803 | 0 |
| Satd. Flow (RTOR) |  | 5 |  |  | 8 |  |  | 49 |  |  | 11 |  |
| Lane Group Flow (vph) | 112 | 777 | 0 | 262 | 1231 | 0 | 73 | 411 | 0 | 162 | 287 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 46.0 |  | 23.0 | 56.0 |  | 13.0 | 38.0 |  | 13.0 | 38.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Efft Green (s) | 57.6 | 46.1 |  | 67.3 | 52.4 |  | 40.8 | 30.1 |  | 43.4 | 33.3 |  |
| Actuated g/C Ratio | 0.48 | 0.38 |  | 0.56 | 0.44 |  | 0.34 | 0.25 |  | 0.36 | 0.28 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.56 | 0.57 |  | 0.66 | 0.80 |  | 0.21 | 0.88 |  | 0.72 | 0.57 |  |
| Control Delay | 32.5 | 26.9 |  | 22.7 | 34.9 |  | 25.0 | 59.1 |  | 44.5 | 41.2 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 32.5 | 26.9 |  | 22.7 | 34.9 |  | 25.0 | 59.1 |  | 44.5 | 41.2 |  |
| LOS | C | C |  | C | C |  | C | E |  | D | D |  |
| Approach Delay |  | 27.6 |  |  | 32.7 |  |  | 54.0 |  |  | 42.4 |  |
| Approach LOS |  | C |  |  | C |  |  | D |  |  | D |  |
| Queue Length 50th (ft) | 38 | 187 |  | 104 | 441 |  | 35 | 269 |  | 82 | 184 |  |
| Queue Length 95th (ft) | 94 | 231 |  | 156 | 541 |  | 67 | \#437 |  | \#145 | 280 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 215 | 1352 |  | 450 | 1532 |  | 359 | 491 |  | 225 | 507 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.52 | 0.57 |  | 0.58 | 0.80 |  | 0.20 | 0.84 |  | 0.72 | 0.57 |  |

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $12(10 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.88
Intersection Signal Delay: 35.8
Intersection LOS: D
Intersection Capacity Utilization 84.4\%
ICU Level of Service E
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 3: 14th St \& IL Route 38


|  | 4 | $\rightarrow$ |  |  |  |  | 4 | 4 | 7 | $\checkmark$ | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | F |  | ${ }^{7}$ | $\uparrow$ | 「 | \％ | 个 ${ }^{\text {a }}$ |  | ${ }^{7}$ | 个 ${ }^{\text {¢ }}$ |  |
| Volume（vph） | 70 | 34 | 112 | 241 | 38 | 310 | 68 | 1638 | 222 | 244 | 1554 | 35 |
| Satd．Flow（prot） | 1770 | 1649 | 0 | 1770 | 1863 | 1583 | 1770 | 3476 | 0 | 1770 | 3529 | 0 |
| FIt Permitted | 0.730 |  |  | 0.301 |  |  | 0.074 |  |  | 0.050 |  |  |
| Satd．Flow（perm） | 1360 | 1649 | 0 | 561 | 1863 | 1583 | 138 | 3476 | 0 | 93 | 3529 | 0 |
| Satd．Flow（RTOR） |  | 90 |  |  |  | 64 |  | 17 |  |  | 3 |  |
| Lane Group Flow（vph） | 76 | 159 | 0 | 262 | 41 | 337 | 74 | 2021 | 0 | 265 | 1727 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | pm＋ov | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 15.0 |  | 20.0 | 22.0 | 21.0 | 13.0 | 84.0 |  | 21.0 | 92.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 19.3 | 8.5 |  | 31.0 | 18.2 | 41.7 | 86.9 | 77.5 |  | 101.0 | 89.5 |  |
| Actuated g／C Ratio | 0.14 | 0.06 |  | 0.22 | 0.13 | 0.30 | 0.62 | 0.55 |  | 0.72 | 0.64 |  |
| v／c Ratio | 0.36 | 0.86 |  | 1.00 | 0.17 | 0.65 | 0.45 | 1.05 |  | 0.98 | 0.77 |  |
| Control Delay | 50.1 | 67.3 |  | 106.5 | 58.7 | 42.2 | 14.8 | 49.5 |  | 92.2 | 21.4 |  |
| Queue Delay | 0.0 | 0.5 |  | 0.0 | 0.0 | 0.0 | 0.0 | 5.6 |  | 0.0 | 0.1 |  |
| Total Delay | 50.1 | 67.8 |  | 106.5 | 58.7 | 42.2 | 14.8 | 55.1 |  | 92.2 | 21.5 |  |
| LOS | D | E |  | F | E | D | B | E |  | F | C |  |
| Approach Delay |  | 62.1 |  |  | 69.6 |  |  | 53.7 |  |  | 30.9 |  |
| Approach LOS |  | E |  |  | E |  |  | D |  |  | C |  |
| Queue Length 50th（ft） | 57 | 63 |  | 220 | 35 | 226 | 14 | $\sim 1027$ |  | 190 | 570 |  |
| Queue Length 95th（ft） | 104 | \＃195 |  | \＃369 | 73 | 341 | m21 | \＃1170 |  | \＃375 | 702 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ t ） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 220 | 184 |  | 262 | 242 | 516 | 192 | 1932 |  | 270 | 2257 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 26 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 1 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 45 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.35 | 0.87 |  | 1.00 | 0.17 | 0.65 | 0.39 | 1.06 |  | 0.98 | 0.78 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 0 （0\％），Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 1.05 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 47.0 |  |  |  |  | Intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 105．4\％ <br> ICU Level of Service G Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |



1：Randall Rd \＆IL Route 38

|  |  |  |  |  |  |  |  | $\dagger$ | 7 |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1 *}$ | 个个 | 「 | \％${ }^{1 / 8}$ | 个个 | 「 | \％${ }^{1 / 4}$ | 个个 | 「 | ${ }^{*}{ }^{1 \%}$ | 个个 | F |
| Volume（vph） | 394 | 506 | 118 | 328 | 384 | 372 | 116 | 1251 | 205 | 380 | 1506 | 301 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Flt Permitted | 0.214 |  |  | 0.242 |  |  | 0.073 |  |  | 0.068 |  |  |
| Satd．Flow（perm） | 773 | 3539 | 1583 | 875 | 3539 | 1583 | 264 | 3539 | 1583 | 246 | 3539 | 1583 |
| Satd．Flow（RTOR） |  |  | 114 |  |  | 73 |  |  | 114 |  |  | 90 |
| Lane Group Flow（vph） | 428 | 550 | 128 | 357 | 417 | 404 | 126 | 1360 | 223 | 413 | 1637 | 327 |
| Turn Type | pm＋pt | NA | pm＋ov | pm＋pt | NA | pm＋ov | pm＋pt | NA | pm＋ov | pm＋pt | NA | $\mathrm{pm}+\mathrm{ov}$ |
| Protected Phases | 5 | 2 | 3 | 1 | 6 | 7 | 3 | ， | 1 | 7 | 4 | 5 |
| Permitted Phases | 2 |  | 2 | 6 |  | 6 | 8 |  | 8 | 4 |  | 4 |
| Total Split（s） | 18.0 | 27.0 | 13.0 | 14.0 | 23.0 | 19.0 | 13.0 | 60.0 | 14.0 | 19.0 | 66.0 | 18.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Effct Green（s） | 36.0 | 20.5 | 34.2 | 28.0 | 16.5 | 36.4 | 63.8 | 54.6 | 70.6 | 74.4 | 60.8 | 80.8 |
| Actuated g／C Ratio | 0.30 | 0.17 | 0.28 | 0.23 | 0.14 | 0.30 | 0.53 | 0.46 | 0.59 | 0.62 | 0.51 | 0.67 |
| v／c Ratio | 0.81 | 0.91 | 0.24 | 0.88 | 0.86 | 0.76 | 0.38 | 0.84 | 0.23 | 0.81 | 0.91 | 0.30 |
| Control Delay | 46.5 | 69.4 | 8.7 | 58.0 | 68.4 | 41.1 | 13.7 | 35.3 | 6.3 | 45.2 | 20.1 | 1.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.2 | 0.0 | 34.3 | 0.0 | 0.0 | 5.7 | 0.0 |
| Total Delay | 46.5 | 69.4 | 8.7 | 58.0 | 68.4 | 41.2 | 13.7 | 69.5 | 6.3 | 45.2 | 25.8 | 1.7 |
| LOS | D | E | A | E | E | D | B | E | A | D | C | A |
| Approach Delay |  | 53.5 |  |  | 55.9 |  |  | 57.2 |  |  | 25.9 |  |
| Approach LOS |  | D |  |  | E |  |  | E |  |  | C |  |
| Queue Length 50th（ft） | 135 | 222 | 8 | 110 | 168 | 232 | 18 | 488 | 36 | 109 | 411 | 7 |
| Queue Length 95th（ft） | \＃193 | \＃324 | 54 | \＃167 | \＃252 | 357 | 30 | 591 | 75 | m124 | m532 | m9 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ ft ） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 531 | 604 | 548 | 406 | 487 | 544 | 367 | 1611 | 978 | 537 | 1793 | 1095 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 127 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 337 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.81 | 0.91 | 0.23 | 0.88 | 0.86 | 0.75 | 0.34 | 1.07 | 0.23 | 0.77 | 0.98 | 0.30 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 13 （11\％），Referenced to phase 4：SBTL and 8：NBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.91 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 44.6 |  |  |  |  | Intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 87．5\％ |  |  |  |  | ICU Level of Service E |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


Prairie Centre 11：15 am 4／16／2016 2026 Total Traffic without Improvements Timing Plan：Sat

|  | $\rangle$ | $\rightarrow$ |  |  |  |  | 4 | $\dagger$ |  | $\checkmark$ | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个t |  | \% | 个 ${ }^{\text {a }}$ |  | 7 | $\uparrow$ |  | \% | $\uparrow$ | F |
| Volume (vph) | 216 | 729 | 65 | 39 | 781 | 55 | 53 | 1 | 70 | 81 | 8 | 189 |
| Satd. Flow (prot) | 1770 | 3497 | 0 | 1770 | 3504 | 0 | 1770 | 1587 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.261 |  |  | 0.328 |  |  | 0.752 |  |  | 0.707 |  |  |
| Satd. Flow (perm) | 486 | 3497 | 0 | 611 | 3504 | 0 | 1401 | 1587 | 0 | 1317 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 14 |  |  | 9 |  |  | 76 |  |  |  | 205 |
| Lane Group Flow (vph) | 235 | 863 | 0 | 42 | 909 | 0 | 58 | 77 | 0 | 88 | - | 205 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | Perm | NA |  | Perm | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 24.0 | 65.0 |  | 13.0 | 54.0 |  | 27.0 | 27.0 |  | 27.0 | 27.0 | 27.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 6.0 | 6.0 |  | 6.0 | 6.0 | 6.0 |
| Act Effct Green (s) | 80.8 | 71.8 |  | 74.0 | 64.9 |  | 14.7 | 14.7 |  | 14.7 | 14.7 | 14.7 |
| Actuated g/C Ratio | 0.77 | 0.68 |  | 0.70 | 0.62 |  | 0.14 | 0.14 |  | 0.14 | 0.14 | 0.14 |
| v/c Ratio | 0.48 | 0.36 |  | 0.08 | 0.42 |  | 0.30 | 0.27 |  | 0.48 | 0.03 | 0.52 |
| Control Delay | 7.2 | 8.6 |  | 2.3 | 5.6 |  | 42.6 | 11.2 |  | 49.1 | 36.1 | 10.2 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 7.2 | 8.6 |  | 2.3 | 5.6 |  | 42.6 | 11.2 |  | 49.1 | 36.1 | 10.2 |
| LOS | A | A |  | A | A |  | D | B |  | D | D | B |
| Approach Delay |  | 8.3 |  |  | 5.4 |  |  | 24.7 |  |  | 22.3 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Queue Length 50th (ft) | 33 | 124 |  | 3 | 61 |  | 36 | 1 |  | 56 | 5 | 0 |
| Queue Length 95th (ft) | 73 | 200 |  | m6 | 87 |  | 70 | 40 |  | 99 | 19 | 60 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length ( t ) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 624 | 2397 |  | 554 | 2167 |  | 280 | 378 |  | 263 | 372 | 480 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | O |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.38 | 0.36 |  | 0.08 | 0.42 |  | 0.21 | 0.20 |  | 0.33 | 0.02 | 0.43 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 105

Actuated Cycle Length: 105
Offset: 60 ( $57 \%$ ), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.52
Intersection Signal Delay: 9.8
Intersection LOS: A
Intersection Capacity Utilization 60.2\% ICU Level of Service B
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 性 |  | 7 | 个t |  | 7 | F |  | \% | F |  |
| Volume (vph) | 75 | 755 | 43 | 268 | 784 | 60 | 55 | 130 | 254 | 131 | 166 | 32 |
| Satd. Flow (prot) | 1770 | 3511 | 0 | 1770 | 3500 | 0 | 1770 | 1678 | 0 | 1770 | 1818 | 0 |
| Flt Permitted | 0.245 |  |  | 0.144 |  |  | 0.587 |  |  | 0.159 |  |  |
| Satd. Flow (perm) | 456 | 3511 | 0 | 268 | 3500 | 0 | 1093 | 1678 | 0 | 296 | 1818 | 0 |
| Satd. Flow (RTOR) |  | 6 |  |  | 8 |  |  | 92 |  |  | 9 |  |
| Lane Group Flow (vph) | 82 | 868 | 0 | 291 | 917 | 0 | 60 | 417 | 0 | 142 | 215 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 37.0 |  | 21.0 | 45.0 |  | 13.0 | 34.0 |  | 13.0 | 34.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Efft Green (s) | 45.7 | 34.9 |  | 56.9 | 44.5 |  | 35.9 | 25.8 |  | 39.4 | 29.3 |  |
| Actuated g/C Ratio | 0.44 | 0.33 |  | 0.54 | 0.42 |  | 0.34 | 0.25 |  | 0.38 | 0.28 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.28 | 0.74 |  | 0.79 | 0.62 |  | 0.14 | 0.87 |  | 0.59 | 0.42 |  |
| Control Delay | 14.5 | 32.8 |  | 35.3 | 27.2 |  | 19.9 | 48.1 |  | 31.0 | 32.8 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 14.5 | 32.8 |  | 35.3 | 27.2 |  | 19.9 | 48.1 |  | 31.0 | 32.8 |  |
| LOS | B | C |  | D | C |  | B | D |  | C | C |  |
| Approach Delay |  | 31.2 |  |  | 29.2 |  |  | 44.5 |  |  | 32.1 |  |
| Approach LOS |  | C |  |  | C |  |  | D |  |  | C |  |
| Queue Length 50th ( ft ) | 24 | 191 |  | 115 | 265 |  | 24 | 208 |  | 60 | 112 |  |
| Queue Length 95th (ft) | 47 | \#271 |  | \#234 | 346 |  | 51 | \#367 |  | 102 | 186 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 324 | 1170 |  | 395 | 1487 |  | 455 | 514 |  | 244 | 513 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.25 | 0.74 |  | 0.74 | 0.62 |  | 0.13 | 0.81 |  | 0.58 | 0.42 |  |

## Intersection Summary

## Cycle Length: 105

Actuated Cycle Length: 105
Offset: $60(57 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.87
Intersection Signal Delay: 32.6
Intersection LOS: C
Intersection Capacity Utilization 83.9\%
ICU Level of Service E
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 3: 14th St \& IL Route 38


11：Randall Rd \＆Prairie St

|  | $\rangle$ |  |  |  |  |  |  | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个 |  | \％ | $\uparrow$ | 「 | ${ }^{17}$ | 性 |  | \％ | 中 ${ }^{\text {b }}$ |  |
| Volume（vph） | 62 | 38 | 117 | 226 | 41 | 370 | 60 | 1676 | 283 | 259 | 1884 | 28 |
| Satd．Flow（prot） | 1770 | 1652 | 0 | 1770 | 1863 | 1583 | 1770 | 3461 | 0 | 1770 | 3532 | 0 |
| FIt Permitted | 0.728 |  |  | 0.384 |  |  | 0.062 |  |  | 0.058 |  |  |
| Satd．Flow（perm） | 1356 | 1652 | 0 | 715 | 1863 | 1583 | 115 | 3461 | 0 | 108 | 3532 | 0 |
| Satd．Flow（RTOR） |  | 100 |  |  |  | 83 |  | 25 |  |  | 2 |  |
| Lane Group Flow（vph） | 67 | 168 | 0 | 246 | 45 | 402 | 65 | 2130 | 0 | 282 | 2078 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | pm＋ov | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 15.0 |  | 15.0 | 17.0 | 18.0 | 13.0 | 72.0 |  | 18.0 | 77.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 18.8 | 8.3 |  | 23.5 | 12.2 | 29.5 | 74.7 | 65.7 |  | 86.2 | 75.0 |  |
| Actuated g／C Ratio | 0.16 | 0.07 |  | 0.20 | 0.10 | 0.25 | 0.62 | 0.55 |  | 0.72 | 0.62 |  |
| v／c Ratio | 0.28 | 0.81 |  | 1.04 | 0.24 | 0.89 | 0.40 | 1.12 |  | 1.04 | 0.94 |  |
| Control Delay | 42.8 | 52.3 |  | 118.6 | 55.3 | 53.4 | 14.7 | 78.8 |  | 99.9 | 31.5 |  |
| Queue Delay | 0.0 | 0.5 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 4.6 |  |
| Total Delay | 42.8 | 52.8 |  | 118.6 | 55.3 | 53.4 | 14.7 | 78.8 |  | 99.9 | 36.0 |  |
| LOS | D | D |  | F | E | D | B | E |  | F | D |  |
| Approach Delay |  | 49.9 |  |  | 76.7 |  |  | 76.9 |  |  | 43.6 |  |
| Approach LOS |  | D |  |  | E |  |  | E |  |  | D |  |
| Queue Length 50th（ft） | 43 | 52 |  | ～187 | 34 | 203 | 12 | ～981 |  | ～182 | 750 |  |
| Queue Length 95th（ft） | 84 | \＃169 |  | \＃368 | 72 | \＃447 | m17 | \＃1127 |  | \＃359 | \＃1017 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ ft ） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 245 | 209 |  | 236 | 200 | 451 | 197 | 1905 |  | 271 | 2208 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 3 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 2 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 101 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.27 | 0.81 |  | 1.04 | 0.23 | 0.89 | 0.33 | 1.12 |  | 1.04 | 0.99 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 0 （0\％），Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 1.12 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 61.4 |  |  |  |  | Intersection LOS：E |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 108．9\％ |  |  |  |  | ICU Level of Service G |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| ～Volume exceeds capacity，queue is theoretically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |



## APPENDIX H

## Synchro Analysis Reports <br> 2026 Total Traffic <br> With Improvements

1. Weekday AM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
2. Weekday PM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
3. Saturday Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street

|  | $\rangle$ |  |  |  |  |  | 4 | 4 | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％${ }^{17}$ | 个4 | F | \％${ }^{1 / 4}$ | 个4 | F | ${ }^{7 *}$ | 个4 | F | \％${ }^{*}$ | 个4 | F |
| Volume（vph） | 242 | 623 | 39 | 181 | 300 | 208 | 98 | 1012 | 129 | 365 | 1013 | 255 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Satd．Flow（RTOR） |  |  | 155 |  |  | 219 |  |  | 155 |  |  | 135 |
| Lane Group Flow（vph） | 263 | 677 | 42 | 197 | 326 | 226 | 107 | 1100 | 140 | 397 | 1101 | 277 |
| Turn Type | Prot | NA | Prot | Prot | NA | Prot | Prot | NA | pt＋ov | Prot | NA | pt＋ov |
| Protected Phases | 5 | 2 | 2 | 1 | 6 | 6 | 3 | 8 | 81 | 7 | ， | 45 |
| Permitted Phases |  |  |  |  |  |  |  |  |  |  |  |  |
| Total Split（s） | 19.0 | 34.0 | 34.0 | 14.0 | 29.0 | 29.0 | 13.0 | 51.0 |  | 21.0 | 59.0 |  |
| Total Lost Time（s） | 4.5 | 6.5 | 6.5 | 4.5 | 6.5 | 6.5 | 4.5 | 6.5 |  | 4.5 | 6.5 |  |
| Act Effct Green（s） | 13.3 | 27.6 | 27.6 | 9.4 | 23.7 | 23.7 | 8.0 | 44.8 | 58.7 | 16.2 | 53.0 | 70.8 |
| Actuated g／C Ratio | 0.11 | 0.23 | 0.23 | 0.08 | 0.20 | 0.20 | 0.07 | 0.37 | 0.49 | 0.14 | 0.44 | 0.59 |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.69 | 0.83 | 0.09 | 0.74 | 0.47 | 0.46 | 0.47 | 0.83 | 0.16 | 0.86 | 0.70 | 0.28 |
| Control Delay | 61.1 | 54.1 | 0.4 | 61.6 | 50.7 | 17.9 | 60.7 | 40.9 | 2.4 | 73.6 | 25.7 | 3.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.6 | 0.0 | 0.0 | 0.2 | 0.0 |
| Total Delay | 61.1 | 54.1 | 0.4 | 61.6 | 50.7 | 17.9 | 60.7 | 41.5 | 2.4 | 73.6 | 25.9 | 3.6 |
| LOS | E | D | A | E | D | B | E | D | A | E | C | A |
| Approach Delay |  | 53.7 |  |  | 43.7 |  |  | 39.0 |  |  | 33.1 |  |
| Approach LOS |  | D |  |  | D |  |  | D |  |  | C |  |
| Queue Length 50th（ft） | 101 | 264 | 0 | 79 | 135 | 39 | 41 | 405 | 0 | 165 | 337 | 17 |
| Queue Length 95th（ft） | 146 | \＃342 | 0 | \＃129 | 182 | 115 | 71 | 496 | 27 | \＃242 | 294 | 36 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ t ） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 414 | 814 | 483 | 271 | 697 | 487 | 243 | 1322 | 855 | 472 | 1562 | 1003 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | ， | 65 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 46 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.64 | 0.83 | 0.09 | 0.73 | 0.47 | 0.47 | 0.44 | 0.86 | 0.16 | 0.84 | 0.74 | 0.28 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 10 （8\％），Referenced to phase 4：SBT and 8：NBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.86 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 40.5 |  |  |  | Intersection LOS：D |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 79．1\％ |  |  |  | ICU Level of Service D |  |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maxim | after two | cycles． |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


|  | $\rangle$ | $\rightarrow$ |  |  | 4 |  | 4 | $\dagger$ |  | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个t |  | \% | 性 |  | \% | $\hat{\square}$ |  | \% | $\uparrow$ | F |
| Volume (vph) | 182 | 879 | 74 | 32 | 480 | 46 | 36 | 2 | 53 | 79 | 1 | 194 |
| Satd. Flow (prot) | 1770 | 3497 | 0 | 1770 | 3493 | 0 | 1770 | 1593 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.400 |  |  | 0.255 |  |  | 0.757 |  |  | 0.586 |  |  |
| Satd. Flow (perm) | 745 | 3497 | 0 | 475 | 3493 | 0 | 1410 | 1593 | 0 | 1092 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 10 |  |  | 11 |  |  | 58 |  |  |  | 211 |
| Lane Group Flow (vph) | 198 | 1035 | 0 | 35 | 572 | 0 | 39 | 60 | 0 | 86 | 1 | 211 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 18.0 | 65.0 |  | 13.0 | 60.0 |  | 13.0 | 29.0 |  | 13.0 | 29.0 | 29.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 | 6.0 |
| Act Effct Green (s) | 90.1 | 81.1 |  | 83.2 | 74.0 |  | 19.0 | 10.0 |  | 21.3 | 13.1 | 13.1 |
| Actuated g/C Ratio | 0.75 | 0.68 |  | 0.69 | 0.62 |  | 0.16 | 0.08 |  | 0.18 | 0.11 | 0.11 |
| v/c Ratio | 0.31 | 0.44 |  | 0.09 | 0.26 |  | 0.16 | 0.32 |  | 0.35 | 0.00 | 0.58 |
| Control Delay | 1.5 | 2.8 |  | 5.1 | 9.1 |  | 38.4 | 17.7 |  | 43.1 | 47.0 | 13.5 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 1.5 | 2.8 |  | 5.1 | 9.1 |  | 38.4 | 17.7 |  | 43.1 | 47.0 | 13.5 |
| LOS | A | A |  | A | A |  | D | B |  | D | D | B |
| Approach Delay |  | 2.6 |  |  | 8.8 |  |  | 25.9 |  |  | 22.2 |  |
| Approach LOS |  | A |  |  | A |  |  | C |  |  | C |  |
| Queue Length 50th (ft) | 5 | 20 |  | 6 | 65 |  | 25 | 2 |  | 57 | 1 | 0 |
| Queue Length 95th (ft) | m8 | 308 |  | 14 | 98 |  | 52 | 42 |  | 96 | 6 | 71 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length ( t ) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 683 | 2366 |  | 445 | 2159 |  | 265 | 352 |  | 247 | 357 | 473 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.29 | 0.44 |  | 0.08 | 0.26 |  | 0.15 | 0.17 |  | 0.35 | 0.00 | 0.45 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $116(97 \%)$, Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.58
Intersection Signal Delay: 7.9
Intersection LOS: A
Intersection Capacity Utilization 54.8\%
ICU Level of Service A
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 2: IL Route $38 \&$ W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个t |  | \% | 中 ${ }^{\text {d }}$ |  | 7 | $\uparrow$ |  | \% | F |  |
| Volume (vph) | 73 | 966 | 19 | 95 | 495 | 47 | 24 | 85 | 221 | 100 | 98 | 34 |
| Satd. Flow (prot) | 1770 | 3529 | 0 | 1770 | 3493 | 0 | 1770 | 1662 | 0 | 1770 | 1790 | 0 |
| Flt Permitted | 0.397 |  |  | 0.162 |  |  | 0.665 |  |  | 0.192 |  |  |
| Satd. Flow (perm) | 740 | 3529 | 0 | 302 | 3493 | 0 | 1239 | 1662 | 0 | 358 | 1790 | 0 |
| Satd. Flow (RTOR) |  | 2 |  |  | 10 |  |  | 106 |  |  | 14 |  |
| Lane Group Flow (vph) | 79 | 1071 | 0 | 103 | 589 | 0 | 26 | 332 | 0 | 109 | 144 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 57.0 |  | 13.0 | 57.0 |  | 13.0 | 37.0 |  | 13.0 | 37.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Efft Green (s) | 70.0 | 59.3 |  | 71.6 | 61.6 |  | 33.1 | 23.9 |  | 38.2 | 30.0 |  |
| Actuated g/C Ratio | 0.58 | 0.49 |  | 0.60 | 0.51 |  | 0.28 | 0.20 |  | 0.32 | 0.25 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.16 | 0.61 |  | 0.37 | 0.33 |  | 0.07 | 0.80 |  | 0.49 | 0.31 |  |
| Control Delay | 4.3 | 7.8 |  | 14.7 | 19.3 |  | 25.3 | 44.8 |  | 35.1 | 34.7 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 4.3 | 7.8 |  | 14.7 | 19.3 |  | 25.3 | 44.8 |  | 35.1 | 34.7 |  |
| LOS | A | A |  | B | B |  | C | D |  | D | C |  |
| Approach Delay |  | 7.6 |  |  | 18.6 |  |  | 43.4 |  |  | 34.9 |  |
| Approach LOS |  | A |  |  | B |  |  | D |  |  | C |  |
| Queue Length 50th (ft) | 9 | 76 |  | 31 | 140 |  | 14 | 170 |  | 60 | 85 |  |
| Queue Length 95th (ft) | 20 | 105 |  | 64 | 210 |  | 32 | 262 |  | 96 | 138 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 524 | 1744 |  | 298 | 1798 |  | 412 | 507 |  | 225 | 484 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.15 | 0.61 |  | 0.35 | 0.33 |  | 0.06 | 0.65 |  | 0.48 | 0.30 |  |

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $6(5 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.80
Intersection Signal Delay: $18.7 \quad$ Intersection LOS: B
Intersection Capacity Utilization 73.3\% ICU Level of Service D
Analysis Period (min) 15
Splits and Phases: 3: 14th St \& IL Route 38


|  | 4 | $\rightarrow$ |  |  | 4 |  |  | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊ | F |  | ${ }^{17}$ | $\uparrow$ | 「 | ${ }^{1}$ | 性 |  | \％ | 性 |  |
| Volume（vph） | 4 | 6 | 19 | 144 | 11 | 237 | 16 | 1307 | 146 | 176 | 1489 | 12 |
| Satd．Flow（prot） | 1770 | 1652 | 0 | 1770 | 1863 | 1583 | 1770 | 3486 | 0 | 1770 | 3536 | 0 |
| Flt Permitted | 0.816 |  |  | 0.739 |  |  | 0.115 |  |  | 0.082 |  |  |
| Satd．Flow（perm） | 1520 | 1652 | 0 | 1377 | 1863 | 1583 | 214 | 3486 | 0 | 153 | 3536 | 0 |
| Satd．Flow（RTOR） |  | 21 |  |  |  | 169 |  | 15 |  |  | 1 |  |
| Lane Group Flow（vph） | 4 | 28 | 0 | 157 | 12 | 258 | 17 | 1580 | 0 | 191 | 1631 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | $\mathrm{pt}+\mathrm{v}$ | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 81 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 18.0 |  | 14.0 | 19.0 |  | 13.0 | 70.0 |  | 18.0 | 75.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 |  | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 11.7 | 7.5 |  | 17.0 | 12.6 | 27.2 | 84.1 | 76.4 |  | 94.8 | 88.4 |  |
| Actuated g／C Ratio | 0.10 | 0.06 |  | 0.14 | 0.10 | 0.23 | 0.70 | 0.64 |  | 0.79 | 0.74 |  |
| v／c Ratio | 0.03 | 0.23 |  | 0.71 | 0.06 | 0.53 | 0.08 | 0.71 |  | 0.68 | 0.63 |  |
| Control Delay | 38.5 | 30.0 |  | 65.4 | 47.5 | 16.1 | 1.9 | 4.9 |  | 28.8 | 11.0 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.3 |  | 0.0 | 0.0 |  |
| Total Delay | 38.5 | 30.0 |  | 65.4 | 47.5 | 16.1 | 1.9 | 5.2 |  | 28.8 | 11.0 |  |
| LOS | D | C |  | E | D | B | A | A |  | C | B |  |
| Approach Delay |  | 31.1 |  |  | 35.1 |  |  | 5.2 |  |  | 12.8 |  |
| Approach LOS |  | C |  |  | D |  |  | A |  |  | B |  |
| Queue Length 50th（ft） | 3 | 5 |  | 114 | 8 | 53 | 1 | 37 |  | 54 | 253 |  |
| Queue Length 95th（ft） | 12 | 35 |  | 173 | 28 | 118 | m2 | 68 |  | 144 | 533 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ft） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 213 | 177 |  | 251 | 222 | 503 | 273 | 2223 |  | 309 | 2605 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 184 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.02 | 0.16 |  | 0.63 | 0.05 | 0.51 | 0.06 | 0.77 |  | 0.62 | 0.63 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length： 120

Actuated Cycle Length： 120
Offset： $24(20 \%)$ ，Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green
Control Type：Actuated－Coordinated
Maximum v／c Ratio： 0.71
Intersection Signal Delay： $12.3 \quad$ Intersection LOS：B
Intersection Capacity Utilization 79．3\％ICU Level of Service D
Analysis Period（min） 15
m Volume for 95 th percentile queue is metered by upstream signal．
Splits and Phases：11：Randall Rd \＆Prairie St


1：Randall Rd \＆IL Route 38

|  |  |  |  |  |  |  | 4 | $\dagger$ | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | 7\％ | 个个 | F | ${ }^{7}$ | 个个 | 「 | \％${ }^{1+1}$ | 个个 | 「 | \％${ }^{*}$ | 个4 | F |
| Volume（vph） | 298 | 408 | 87 | 251 | 616 | 431 | 111 | 1250 | 176 | 317 | 1231 | 290 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Satd．Flow（RTOR） |  |  | 73 |  |  | 73 |  |  | 73 |  |  | 73 |
| Lane Group Flow（vph） | 324 | 443 | 95 | 273 | 670 | 468 | 121 | 1359 | 191 | 345 | 1338 | 315 |
| Turn Type | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | $\mathrm{pm}+\mathrm{ov}$ |
| Protected Phases | 5 | 2 | 3 | 1 | ， | 7 | 3 | 8 | 1 | 7 | 4 | 5 |
| Permitted Phases |  |  | 2 |  |  | 6 |  |  | 8 |  |  | 4 |
| Total Split（s） | 16.0 | 30.0 | 13.0 | 16.0 | 30.0 | 17.0 | 13.0 | 57.0 | 16.0 | 17.0 | 61.0 | 16.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Efft Green（s） | 11.5 | 23.5 | 38.2 | 11.5 | 23.5 | 42.5 | 8.2 | 50.5 | 68.5 | 12.5 | 54.8 | 72.8 |
| Actuated g／C Ratio | 0.10 | 0.20 | 0.32 | 0.10 | 0.20 | 0.35 | 0.07 | 0.42 | 0.57 | 0.10 | 0.46 | 0.61 |
| v／c Ratio | 0.99 | 0.64 | 0.17 | 0.83 | 0.97 | 0.77 | 0.52 | 0.91 | 0.20 | 0.97 | 0.83 | 0.32 |
| Control Delay | 101.1 | 49.2 | 10.6 | 62.1 | 61.1 | 37.4 | 62.1 | 43.2 | 8.1 | 86.3 | 20.0 | 6.9 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 35.6 | 0.0 | 0.0 | 0.7 | 0.0 |
| Total Delay | 101.1 | 49.2 | 10.6 | 62.1 | 61.1 | 37.5 | 62.1 | 78.8 | 8.1 | 86.3 | 20.7 | 6.9 |
| LOS | F | D | B | E | E | D | E | E | A | F | C | A |
| Approach Delay |  | 64.4 |  |  | 53.5 |  |  | 69.5 |  |  | 29.8 |  |
| Approach LOS |  | E |  |  | D |  |  | E |  |  | C |  |
| Queue Length 50th（ t ） | 131 | 167 | 12 | 98 | 280 | 338 | 47 | 512 | 40 | 131 | 494 | 82 |
| Queue Length 95th（ft） | \＃226 | 224 | 51 | \＃176 | \＃401 | 436 | 79 | \＃659 | 78 | m\＃197 | m468 | m103 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（tt） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 328 | 694 | 557 | 328 | 693 | 607 | 243 | 1489 | 934 | 357 | 1616 | 989 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 80 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 225 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.99 | 0.64 | 0.17 | 0.83 | 0.97 | 0.78 | 0.50 | 1.08 | 0.20 | 0.97 | 0.87 | 0.32 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 12 （10\％），Referenced to phase 4：SBT and 8：NBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.99 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 51.6 |  |  |  |  | ntersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 87．5\％ |  |  |  |  | ICU Level of Service E |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95th perc | queue | metere | d by upst | eam sig |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


Prairie Centre 4：30 pm 4／12／2016 2026 Total Traffic with Improvements

|  | $\rangle$ | $\rightarrow$ |  |  | 4 |  | 4 | $\dagger$ |  | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 中 ${ }^{\text {¢ }}$ |  | \% | 性 |  | \% | $\hat{\square}$ |  | \% | $\uparrow$ | F |
| Volume (vph) | 212 | 650 | 51 | 63 | 1078 | 54 | 53 | 2 | 66 | 73 | 4 | 167 |
| Satd. Flow (prot) | 1770 | 3500 | 0 | 1770 | 3514 | 0 | 1770 | 1591 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.150 |  |  | 0.363 |  |  | 0.755 |  |  | 0.681 |  |  |
| Satd. Flow (perm) | 279 | 3500 | 0 | 676 | 3514 | 0 | 1406 | 1591 | 0 | 1269 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 11 |  |  | , |  |  | 72 |  |  |  | 182 |
| Lane Group Flow (vph) | 230 | 762 | 0 | 68 | 1231 | 0 | 58 | 74 | 0 | 79 | 4 | 182 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 24.0 | 73.0 |  | 13.0 | 62.0 |  | 13.0 | 21.0 |  | 13.0 | 21.0 | 21.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 | 6.0 |
| Act Effct Green (s) | 90.3 | 78.9 |  | 80.7 | 70.9 |  | 19.5 | 9.9 |  | 20.2 | 10.3 | 10.3 |
| Actuated g/C Ratio | 0.75 | 0.66 |  | 0.67 | 0.59 |  | 0.16 | 0.08 |  | 0.17 | 0.09 | 0.09 |
| v/c Ratio | 0.62 | 0.33 |  | 0.13 | 0.59 |  | 0.23 | 0.38 |  | 0.32 | 0.03 | 0.60 |
| Control Delay | 14.9 | 14.8 |  | 2.4 | 6.9 |  | 40.3 | 17.4 |  | 42.4 | 48.5 | 16.0 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 14.9 | 14.8 |  | 2.4 | 6.9 |  | 40.3 | 17.4 |  | 42.4 | 48.5 | 16.0 |
| LOS | B | B |  | A | A |  | D | B |  | D | D | B |
| Approach Delay |  | 14.8 |  |  | 6.7 |  |  | 27.4 |  |  | 24.4 |  |
| Approach LOS |  | B |  |  | A |  |  | C |  |  | C |  |
| Queue Length 50th (ft) | 85 | 214 |  | 4 | 83 |  | 38 | 2 |  | 52 | 3 | 0 |
| Queue Length 95th (ft) | m119 | m340 |  | m8 | 130 |  | 71 | 47 |  | 90 | 14 | 67 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length ( t ) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 464 | 2303 |  | 556 | 2077 |  | 263 | 261 |  | 255 | 232 | 357 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.50 | 0.33 |  | 0.12 | 0.59 |  | 0.22 | 0.28 |  | 0.31 | 0.02 | 0.51 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $19(16 \%)$, Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.62
Intersection Signal Delay: 12.4 Intersection LOS: B
Intersection Capacity Utilization $67.7 \%$ ICU Level of Service $C$
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个t |  | \% | 个t |  | 7 | $\uparrow$ |  | 7 | $\uparrow$ |  |
| Volume (vph) | 103 | 677 | 38 | 241 | 1052 | 81 | 67 | 171 | 207 | 149 | 208 | 56 |
| Satd. Flow (prot) | 1770 | 3511 | 0 | 1770 | 3500 | 0 | 1770 | 1710 | 0 | 1770 | 1803 | 0 |
| Flt Permitted | 0.099 |  |  | 0.219 |  |  | 0.433 |  |  | 0.163 |  |  |
| Satd. Flow (perm) | 184 | 3511 | 0 | 408 | 3500 | 0 | 807 | 1710 | 0 | 304 | 1803 | 0 |
| Satd. Flow (RTOR) |  | 5 |  |  | 8 |  |  | 49 |  |  | 11 |  |
| Lane Group Flow (vph) | 112 | 777 | 0 | 262 | 1231 | 0 | 73 | 411 | 0 | 162 | 287 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 46.0 |  | 23.0 | 56.0 |  | 13.0 | 38.0 |  | 13.0 | 38.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Efft Green (s) | 57.6 | 46.1 |  | 67.3 | 52.4 |  | 40.8 | 30.1 |  | 43.4 | 33.3 |  |
| Actuated g/C Ratio | 0.48 | 0.38 |  | 0.56 | 0.44 |  | 0.34 | 0.25 |  | 0.36 | 0.28 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.56 | 0.57 |  | 0.66 | 0.80 |  | 0.21 | 0.88 |  | 0.72 | 0.57 |  |
| Control Delay | 39.6 | 19.3 |  | 22.7 | 34.9 |  | 25.0 | 59.1 |  | 44.5 | 41.2 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 39.6 | 19.3 |  | 22.7 | 34.9 |  | 25.0 | 59.1 |  | 44.5 | 41.2 |  |
| LOS | D | B |  | C | C |  | C | E |  | D | D |  |
| Approach Delay |  | 21.8 |  |  | 32.7 |  |  | 54.0 |  |  | 42.4 |  |
| Approach LOS |  | C |  |  | C |  |  | D |  |  | D |  |
| Queue Length 50th (ft) | 42 | 98 |  | 104 | 441 |  | 35 | 269 |  | 82 | 184 |  |
| Queue Length 95th (ft) | 105 | 232 |  | 156 | 541 |  | 67 | \#437 |  | \#145 | 280 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 215 | 1352 |  | 450 | 1532 |  | 359 | 491 |  | 225 | 507 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.52 | 0.57 |  | 0.58 | 0.80 |  | 0.20 | 0.84 |  | 0.72 | 0.57 |  |

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $13(11 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.88
Intersection Signal Delay: 34.2
Intersection LOS: C
Intersection Capacity Utilization 84.4\%
ICU Level of Service E
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 3: 14th St \& IL Route 38


11：Randall Rd \＆Prairie St

|  |  |  |  |  |  |  |  | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ | 「 | ＊ | 个t |  | ${ }^{7}$ | 中 ${ }^{\text {c }}$ |  |
| Volume（vph） | 70 | 34 | 112 | 241 | 38 | 310 | 68 | 1638 | 222 | 244 | 1554 | 35 |
| Satd．Flow（prot） | 1770 | 1649 | 0 | 1770 | 1863 | 1583 | 1770 | 3476 | 0 | 1770 | 3529 | 0 |
| Flt Permitted | 0.730 |  |  | 0.417 |  |  | 0.081 |  |  | 0.069 |  |  |
| Satd．Flow（perm） | 1360 | 1649 | 0 | 777 | 1863 | 1583 | 151 | 3476 | 0 | 129 | 3529 | 0 |
| Satd．Flow（RTOR） |  | 106 |  |  |  | 127 |  | 19 |  |  | 3 |  |
| Lane Group Flow（vph） | 76 | 159 | 0 | 262 | 41 | 337 | 74 | 2021 | 0 | 265 | 1727 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | pm＋ov | pm＋p | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 15.0 |  | 15.0 | 17.0 | 18.0 | 13.0 | 72.0 |  | 18.0 | 77.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 18.8 | 8.2 |  | 23.2 | 11.9 | 29.2 | 68.3 | 65.8 |  | 75.9 | 73.4 |  |
| Actuated g／C Ratio | 0.16 | 0.07 |  | 0.19 | 0.10 | 0.24 | 0.57 | 0.55 |  | 0.63 | 0.61 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.31 | 0.75 |  | 1.09 | 0.22 | 0.70 | 0.38 | 1.06 |  | 0.97 | 0.80 |  |
| Control Delay | 43.7 | 42.7 |  | 130.3 | 55.1 | 29.3 | 21.9 | 50.5 |  | 80.8 | 22.2 |  |
| Queue Delay | 0.0 | 0.1 |  | 0.0 | 0.0 | 0.0 | 0.0 | 4.0 |  | 0.0 | 0.2 |  |
| Total Delay | 43.7 | 42.8 |  | 130.3 | 55.1 | 29.3 | 21.9 | 54.5 |  | 80.8 | 22.4 |  |
| LOS | D | D |  | F | E | C | C | D |  | F | C |  |
| Approach Delay |  | 43.1 |  |  | 72.3 |  |  | 53.4 |  |  | 30.2 |  |
| Approach LOS |  | D |  |  | E |  |  | D |  |  | C |  |
| Queue Length 50th（ft） | 49 | 40 |  | ～211 | 31 | 123 | 19 | $\sim 886$ |  | 154 | 536 |  |
| Queue Length 95th（ft） | 93 | \＃142 |  | \＃393 | 67 | 214 | m22 | 1011 |  | \＃330 | 646 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ t ） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 246 | 215 |  | 241 | 199 | 481 | 207 | 1914 |  | 272 | 2159 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 18 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 1 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 51 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.31 | 0.74 |  | 1.09 | 0.21 | 0.70 | 0.36 | 1.07 |  | 0.97 | 0.82 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 106 （88\％），Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 1.09 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 46.0 |  |  |  |  | Intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 105．4\％ |  |  |  |  | ICU Level of Service G |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| ～Volume exceeds capacity，queue is theoretically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| $m$ Volume for 95th perc | queue | metere | by ups | eam sig |  |  |  |  |  |  |  |  |



1：Randall Rd \＆IL Route 38

|  |  |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊＊ | 个个 | 「 | \％${ }^{1 / 8}$ | 个个 | 「 | ${ }^{1+1}$ | 个4 | 「 | ${ }^{\text {\％}}$ | 个个 | F |
| Volume（vph） | 394 | 506 | 118 | 328 | 384 | 372 | 116 | 1251 | 205 | 380 | 1506 | 301 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 |
| Flt Permitted | 0.244 |  |  | 0.184 |  |  | 0.061 |  |  | 0.066 |  |  |
| Satd．Flow（perm） | 882 | 3539 | 1583 | 665 | 3539 | 1583 | 220 | 3539 | 1583 | 239 | 3539 | 1583 |
| Satd．Flow（RTOR） |  |  | 97 |  |  | 62 |  |  | 97 |  |  | 70 |
| Lane Group Flow（vph） | 428 | 550 | 128 | 357 | 417 | 404 | 126 | 1360 | 223 | 413 | 1637 | 327 |
| Turn Type | pm＋pt | NA | pm＋ov | pm＋pt | NA | pm＋ov | pm＋pt | NA | pm＋ov | pm＋pt | NA | pm＋ov |
| Protected Phases | 5 | 2 | 3 | 1 | 6 | 7 |  | 8 | 1 | 7 | 4 | 5 |
| Permitted Phases | 2 |  | 2 | 6 |  | 6 | 8 |  | 8 | 4 |  | 4 |
| Total Split（s） | 20.0 | 31.0 | 13.0 | 17.0 | 28.0 | 21.0 | 13.0 | 71.0 | 17.0 | 21.0 | 79.0 | 20.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Effct Green（s） | 41.8 | 24.5 | 38.4 | 36.2 | 21.7 | 43.5 | 75.1 | 65.6 | 84.6 | 87.5 | 73.6 | 95.4 |
| Actuated g／C Ratio | 0.30 | 0.18 | 0.27 | 0.26 | 0.16 | 0.31 | 0.54 | 0.47 | 0.60 | 0.62 | 0.53 | 0.68 |
| v／c Ratio | 0.79 | 0.89 | 0.25 | 0.85 | 0.76 | 0.76 | 0.44 | 0.82 | 0.22 | 0.83 | 0.88 | 0.30 |
| Control Delay | 49.8 | 73.7 | 13.1 | 47.5 | 55.2 | 43.2 | 18.3 | 37.5 | 7.7 | 50.3 | 19.4 | 3.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 4.3 | 0.0 | 2.0 | 0.0 | 0.0 | 8.8 | 0.0 |
| Total Delay | 49.8 | 73.7 | 13.1 | 47.5 | 55.2 | 47.5 | 18.3 | 39.5 | 7.7 | 50.3 | 28.1 | 3.6 |
| LOS | D | E | B | D | E | D | B | D | A | D | C | A |
| Approach Delay |  | 57.5 |  |  | 50.2 |  |  | 33.8 |  |  | 28.6 |  |
| Approach LOS |  | E |  |  | D |  |  | C |  |  | C |  |
| Queue Length 50th（ft） | 158 | 260 | 21 | 128 | 196 | 317 | 21 | 561 | 48 | 120 | 524 | 28 |
| Queue Length 95th（ft） | 208 | \＃359 | 73 | \＃187 | 257 | 436 | 38 | 662 | 89 | m138 | m648 | m43 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ft） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 547 | 619 | 516 | 419 | 547 | 547 | 314 | 1658 | 995 | 525 | 1859 | 1102 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 212 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 83 | 0 | 169 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.78 | 0.89 | 0.25 | 0.85 | 0.76 | 0.87 | 0.40 | 0.91 | 0.22 | 0.79 | 0.99 | 0.30 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $19(14 \%)$ ，Referenced to phase 4：SBTL and 8：NBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.89 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 39.0 |  |  |  |  | intersection LOS：D |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 87．5\％ |  |  |  |  | ICU Level of Service E |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


Prairie Centre 11：15 am 4／16／2016 2026 Total Traffic with Improvements
Synchro 8 Report Timing Plan：Sat

|  | $\star$ |  |  |  | 4 |  | 4 | $\dagger$ |  | * | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个 ${ }_{\text {¢ }}$ |  | \% | 中 ${ }^{\text {c }}$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ | F |
| Volume (vph) | 216 | 729 | 65 | 39 | 781 | 55 | 53 | 1 | 70 | 81 | 8 | 189 |
| Satd. Flow (prot) | 1770 | 3497 | 0 | 1770 | 3504 | 0 | 1770 | 1587 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.262 |  |  | 0.317 |  |  | 0.752 |  |  | 0.608 |  |  |
| Satd. Flow (perm) | 488 | 3497 | 0 | 590 | 3504 | 0 | 1401 | 1587 | 0 | 1133 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 11 |  |  | 7 |  |  | 76 |  |  |  | 205 |
| Lane Group Flow (vph) | 235 | 863 | 0 | 42 | 909 | 0 | 58 | 77 | 0 | 88 | 9 | 205 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | , | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | . |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 29.0 | 84.0 |  | 13.0 | 68.0 |  | 13.0 | 30.0 |  | 13.0 | 30.0 | 30.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 | 6.0 |
| Act Effct Green (s) | 107.5 | 96.5 |  | 99.7 | 90.4 |  | 21.4 | 10.4 |  | 23.0 | 12.9 | 12.9 |
| Actuated g/C Ratio | 0.77 | 0.69 |  | 0.71 | 0.65 |  | 0.15 | 0.07 |  | 0.16 | 0.09 | 0.09 |
| v/c Ratio | 0.50 | 0.36 |  | 0.09 | 0.40 |  | 0.25 | 0.41 |  | 0.39 | 0.05 | 0.62 |
| Control Delay | 15.8 | 22.9 |  | 2.5 | 5.6 |  | 49.8 | 19.0 |  | 53.5 | 58.5 | 15.9 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 15.8 | 22.9 |  | 2.5 | 5.6 |  | 49.8 | 19.0 |  | 53.5 | 58.5 | 15.9 |
| LOS | B | C |  | A | A |  | D | B |  | D | E | B |
| Approach Delay |  | 21.4 |  |  | 5.5 |  |  | 32.2 |  |  | 28.2 |  |
| Approach LOS |  | C |  |  | A |  |  | C |  |  | C |  |
| Queue Length 50th (ft) | 107 | 346 |  | 3 | 67 |  | 46 | 1 |  | 71 | 8 | 0 |
| Queue Length 95th (ft) | m193 | m426 |  | m8 | 90 |  | 83 | 50 |  | 115 | 26 | 77 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length (ft) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 608 | 2414 |  | 513 | 2263 |  | 249 | 335 |  | 229 | 319 | 441 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.39 | 0.36 |  | 0.08 | 0.40 |  | 0.23 | 0.23 |  | 0.38 | 0.03 | 0.46 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 140

Actuated Cycle Length: 140
Offset: 46 (33\%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.62
Intersection Signal Delay: $16.7 \quad$ Intersection LOS: B
Intersection Capacity Utilization 60.2\% ICU Level of Service B
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.

Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个t |  | 7 | 个t |  | 7 | F |  | \% | F |  |
| Volume (vph) | 75 | 755 | 43 | 268 | 784 | 60 | 55 | 130 | 254 | 131 | 166 | 32 |
| Satd. Flow (prot) | 1770 | 3511 | 0 | 1770 | 3500 | 0 | 1770 | 1678 | 0 | 1770 | 1818 | 0 |
| Flt Permitted | 0.257 |  |  | 0.179 |  |  | 0.551 |  |  | 0.144 |  |  |
| Satd. Flow (perm) | 479 | 3511 | 0 | 333 | 3500 | 0 | 1026 | 1678 | 0 | 268 | 1818 | 0 |
| Satd. Flow (RTOR) |  | 4 |  |  | 7 |  |  | 70 |  |  | 7 |  |
| Lane Group Flow (vph) | 82 | 868 | 0 | 291 | 917 | 0 | 60 | 417 | 0 | 142 | 215 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 51.0 |  | 29.0 | 67.0 |  | 13.0 | 46.0 |  | 14.0 | 47.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Efft Green (s) | 66.3 | 55.1 |  | 81.0 | 66.3 |  | 46.3 | 35.6 |  | 50.6 | 39.7 |  |
| Actuated g/C Ratio | 0.47 | 0.39 |  | 0.58 | 0.47 |  | 0.33 | 0.25 |  | 0.36 | 0.28 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.27 | 0.63 |  | 0.74 | 0.55 |  | 0.16 | 0.87 |  | 0.68 | 0.41 |  |
| Control Delay | 9.2 | 19.1 |  | 30.0 | 28.6 |  | 28.1 | 60.3 |  | 46.4 | 42.0 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 9.2 | 19.1 |  | 30.0 | 28.6 |  | 28.1 | 60.3 |  | 46.4 | 42.0 |  |
| LOS | A | B |  | C | C |  | C | E |  | D | D |  |
| Approach Delay |  | 18.3 |  |  | 29.0 |  |  | 56.2 |  |  | 43.8 |  |
| Approach LOS |  | B |  |  | C |  |  | E |  |  | D |  |
| Queue Length 50th ( ft ) | 17 | 141 |  | 134 | 317 |  | 34 | 304 |  | 85 | 154 |  |
| Queue Length 95th (ft) | 28 | 171 |  | 226 | 401 |  | 65 | 432 |  | \#134 | 229 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 318 | 1384 |  | 454 | 1661 |  | 399 | 529 |  | 209 | 537 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.26 | 0.63 |  | 0.64 | 0.55 |  | 0.15 | 0.79 |  | 0.68 | 0.40 |  |

## Intersection Summary

## Cycle Length: 140

Actuated Cycle Length: 140
Offset: $40(29 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.87
Intersection Signal Delay: 31.7
Intersection LOS: C
Intersection Capacity Utilization 83.9\%
ICU Level of Service E
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 3: 14th St \& IL Route 38


|  | 4 |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个 |  | \％ | $\uparrow$ | 「 | \％ | 个 F |  | \％ | 中 ${ }^{\text {b }}$ |  |
| Volume（vph） | 62 | 38 | 117 | 226 | 41 | 370 | 60 | 1676 | 283 | 259 | 1884 | 28 |
| Satd．Flow（prot） | 1770 | 1652 | 0 | 1770 | 1863 | 1583 | 1770 | 3461 | 0 | 1770 | 3532 | 0 |
| FIt Permitted | 0.728 |  |  | 0.301 |  |  | 0.051 |  |  | 0.047 |  |  |
| Satd．Flow（perm） | 1356 | 1652 | 0 | 561 | 1863 | 1583 | 95 | 3461 | 0 | 88 | 3532 | 0 |
| Satd．Flow（RTOR） |  | 85 |  |  |  | 68 |  | 23 |  |  | 2 |  |
| Lane Group Flow（vph） | 67 | 168 | 0 | 246 | 45 | 402 | 65 | 2130 | 0 | 282 | 2078 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | $\mathrm{pm}+\mathrm{ov}$ | pm＋pt | NA |  | pm＋pt | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 | 2 |  |  | 6 |  |  |
| Total Split（s） | 13.0 | 15.0 |  | 17.0 | 19.0 | 21.0 | 13.0 | 87.0 |  | 21.0 | 95.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 19.1 | 8.5 |  | 28.0 | 15.3 | 35.6 | 91.2 | 80.5 |  | 104.0 | 91.1 |  |
| Actuated g／C Ratio | 0.14 | 0.06 |  | 0.20 | 0.11 | 0.25 | 0.65 | 0.58 |  | 0.74 | 0.65 |  |
| v／c Ratio | 0.32 | 0.93 |  | 1.10 | 0.22 | 0.89 | 0.41 | 1.07 |  | 1.05 | 0.90 |  |
| Control Delay | 51.4 | 83.2 |  | 137.1 | 62.5 | 58.4 | 24.2 | 58.9 |  | 116.5 | 28.5 |  |
| Queue Delay | 0.0 | 2.9 |  | 0.0 | 0.0 | 0.0 | 0.0 | 1.3 |  | 0.0 | 3.8 |  |
| Total Delay | 51.4 | 86.1 |  | 137.1 | 62.5 | 58.4 | 24.2 | 60.2 |  | 116.5 | 32.3 |  |
| LOS | D | F |  | F | E | E | C | E |  | F | C |  |
| Approach Delay |  | 76.2 |  |  | 86.6 |  |  | 59.1 |  |  | 42.3 |  |
| Approach LOS |  | E |  |  | F |  |  | E |  |  | D |  |
| Queue Length 50th（ft） | 51 | 77 |  | ～234 | 39 | 261 | 14 | ～1101 |  | $\sim 225$ | 839 |  |
| Queue Length 95th（ft） | 96 | \＃223 |  | \＃370 | 81 | \＃363 | m16 | \＃1244 |  | \＃413 | 984 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ ft ） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 220 | 180 |  | 224 | 203 | 453 | 170 | 1999 |  | 269 | 2299 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 6 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 3 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 160 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.30 | 0.95 |  | 1.10 | 0.22 | 0.89 | 0.38 | 1.07 |  | 1.05 | 0.97 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 140 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 0 （0\％），Referenced to phase 2：NBTL and 6：SBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 1.10 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 56.1 |  |  |  |  | Intersection LOS：E |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 108．9\％ |  |  |  |  | ICU Level of Service G |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| ～Volume exceeds capacity，queue is theoretically infinite． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95 th percentile queue is metered by upstream signal． |  |  |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：11：Randall Rd \＆Prairie St


## APPENDIXI

## Synchro Analysis Reports <br> 2026 Total Traffic

With Improvements and Randall Road Widening

1. Weekday AM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
2. Weekday PM Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street
3. Saturday Peak Hour
a. Randall Road \& IL Route 38
b. IL Route 38 \& West Mall Entrance
c. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
d. Randall Road \& Prairie Street

1：Randall Rd \＆IL Route 38

|  | 4 |  |  |  |  |  | 4 | 4 | $>$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | ¢ | 「 | \％＊ | ¢ $\uparrow$ | F | 7＊＊ | 愅 | F | ${ }^{7} 1$ | 愅 | F |
| Volume（vph） | 242 | 623 | 39 | 181 | 300 | 208 | 98 | 1012 | 129 | 365 | 1013 | 255 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 5085 | 1583 | 3433 | 5085 | 1583 |
| FIt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 5085 | 1583 | 3433 | 5085 | 1583 |
| Satd．Flow（RTOR） |  |  | 185 |  |  | 226 |  |  | 136 |  |  | 156 |
| Lane Group Flow（vph） | 263 | 677 | 42 | 197 | 326 | 226 | 107 | 1100 | 140 | 397 | 1101 | 277 |
| Turn Type | Prot | NA | Prot | Prot | NA | Prot | Prot | NA | pt＋ov | Prot | NA | pt＋ov |
| Protected Phases | 5 | 2 | 2 | 1 | 6 | 6 | 3 | 8 | 81 | 7 | 4 | 45 |
| Permitted Phases |  |  |  |  |  |  |  |  |  |  |  |  |
| Total Split（s） | 17.0 | 31.0 | 31.0 | 13.0 | 27.0 | 27.0 | 13.0 | 36.0 |  | 20.0 | 43.0 |  |
| Total Lost Time（s） | 4.5 | 6.5 | 6.5 | 4.5 | 6.5 | 6.5 | 4.5 | 6.5 |  | 4.5 | 6.5 |  |
| Act Efftt Green（s） | 11.7 | 24.6 | 24.6 | 8.4 | 21.3 | 21.3 | 7.8 | 30.2 | 43.1 | 14.8 | 37.2 | 53.3 |
| Actuated g／C Ratio | 0.12 | 0.25 | 0.25 | 0.08 | 0.21 | 0.21 | 0.08 | 0.30 | 0.43 | 0.15 | 0.37 | 0.53 |
| v／c Ratio | 0.66 | 0.78 | 0.08 | 0.68 | 0.43 | 0.44 | 0.40 | 0.72 | 0.18 | 0.78 | 0.58 | 0.30 |
| Control Delay | 50.5 | 42.4 | 0.3 | 46.9 | 38.1 | 15.3 | 48.2 | 34.4 | 4.0 | 63.2 | 18.7 | 1.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 50.5 | 42.4 | 0.3 | 46.9 | 38.1 | 15.3 | 48.2 | 34.4 | 4.0 | 63.2 | 18.7 | 1.7 |
| LOS | D | D | A | D | D | B | D | C | A | E | B | A |
| Approach Delay |  | 42.8 |  |  | 33.5 |  |  | 32.3 |  |  | 26.0 |  |
| Approach LOS |  | D |  |  | C |  |  | C |  |  | C |  |
| Queue Length 50th（ft） | 82 | 212 | 0 | 43 | 112 | 33 | 34 | 230 | 1 | 134 | 164 | 9 |
| Queue Length 95th（ft） | 125 | 280 | 0 | \＃94 | 145 | 108 | 60 | 282 | 36 | \＃191 | 98 | 4 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ft） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 429 | 870 | 528 | 291 | 755 | 515 | 291 | 1535 | 760 | 532 | 1889 | 928 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.61 | 0.78 | 0.08 | 0.68 | 0.43 | 0.44 | 0.37 | 0.72 | 0.18 | 0.75 | 0.58 | 0.30 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 21 （21\％），Referenced to phase 4：SBT and 8：NBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.78 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 32.3 |  |  |  | Intersection LOS：C |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 72．0\％ |  |  |  | ICU Level of Service C |  |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maxim | after two | cycles． |  |  |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


|  | $\stackrel{ }{*}$ | $\rightarrow$ |  | 7 | 4 |  |  | 4 | 7 | $\checkmark$ | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个t |  | \% | 性 |  | \% | $\hat{1}$ |  | \% | $\uparrow$ | F |
| Volume (vph) | 182 | 879 | 74 | 32 | 480 | 46 | 36 | 2 | 53 | 79 | 1 | 194 |
| Satd. Flow (prot) | 1770 | 3497 | 0 | 1770 | 3493 | 0 | 1770 | 1593 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.388 |  |  | 0.247 |  |  | 0.757 |  |  | 0.578 |  |  |
| Satd. Flow (perm) | 723 | 3497 | 0 | 460 | 3493 | 0 | 1410 | 1593 | 0 | 1077 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 11 |  |  | 12 |  |  | 58 |  |  |  | 211 |
| Lane Group Flow (vph) | 198 | 1035 | 0 | 35 | 572 | 0 | 39 | 60 | 0 | 86 | 1 | 211 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 14.0 | 50.0 |  | 13.0 | 49.0 |  | 13.0 | 24.0 |  | 13.0 | 24.0 | 24.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 | 6.0 |
| Act Effct Green (s) | 70.7 | 61.7 |  | 63.9 | 54.7 |  | 18.2 | 9.6 |  | 21.0 | 12.8 | 12.8 |
| Actuated g/C Ratio | 0.71 | 0.62 |  | 0.64 | 0.55 |  | 0.18 | 0.10 |  | 0.21 | 0.13 | 0.13 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.32 | 0.48 |  | 0.09 | 0.30 |  | 0.14 | 0.29 |  | 0.30 | 0.00 | 0.55 |
| Control Delay | 2.0 | 6.4 |  | 10.4 | 18.8 |  | 29.1 | 15.3 |  | 32.2 | 38.0 | 11.4 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 2.0 | 6.4 |  | 10.4 | 18.8 |  | 29.1 | 15.3 |  | 32.2 | 38.0 | 11.4 |
| LOS | A | A |  | B | B |  | C | B |  | C | D | B |
| Approach Delay |  | 5.7 |  |  | 18.3 |  |  | 20.7 |  |  | 17.5 |  |
| Approach LOS |  | A |  |  | B |  |  | C |  |  | B |  |
| Queue Length 50th (ft) | 5 | 221 |  | 6 | 74 |  | 20 | 1 |  | 44 | 1 | 0 |
| Queue Length 95th (ft) | m8 | 282 |  | 28 | 187 |  | 42 | 37 |  | 78 | 6 | 63 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length ( t ) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 626 | 2161 |  | 433 | 1915 |  | 312 | 334 |  | 292 | 342 | 462 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.32 | 0.48 |  | 0.08 | 0.30 |  | 0.13 | 0.18 |  | 0.29 | 0.00 | 0.46 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 100

Actuated Cycle Length: 100
Offset: 14 (14\%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.55
Intersection Signal Delay: $11.3 \quad$ Intersection LOS: B
Intersection Capacity Utilization 54.8\% ICU Level of Service A
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个 ${ }_{\text {S }}$ |  | \% | 中 ${ }^{\text {d }}$ |  | 7 | $\uparrow$ |  | \% | F |  |
| Volume (vph) | 73 | 966 | 19 | 95 | 495 | 47 | 24 | 85 | 221 | 100 | 98 | 34 |
| Satd. Flow (prot) | 1770 | 3529 | 0 | 1770 | 3493 | 0 | 1770 | 1662 | 0 | 1770 | 1790 | 0 |
| Flt Permitted | 0.382 |  |  | 0.146 |  |  | 0.665 |  |  | 0.218 |  |  |
| Satd. Flow (perm) | 712 | 3529 | 0 | 272 | 3493 | 0 | 1239 | 1662 | 0 | 406 | 1790 | 0 |
| Satd. Flow (RTOR) |  | 2 |  |  | 11 |  |  | 124 |  |  | 16 |  |
| Lane Group Flow (vph) | 79 | 1071 | 0 | 103 | 589 | 0 | 26 | 332 | 0 | 109 | 144 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 44.0 |  | 13.0 | 44.0 |  | 13.0 | 30.0 |  | 13.0 | 30.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Efft Green (s) | 55.5 | 46.1 |  | 56.3 | 46.5 |  | 28.4 | 19.4 |  | 33.8 | 27.5 |  |
| Actuated g/C Ratio | 0.56 | 0.46 |  | 0.56 | 0.46 |  | 0.28 | 0.19 |  | 0.34 | 0.28 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.17 | 0.66 |  | 0.38 | 0.36 |  | 0.07 | 0.79 |  | 0.42 | 0.29 |  |
| Control Delay | 6.4 | 12.4 |  | 14.7 | 19.7 |  | 20.0 | 36.9 |  | 26.5 | 26.9 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 6.4 | 12.4 |  | 14.7 | 19.7 |  | 20.0 | 36.9 |  | 26.5 | 26.9 |  |
| LOS | A | B |  | B | B |  | C | D |  | C | C |  |
| Approach Delay |  | 12.0 |  |  | 18.9 |  |  | 35.7 |  |  | 26.8 |  |
| Approach LOS |  | B |  |  | B |  |  | D |  |  | C |  |
| Queue Length 50th (ft) | 8 | 74 |  | 28 | 128 |  | 11 | 126 |  | 47 | 58 |  |
| Queue Length 95th (ft) | 31 | 183 |  | 57 | 192 |  | 27 | 220 |  | 81 | 120 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 503 | 1626 |  | 297 | 1628 |  | 439 | 493 |  | 266 | 510 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.16 | 0.66 |  | 0.35 | 0.36 |  | 0.06 | 0.67 |  | 0.41 | 0.28 |  |

## Intersection Summary

## Cycle Length: 100

Actuated Cycle Length: 100
Offset: $24(24 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.79
Intersection Signal Delay: 18.9 Intersection LOS: B
Intersection Capacity Utilization 73.3\% ICU Level of Service D
Analysis Period (min) 15
Splits and Phases: 3: 14th St \& IL Route 38


11：Randall Rd \＆Prairie St

|  | 4 |  |  | 7 | $\Perp$ |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | ¢ |  | \％ | $\uparrow$ | 「 | \％ | 个中献 |  | ${ }^{7}$ | 个中t |  |
| Volume（vph） | 4 | － | 19 | 144 | 11 | 237 | 16 | 1307 | 146 | 176 | 1489 | 12 |
| Satd．Flow（prot） | 1770 | 1652 | 0 | 1770 | 1863 | 1583 | 1770 | 5009 | 0 | 1770 | 5080 | 0 |
| Flt Permitted | 0.833 |  |  | 0.739 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 1552 | 1652 | 0 | 1377 | 1863 | 1583 | 1770 | 5009 | 0 | 1770 | 5080 | 0 |
| Satd．Flow（RTOR） |  | 21 |  |  |  | 229 |  | 23 |  |  | 2 |  |
| Lane Group Flow（vph） | 4 | 28 | 0 | 157 | 12 | 258 | 17 | 1580 | 0 | 191 | 1631 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | pt＋ov | Prot | NA |  | Prot | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 81 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  |  |  |  |  |  |  |  |
| Total Split（s） | 13.0 | 17.0 |  | 13.0 | 17.0 |  | 13.0 | 47.0 |  | 23.0 | 57.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 |  | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 11.8 | 7.4 |  | 15.5 | 11.3 | 29.0 | 6.8 | 54.4 |  | 15.0 | 68.2 |  |
| Actuated g／C Ratio | 0.12 | 0.07 |  | 0.16 | 0.11 | 0.29 | 0.07 | 0.54 |  | 0.15 | 0.68 |  |
| v／c Ratio | 0.02 | 0.20 |  | 0.65 | 0.06 | 0.42 | 0.14 | 0.58 |  | 0.72 | 0.47 |  |
| Control Delay | 30.2 | 25.1 |  | 51.0 | 38.8 | 6.0 | 28.6 | 5.6 |  | 55.4 | 10.1 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 30.2 | 25.1 |  | 51.0 | 38.8 | 6.0 | 28.6 | 5.6 |  | 55.4 | 10.1 |  |
| LOS | C | C |  | D | D | A | C | A |  | E | B |  |
| Approach Delay |  | 25.8 |  |  | 23.5 |  |  | 5.8 |  |  | 14.8 |  |
| Approach LOS |  | C |  |  | C |  |  | A |  |  | B |  |
| Queue Length 50th（ft） | 2 | 4 |  | 90 | 7 | 12 | 10 | 28 |  | 117 | 139 |  |
| Queue Length 95th（ft） | 11 | 31 |  | 142 | 24 | 55 | m15 | 373 |  | 184 | 305 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ft） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 257 | 192 |  | 271 | 239 | 661 | 159 | 2734 |  | 336 | 3466 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.02 | 0.15 |  | 0.58 | 0.05 | 0.39 | 0.11 | 0.58 |  | 0.57 | 0.47 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 100 |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length： 100

Actuated Cycle Length： 100
Offset： 19 （19\％），Referenced to phase 2：NBT and 6：SBT，Start of 1st Green
Control Type：Actuated－Coordinated
Maximum v／c Ratio： 0.72
Intersection Signal Delay： 12.2 Intersection LOS：B
Intersection Capacity Utilization 67．1\％ICU Level of Service C
Analysis Period（min） 15
m Volume for 95 th percentile queue is metered by upstream signal．

Splits and Phases：11：Randall Rd \＆Prairie St


1：Randall Rd \＆IL Route 38

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \％${ }^{1}$ | 4 4 | 「 | \％ 1 | 4 $\uparrow$ | 「 | \％ 11 | 4乐 | 「 | \％${ }^{17}$ | ¢44 | F |
| Volume（vph） | 298 | 408 | 87 | 251 | 616 | 431 | 111 | 1250 | 176 | 317 | 1231 | 290 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 5085 | 1583 | 3433 | 5085 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 5085 | 1583 | 3433 | 5085 | 1583 |
| Satd．Flow（RTOR） |  |  | 114 |  |  | 73 |  |  | 114 |  |  | 73 |
| Lane Group Flow（vph） | 324 | 443 | 95 | 273 | 670 | 468 | 121 | 1359 | 191 | 345 | 1338 | 315 |
| Turn Type | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov | Prot | NA | pm＋ov |
| Protected Phases | 5 | 2 | 3 | 1 | 6 | 7 | 3 | 8 | 1 | 7 | 4 | 5 |
| Permitted Phases |  |  | 2 |  |  | 6 |  |  | 8 |  |  | 4 |
| Total Split（s） | 19.0 | 32.0 | 13.0 | 20.0 | 33.0 | 21.0 | 13.0 | 47.0 | 20.0 | 21.0 | 55.0 | 19.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Effct Green（s） | 14.1 | 27.1 | 41.8 | 13.9 | 26.9 | 48.9 | 8.2 | 41.5 | 61.9 | 15.5 | 48.8 | 69.4 |
| Actuated g／C Ratio | 0.12 | 0.23 | 0.35 | 0.12 | 0.22 | 0.41 | 0.07 | 0.35 | 0.52 | 0.13 | 0.41 | 0.58 |
| v／c Ratio | 0.80 | 0.56 | 0.15 | 0.69 | 0.84 | 0.68 | 0.52 | 0.77 | 0.22 | 0.78 | 0.65 | 0.33 |
| Control Delay | 67.5 | 44.6 | 3.8 | 48.0 | 43.5 | 32.4 | 62.1 | 38.9 | 7.0 | 62.3 | 18.3 | 8.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.3 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 67.5 | 44.6 | 3.8 | 48.0 | 43.5 | 32.4 | 62.1 | 39.2 | 7.0 | 62.3 | 18.3 | 8.2 |
| LOS | E | D | A | D | D | C | E | D | A | E | B | A |
| Approach Delay |  | 48.7 |  |  | 40.7 |  |  | 37.2 |  |  | 24.3 |  |
| Approach LOS |  | D |  |  | D |  |  | D |  |  | C |  |
| Queue Length 50th（ft） | 127 | 162 | 0 | 90 | 279 | 337 | 47 | 346 | 29 | 107 | 322 | 94 |
| Queue Length 95th（ft） | \＃190 | 220 | 27 | 139 | \＃362 | 428 | 79 | 405 | 68 | m164 | 347 | m154 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ft） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 414 | 798 | 628 | 443 | 793 | 700 | 243 | 1756 | 891 | 472 | 2068 | 951 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 67 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.78 | 0.56 | 0.15 | 0.62 | 0.84 | 0.67 | 0.50 | 0.80 | 0.21 | 0.73 | 0.65 | 0.33 |

## Intersection Summary

## Cycle Length： 120

Actuated Cycle Length： 120
Offset： 10 （8\％），Referenced to phase 4：SBT and 8：NBT，Start of 1st Green
Control Type：Actuated－Coordinated
Maximum v／c Ratio： 0.84
Intersection Signal Delay： 35.4
Intersection LOS：D
Intersection Capacity Utilization 77．1\％
ICU Level of Service D
Analysis Period（min） 15
\＃95th percentile volume exceeds capacity，queue may be longer．
Queue shown is maximum after two cycles．
m Volume for 95 th percentile queue is metered by upstream signal．
Splits and Phases：1：Randall Rd \＆IL Route 38


Prairie Centre 4：30 pm 4／12／2016 2026 Total Traffic with Randall Widening
Synchro 8 Report Timing Plan：PM

|  | 4 |  |  | 7 |  |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个t |  | \% | 个t |  | \% | F |  | ${ }^{7}$ | $\uparrow$ | 7 |
| Volume (vph) | 212 | 650 | 51 | 63 | 1078 | 54 | 53 | 2 | 66 | 73 | 4 | 167 |
| Satd. Flow (prot) | 1770 | 3500 | 0 | 1770 | 3514 | 0 | 1770 | 1591 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.150 |  |  | 0.363 |  |  | 0.755 |  |  | 0.681 |  |  |
| Satd. Flow (perm) | 279 | 3500 | 0 | 676 | 3514 | 0 | 1406 | 1591 | 0 | 1269 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 11 |  |  | 6 |  |  | 72 |  |  |  | 182 |
| Lane Group Flow (vph) | 230 | 762 | 0 | 68 | 1231 | 0 | 58 | 74 | 0 | 79 | 4 | 182 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | , | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 24.0 | 73.0 |  | 13.0 | 62.0 |  | 13.0 | 21.0 |  | 13.0 | 21.0 | 21.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 | 6.0 |
| Act Effct Green (s) | 90.3 | 78.9 |  | 80.7 | 70.9 |  | 19.5 | 9.9 |  | 20.2 | 10.3 | 10.3 |
| Actuated g/C Ratio | 0.75 | 0.66 |  | 0.67 | 0.59 |  | 0.16 | 0.08 |  | 0.17 | 0.09 | 0.09 |
| v/c Ratio | 0.62 | 0.33 |  | 0.13 | 0.59 |  | 0.23 | 0.38 |  | 0.32 | 0.03 | 0.60 |
| Control Delay | 16.8 | 15.3 |  | 2.3 | 5.6 |  | 40.3 | 17.4 |  | 42.4 | 48.5 | 16.0 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 16.8 | 15.3 |  | 2.3 | 5.6 |  | 40.3 | 17.4 |  | 42.4 | 48.5 | 16.0 |
| LOS | B | B |  | A | A |  | D | B |  | D | D | B |
| Approach Delay |  | 15.7 |  |  | 5.4 |  |  | 27.4 |  |  | 24.4 |  |
| Approach LOS |  | B |  |  | A |  |  | C |  |  | C |  |
| Queue Length 50th (ft) | 85 | 192 |  | 4 | 73 |  | 38 | 2 |  | 52 | 3 | 0 |
| Queue Length 95th (ft) | m131 | 345 |  | m7 | 98 |  | 71 | 47 |  | 90 | 14 | 67 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length (ft) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 464 | 2303 |  | 556 | 2077 |  | 263 | 261 |  | 255 | 232 | 357 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.50 | 0.33 |  | 0.12 | 0.59 |  | 0.22 | 0.28 |  | 0.31 | 0.02 | 0.51 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $16(13 \%)$, Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.62
Intersection Signal Delay: 12.1 Intersection LOS: B
Intersection Capacity Utilization $67.7 \%$ ICU Level of Service $C$
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.
Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个t |  | \% | 个 ${ }^{\text {a }}$ |  | 7 | $\uparrow$ |  | \% | F |  |
| Volume (vph) | 103 | 677 | 38 | 241 | 1052 | 81 | 67 | 171 | 207 | 149 | 208 | 56 |
| Satd. Flow (prot) | 1770 | 3511 | 0 | 1770 | 3500 | 0 | 1770 | 1710 | 0 | 1770 | 1803 | 0 |
| Flt Permitted | 0.099 |  |  | 0.219 |  |  | 0.433 |  |  | 0.163 |  |  |
| Satd. Flow (perm) | 184 | 3511 | 0 | 408 | 3500 | 0 | 807 | 1710 | 0 | 304 | 1803 | 0 |
| Satd. Flow (RTOR) |  | 5 |  |  | 8 |  |  | 49 |  |  | 11 |  |
| Lane Group Flow (vph) | 112 | 777 | 0 | 262 | 1231 | 0 | 73 | 411 | 0 | 162 | 287 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 46.0 |  | 23.0 | 56.0 |  | 13.0 | 38.0 |  | 13.0 | 38.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Efft Green (s) | 57.6 | 46.1 |  | 67.3 | 52.4 |  | 40.8 | 30.1 |  | 43.4 | 33.3 |  |
| Actuated g/C Ratio | 0.48 | 0.38 |  | 0.56 | 0.44 |  | 0.34 | 0.25 |  | 0.36 | 0.28 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.56 | 0.57 |  | 0.66 | 0.80 |  | 0.21 | 0.88 |  | 0.72 | 0.57 |  |
| Control Delay | 39.8 | 22.2 |  | 22.7 | 34.9 |  | 25.0 | 59.1 |  | 44.5 | 41.2 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 39.8 | 22.2 |  | 22.7 | 34.9 |  | 25.0 | 59.1 |  | 44.5 | 41.2 |  |
| LOS | D | C |  | C | C |  | C | E |  | D | D |  |
| Approach Delay |  | 24.4 |  |  | 32.7 |  |  | 54.0 |  |  | 42.4 |  |
| Approach LOS |  | C |  |  | C |  |  | D |  |  | D |  |
| Queue Length 50th (ft) | 45 | 106 |  | 104 | 441 |  | 35 | 269 |  | 82 | 184 |  |
| Queue Length 95th (ft) | 105 | 260 |  | 156 | 541 |  | 67 | \#437 |  | \#145 | 280 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 215 | 1352 |  | 450 | 1532 |  | 359 | 491 |  | 225 | 507 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.52 | 0.57 |  | 0.58 | 0.80 |  | 0.20 | 0.84 |  | 0.72 | 0.57 |  |

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: 7 (6\%), Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.88
Intersection Signal Delay: 34.9
Intersection LOS: C
Intersection Capacity Utilization 84.4\%
ICU Level of Service E
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 3: 14th St \& IL Route 38


11：Randall Rd \＆Prairie St

|  | 4 |  |  |  |  |  |  | 4 |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ | 「 | ${ }^{7}$ | 个个官 |  | ${ }^{7}$ | 个中献 |  |
| Volume（vph） | 70 | 34 | 112 | 241 | 38 | 310 | 68 | 1638 | 222 | 244 | 1554 | 35 |
| Satd．Flow（prot） | 1770 | 1649 | 0 | 1770 | 1863 | 1583 | 1770 | 4994 | 0 | 1770 | 5070 | 0 |
| Flt Permitted | 0.730 |  |  | 0.296 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 1360 | 1649 | 0 | 551 | 1863 | 1583 | 1770 | 4994 | 0 | 1770 | 5070 | 0 |
| Satd．Flow（RTOR） |  | 107 |  |  |  | 127 |  | 25 |  |  | 4 |  |
| Lane Group Flow（vph） | 76 | 159 | 0 | 262 | 41 | 337 | 74 | 2021 | 0 | 265 | 1727 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | pm＋ov | Prot | NA |  | Prot | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 |  |  |  |  |  |  |
| Total Split（s） | 13.0 | 16.0 |  | 20.0 | 23.0 | 26.0 | 14.0 | 58.0 |  | 26.0 | 70.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 19.2 | 8.7 |  | 31.0 | 18.4 | 45.3 | 9.2 | 54.1 |  | 20.5 | 67.3 |  |
| Actuated g／C Ratio | 0.16 | 0.07 |  | 0.26 | 0.15 | 0.38 | 0.08 | 0.45 |  | 0.17 | 0.56 |  |
| v／c Ratio | 0.31 | 0.73 |  | 0.87 | 0.14 | 0.50 | 0.55 | 0.89 |  | 0.88 | 0.61 |  |
| Control Delay | 38.3 | 39.6 |  | 66.8 | 47.2 | 20.3 | 50.1 | 22.3 |  | 77.1 | 19.5 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 38.3 | 39.6 |  | 66.8 | 47.2 | 20.3 | 50.1 | 22.3 |  | 77.1 | 19.5 |  |
| LOS | D | D |  | E | D | C | D | C |  | E | B |  |
| Approach Delay |  | 39.2 |  |  | 41.0 |  |  | 23.3 |  |  | 27.1 |  |
| Approach LOS |  | D |  |  | D |  |  | C |  |  | C |  |
| Queue Length 50th（ft） | 45 | 39 |  | 175 | 28 | 121 | 58 | 227 |  | 199 | 335 |  |
| Queue Length 95th（ft） | 87 | \＃130 |  | \＃281 | 63 | 211 | m70 | \＃255 |  | \＃337 | 386 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ft） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 259 | 229 |  | 304 | 285 | 695 | 147 | 2263 |  | 324 | 2846 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 3 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.29 | 0.69 |  | 0.86 | 0.14 | 0.48 | 0.50 | 0.89 |  | 0.82 | 0.61 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $108(90 \%)$ ，Referenced to phase 2：NBT and 6：SBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.89 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 27.9 |  |  |  |  | Intersection LOS：C |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 89．6\％ |  |  |  |  | ICU Level of Service E |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| $m$ Volume for 95th perc | equeue | metere | by ups | eam sig |  |  |  |  |  |  |  |  |

Splits and Phases：11：Randall Rd \＆Prairie St


Prairie Centre 4：30 pm 4／12／2016 2026 Total Traffic with Randall Widening

1：Randall Rd \＆IL Route 38

|  | 4 |  |  |  |  |  | 4 | $\uparrow$ | $>$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7 \%}$ | 个4 | F | \％${ }^{1+1}$ | 个 $\uparrow$ | F | \％${ }^{1+1}$ | 个乐 | 「 | \％${ }^{1+1}$ | 个个个 | F |
| Volume（vph） | 394 | 506 | 118 | 328 | 384 | 372 | 116 | 1251 | 205 | 380 | 1506 | 301 |
| Satd．Flow（prot） | 3433 | 3539 | 1583 | 3433 | 3539 | 1583 | 3433 | 5085 | 1583 | 3433 | 5085 | 1583 |
| Flt Permitted | 0.287 |  |  | 0.269 |  |  | 0.085 |  |  | 0.094 |  |  |
| Satd．Flow（perm） | 1037 | 3539 | 1583 | 972 | 3539 | 1583 | 307 | 5085 | 1583 | 340 | 5085 | 1583 |
| Satd．Flow（RTOR） |  |  | 114 |  |  | 73 |  |  | 114 |  |  | 95 |
| Lane Group Flow（vph） | 428 | 550 | 128 | 357 | 417 | 404 | 126 | 1360 | 223 | 413 | 1637 | 327 |
| Turn Type | pm＋pt | NA | pm＋ov | pm＋pt | NA | pm＋ov | pm＋pt | NA | pm＋ov | pm＋pt | NA | pm＋ov |
| Protected Phases | 5 | 2 | 3 | 1 | 6 | 7 | 3 | ， | 1 | 7 | 4 | 5 |
| Permitted Phases | 2 |  | 2 | 6 |  | 6 | 8 |  | 8 | 4 |  | 4 |
| Total Split（s） | 20.0 | 31.0 | 13.0 | 16.0 | 27.0 | 23.0 | 13.0 | 50.0 | 16.0 | 23.0 | 60.0 | 20.0 |
| Total Lost Time（s） | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 | 4.5 | 6.5 | 4.5 |
| Act Effct Green（s） | 41.2 | 24.5 | 38.4 | 34.5 | 21.1 | 42.4 | 56.7 | 47.3 | 65.2 | 68.5 | 54.8 | 76.0 |
| Actuated g／C Ratio | 0.34 | 0.20 | 0.32 | 0.29 | 0.18 | 0.35 | 0.47 | 0.39 | 0.54 | 0.57 | 0.46 | 0.63 |
| v／c Ratio | 0.66 | 0.76 | 0.22 | 0.70 | 0.67 | 0.67 | 0.37 | 0.68 | 0.24 | 0.72 | 0.71 | 0.32 |
| Control Delay | 34.1 | 52.9 | 7.7 | 27.7 | 42.4 | 34.2 | 16.3 | 32.7 | 8.1 | 30.4 | 15.4 | 5.0 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 34.1 | 52.9 | 7.7 | 27.7 | 42.4 | 34.2 | 16.3 | 32.8 | 8.1 | 30.4 | 15.4 | 5.0 |
| LOS | C | D | A | C | D | C | B | C | A | C | B | A |
| Approach Delay |  | 40.4 |  |  | 35.1 |  |  | 28.4 |  |  | 16.6 |  |
| Approach LOS |  | D |  |  | D |  |  | C |  |  | B |  |
| Queue Length 50th（ft） | 125 | 213 | 7 | 77 | 168 | 276 | 21 | 319 | 40 | 62 | 390 | 62 |
| Queue Length 95th（ft） | 168 | 277 | 51 | 107 | 224 | 358 | 35 | 390 | 89 | m113 | 397 | m113 |
| Internal Link Dist（ft） |  | 670 |  |  | 319 |  |  | 781 |  |  | 597 |  |
| Turn Bay Length（ t ） | 425 |  | 490 |  |  |  | 235 |  | 460 | 240 |  | 220 |
| Base Capacity（vph） | 672 | 722 | 597 | 516 | 622 | 653 | 369 | 2004 | 913 | 670 | 2320 | 1046 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 55 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v／c Ratio | 0.64 | 0.76 | 0.21 | 0.69 | 0.67 | 0.62 | 0.34 | 0.70 | 0.24 | 0.62 | 0.71 | 0.31 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： 4 （3\％），Referenced to phase 4：SBTL and 8：NBTL，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.76 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 27.3 |  |  |  | Intersection LOS：C |  |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 77．1\％ |  |  |  | ICU Level of Service D |  |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  | － |  |  |  |  |  |  |  |  |

Splits and Phases：1：Randall Rd \＆IL Route 38


[^2]Synchro 8 Report

|  | 4 | $\rightarrow$ |  | 7 | 4 |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个t |  | \% | 个t |  | \% | F |  | ${ }^{7}$ | $\uparrow$ | 7 |
| Volume (vph) | 216 | 729 | 65 | 39 | 781 | 55 | 53 | 1 | 70 | 81 | 8 | 189 |
| Satd. Flow (prot) | 1770 | 3497 | 0 | 1770 | 3504 | 0 | 1770 | 1587 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.253 |  |  | 0.316 |  |  | 0.752 |  |  | 0.543 |  |  |
| Satd. Flow (perm) | 471 | 3497 | 0 | 589 | 3504 | 0 | 1401 | 1587 | 0 | 1011 | 1863 | 1583 |
| Satd. Flow (RTOR) |  | 11 |  |  | 7 |  |  | 76 |  |  |  | 205 |
| Lane Group Flow (vph) | 235 | 863 | 0 | 42 | 909 | 0 | 58 | 77 | 0 | 88 | 9 | 205 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  | 4 |
| Total Split (s) | 25.0 | 68.0 |  | 13.0 | 56.0 |  | 13.0 | 26.0 |  | 13.0 | 26.0 | 26.0 |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 | 6.0 |
| Act Effct Green (s) | 90.0 | 79.0 |  | 82.3 | 73.0 |  | 18.8 | 10.2 |  | 20.6 | 10.6 | 10.6 |
| Actuated g/C Ratio | 0.75 | 0.66 |  | 0.69 | 0.61 |  | 0.16 | 0.08 |  | 0.17 | 0.09 | 0.09 |
| v/c Ratio | 0.50 | 0.37 |  | 0.09 | 0.43 |  | 0.24 | 0.38 |  | 0.38 | 0.05 | 0.63 |
| Control Delay | 9.2 | 13.8 |  | 4.1 | 8.3 |  | 40.2 | 16.5 |  | 43.7 | 48.8 | 15.7 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 9.2 | 13.8 |  | 4.1 | 8.3 |  | 40.2 | 16.5 |  | 43.7 | 48.8 | 15.7 |
| LOS | A | B |  | A | A |  | D | B |  | D | D | B |
| Approach Delay |  | 12.8 |  |  | 8.1 |  |  | 26.7 |  |  | 24.8 |  |
| Approach LOS |  | B |  |  | A |  |  | C |  |  | C |  |
| Queue Length 50th (ft) | 75 | 252 |  | 5 | 90 |  | 38 | 1 |  | 58 | 7 | 0 |
| Queue Length 95th (ft) | m132 | 342 |  | m10 | 121 |  | 70 | 45 |  | 98 | 22 | 70 |
| Internal Link Dist (ft) |  | 440 |  |  | 624 |  |  | 190 |  |  | 310 |  |
| Turn Bay Length (ft) | 170 |  |  | 215 |  |  | 65 |  |  | 100 |  | 100 |
| Base Capacity (vph) | 586 | 2306 |  | 513 | 2133 |  | 263 | 327 |  | 236 | 310 | 434 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.40 | 0.37 |  | 0.08 | 0.43 |  | 0.22 | 0.24 |  | 0.37 | 0.03 | 0.47 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: 15 (13\%), Referenced to phase 2:EBTL and 6:WBTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.63
Intersection Signal Delay: 13.2 Intersection LOS: B
Intersection Capacity Utilization 60.2\% ICU Level of Service B
Analysis Period (min) 15
m Volume for 95 th percentile queue is metered by upstream signal.

Splits and Phases: 2: IL Route 38 \& W Mall Entrance


| Lane Group | SEL | SET | SER | NWL | NWT | NWR | NEL | NET | NER | SWL | SWT | SWR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | 个 ${ }^{\text {a }}$ |  | \% | 个 ${ }^{\text {a }}$ |  | * | $\uparrow$ |  | \% | $\uparrow$ |  |
| Volume (vph) | 75 | 755 | 43 | 268 | 784 | 60 | 55 | 130 | 254 | 131 | 166 | 32 |
| Satd. Flow (prot) | 1770 | 3511 | 0 | 1770 | 3500 | 0 | 1770 | 1678 | 0 | 1770 | 1818 | 0 |
| Flt Permitted | 0.260 |  |  | 0.162 |  |  | 0.562 |  |  | 0.153 |  |  |
| Satd. Flow (perm) | 484 | 3511 | 0 | 302 | 3500 | 0 | 1047 | 1678 | 0 | 285 | 1818 | 0 |
| Satd. Flow (RTOR) |  | 5 |  |  | 8 |  |  | 81 |  |  | 8 |  |
| Lane Group Flow (vph) | 82 | 868 | 0 | 291 | 917 | 0 | 60 | 417 | 0 | 142 | 215 | 0 |
| Turn Type | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  | pm+pt | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  |  | 8 |  |  | 4 |  |  |
| Total Split (s) | 13.0 | 43.0 |  | 25.0 | 55.0 |  | 13.0 | 39.0 |  | 13.0 | 39.0 |  |
| Total Lost Time (s) | 3.5 | 6.5 |  | 3.5 | 6.5 |  | 3.5 | 6.0 |  | 3.5 | 6.0 |  |
| Act Effct Green (s) | 54.4 | 43.4 |  | 67.5 | 54.9 |  | 40.4 | 30.1 |  | 43.6 | 33.5 |  |
| Actuated g/C Ratio | 0.45 | 0.36 |  | 0.56 | 0.46 |  | 0.34 | 0.25 |  | 0.36 | 0.28 |  |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.27 | 0.68 |  | 0.76 | 0.57 |  | 0.15 | 0.87 |  | 0.65 | 0.42 |  |
| Control Delay | 9.4 | 22.6 |  | 31.1 | 27.0 |  | 23.7 | 52.9 |  | 38.9 | 37.0 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 9.4 | 22.6 |  | 31.1 | 27.0 |  | 23.7 | 52.9 |  | 38.9 | 37.0 |  |
| LOS | A | C |  | C | C |  | C | D |  | D | D |  |
| Approach Delay |  | 21.5 |  |  | 28.0 |  |  | 49.3 |  |  | 37.8 |  |
| Approach LOS |  | C |  |  | C |  |  | D |  |  | D |  |
| Queue Length 50th (ft) | 18 | 247 |  | 120 | 288 |  | 28 | 247 |  | 70 | 129 |  |
| Queue Length 95th (ft) | 30 | 186 |  | 217 | 366 |  | 57 | \#404 |  | 117 | 206 |  |
| Internal Link Dist (ft) |  | 799 |  |  | 397 |  |  | 204 |  |  | 454 |  |
| Turn Bay Length (ft) | 150 |  |  | 215 |  |  | 45 |  |  | 200 |  |  |
| Base Capacity (vph) | 327 | 1273 |  | 432 | 1606 |  | 424 | 520 |  | 221 | 517 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.25 | 0.68 |  | 0.67 | 0.57 |  | 0.14 | 0.80 |  | 0.64 | 0.42 |  |

## Intersection Summary

## Cycle Length: 120

Actuated Cycle Length: 120
Offset: $16(13 \%)$, Referenced to phase 2:SETL and 6:NWTL, Start of 1st Green
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.87
Intersection Signal Delay: 30.5
Intersection LOS: C
Intersection Capacity Utilization 83.9\%
ICU Level of Service E
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 3: 14th St \& IL Route 38


11：Randall Rd \＆Prairie St

|  | 4 |  |  |  |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个 |  | \％ | $\uparrow$ | 「 | \％ | 个个觡 |  | \％ | 个中解 |  |
| Volume（vph） | 62 | 38 | 117 | 226 | 41 | 370 | 60 | 1676 | 283 | 259 | 1884 | 28 |
| Satd．Flow（prot） | 1770 | 1652 | 0 | 1770 | 1863 | 1583 | 1770 | 4973 | 0 | 1770 | 5075 | 0 |
| FIt Permitted | 0.728 |  |  | 0.305 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（perm） | 1356 | 1652 | 0 | 568 | 1863 | 1583 | 1770 | 4973 | 0 | 1770 | 5075 | 0 |
| Satd．Flow（RTOR） |  | 100 |  |  |  | 127 |  | 36 |  |  | 3 |  |
| Lane Group Flow（vph） | 67 | 168 | 0 | 246 | 45 | 402 | 65 | 2130 | 0 | 282 | 2078 | 0 |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | pm＋ov | Prot | NA |  | Prot | NA |  |
| Protected Phases | 7 | 4 |  | 3 | 8 | 1 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  |  | 8 |  | 8 |  |  |  |  |  |  |
| Total Split（s） | 13.0 | 15.0 |  | 18.0 | 20.0 | 25.0 | 14.0 | 62.0 |  | 25.0 | 73.0 |  |
| Total Lost Time（s） | 4.0 | 6.5 |  | 4.0 | 6.5 | 4.0 | 4.0 | 6.5 |  | 4.0 | 6.5 |  |
| Act Effct Green（s） | 18.7 | 8.3 |  | 28.8 | 16.4 | 43.4 | 9.1 | 56.1 |  | 20.5 | 69.5 |  |
| Actuated g／C Ratio | 0.16 | 0.07 |  | 0.24 | 0.14 | 0.36 | 0.08 | 0.47 |  | 0.17 | 0.58 |  |
| v／c Ratio | 0.28 | 0.81 |  | 0.89 | 0.18 | 0.62 | 0.49 | 0.91 |  | 0.93 | 0.71 |  |
| Control Delay | 39.7 | 52.3 |  | 74.0 | 50.3 | 26.4 | 51.4 | 24.3 |  | 86.6 | 20.4 |  |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 39.7 | 52.3 |  | 74.0 | 50.3 | 26.4 | 51.4 | 24.4 |  | 86.6 | 20.4 |  |
| LOS | D | D |  | E | D | C | D | C |  | F | C |  |
| Approach Delay |  | 48.7 |  |  | 44.8 |  |  | 25.2 |  |  | 28.3 |  |
| Approach LOS |  | D |  |  | D |  |  | C |  |  | C |  |
| Queue Length 50th（ft） | 41 | 52 |  | 169 | 32 | 179 | 51 | 258 |  | 217 | 424 |  |
| Queue Length 95th（ft） | 81 | \＃169 |  | \＃279 | 70 | 294 | m70 | 280 |  | \＃379 | 484 |  |
| Internal Link Dist（ft） |  | 187 |  |  | 212 |  |  | 597 |  |  | 526 |  |
| Turn Bay Length（ft） | 70 |  |  | 100 |  | 100 | 145 |  |  | 170 |  |  |
| Base Capacity（vph） | 255 | 209 |  | 276 | 254 | 659 | 147 | 2345 |  | 309 | 2939 |  |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 2 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 |  |
| Reduced v／c Ratio | 0.26 | 0.80 |  | 0.89 | 0.18 | 0.61 | 0.44 | 0.91 |  | 0.91 | 0.71 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $100(83 \%)$ ，Referenced to phase 2：NBT and 6：SBT，Start of 1st Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v／c Ratio： 0.93 |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Signal Delay： 30.0 |  |  |  |  | Intersection LOS：C |  |  |  |  |  |  |  |
| Intersection Capacity Utilization 92．3\％ |  |  |  |  | ICU Level of Service F |  |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |  |  |  |  |  |  |
| \＃95th percentile volume exceeds capacity，queue may be longer． |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue shown is maximum after two cycles． |  |  |  |  |  |  |  |  |  |  |  |  |
| m Volume for 95th perc | equeue | metere | by upst | ream sig |  |  |  |  |  |  |  |  |

Splits and Phases：11：Randall Rd \＆Prairie St


Prairie Centre 11：15 am 4／16／2016 2026 Total Traffic with Randall Widening Timing Plan：Sat

Synchro 8 Report
Page 4

## APPENDIX J

CMAP Memo to St. Charles
Re: Traffic Projections
June 3, 2016

## Chicago Metropolitan Agency for Planning

Chris Bong, P.E.
Development Engineering Division Manager
City of St. Charles
2 East Main Street
St. Charles, IL 60174
Subject: Prairie Centre
City of St. Charles
Dear Mr. Bong:
In response to a request made on your behalf and dated June 3, 2016, we have developed year 2040 average daily traffic (ADT) projections for the subject location.

| ROAD SEGMENT | Year 2040 ADT |
| :--- | :---: |
| Randall Rd between IL 38 and Prairie St | 42,000 |
| IL 38 between Randall Rd and $14^{\mathrm{id}} \mathrm{St}$ | 17,000 |
| Prairie St between Randall Rd and $3^{\mathrm{d}} \mathrm{St}$ | 5,000 |
| $14^{4 \mathrm{~h}}$ St between IL 38 and Prairie St | 6,000 |

Traffic projections are developed using existing ADT data provided in the request letter and the results from the March 2016 CMAP Travel Demand Analysis. The regional travel model uses CMAP 2040 socioeconomic projections and assumes the implementation of the GO TO 2040 Comprehensive Regional Plan for the Northeastern Illinois area.

If you have any questions, please call me at (312) 386-8806.
Sincerely,


Jose Rodriguez, PTP, AICP
Senior Planner, Research \& Analysis

## APPENDIX K

Memo from St. Charles

Re: Alternative Development Plan
January 29, 2009


ST. CHARLES
SINCE 1834

## MEMO

## TO: Diane Lukas, Hampton Lenzini Renwick

FROM: Russell Colby, Planner
CC: David Patzelt, Shodeen
Rita Tungare, Director of Community Development
RE: $\quad$ St. Charles Towne Centre Traffic Impact Study- Alternative Development Plan
DATE: January 29, 2009

In conjunction with the second amendment to the traffic impact study for the St. Charles Towne Centre development, this memorandum provides a recommendation from City staff for an Alternative Development Plan for the site.

The Alternative Development plan is solely for the purpose of comparing the potential traffic impact of the proposed Towne Centre development to the potential traffic impact of the development of the site under the existing zoning classification.

For purposes of this analysis, the Alternative Development Plan represents the most intensive development of the site that would be reasonably possible as a retail-oriented development.

## PROPOSED LAND USE

The BR Regional Business Zoning District permits a diverse range of commercial land uses. However, based upon the existing configuration of the property and the existing surrounding uses, the site is most likely to develop with one or more of the uses classified as "Retail and Service Uses", as listed in the St. Charles Zoning Ordinance, Table 17.14-1 Permitted and Special Uses.

For purposes of traffic trip generation, City staff recommends utilizing the Institute of Traffic Engineers Land Use category "Shopping Center" (Land Use \#820). This category represents an integrated development that may include a variety of individual land uses, all of which are permitted under the existing $B R$ zoning classification.

## DEVELOPMENT SIZE

## Site Area

The proposed development includes a detention area located in an existing PUD development. This property cannot be used as a part of the site area for a development under the existing zoning.

| St. Charles Towne Centre Site Area | 30.59 acres |
| :---: | :---: |
| $14^{\text {th }}$ St basin property (existing PUD) | 2.39 acres (not available for development) |
| Net developable site area under BR | 28.2 acres (1,228,392 square feet) |

## Square footage

The total square footage of buildings that can be accommodated on the site is regulated by the "Building Coverage" and "Building Height" standards contained in the St. Charles Zoning Ordinance, Table 17.142, Bulk Regulations.

- The Building Coverage standard limits the ground floor square footage of any building structures to no more than $30 \%$ of the total site area.
- The Maximum Building Height limits buildings to 40 ft . Depending on the use of the building, this could allow for a 2 or 3 story structure. For "Retail and Service" uses, the ceiling heights for each level are typically too high to accommodate three full levels within the 40 ft . height requirement. Therefore, the maximum possible building height is assumed to be 2 stories.

|  | BR Zoning <br> Standard | BR standard applied to net developable site |
| :---: | :---: | :---: |
| Maximum Building <br> Coverage | $30 \%$ of lot area | $30 \% \times 1,228,392$ sf $=368,517$ square feet |
| Maximum Building <br> Height | 40 ft. | Approx. 2 stories for a Retail/Service use |
| Estimated <br> Maximum <br> Floor Area | 1 story building: 368,517 square feet <br> 2 story building ( 2 x 1 story building sf) |  |

## Adjustment to Gross-Leasable Area

The ITE "Shopping Center" land use trip generation calculations apply to Gross Leasable Area of a Shopping Center, as opposed to the overall Gross Floor Area. Gross Leasable Area is commonly derived by taking $85 \%$ of the Gross Floor Area.

| Estimated Max Gross <br> Floor Area | Adjustment to Gross <br> Leasible Area | Total square footage for purposes of <br> traffic generation |
| :---: | :---: | :---: |
| 737,034 square feet | $85 \%$ | 626,479 square feet |

## RECOMMENDATION

Staff recommends that for purposes of the second traffic study amendment for St. Charles Towne Centre, the Alternative Development Plan consist of a 626,479 sq. ft. Gross Leasable shopping center, categorized under ITE Land Use \#820.

## APPENDIX L

## ITE Land Use Diagrams

Weekday AM, Weekday PM, \& Saturday Peak Hours and Weekday Total

1. 220 - Apartment
2. 820 - Shopping Center
3. 826 - Specialty Retail Center (no Weekday AM Peak Hour)
4. 881 - Pharmacy/Drugstore with Drive-through Window
5. 932 - High-turnover (Sit-down) Restaurant
6. 934 - Fast-food Restaurant with Drive-through Window

## Apartment (220)

## Average Vehicle Trip Ends vs: Dwelling Units

## On a: Weekday, <br> Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Number of Studies: 78
Avg. Number of Dwelling Units: 235
Directional Distribution: 20\% entering, $80 \%$ exiting
Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.51 | $0.10-1.02$ | 0.73 |

Data Plot and Equation


## Average Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Number of Studies: 90
Avg. Number of Dwelling Units: 233
Directional Distribution: 65\% entering, 35\% exiting
Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.62 | $0.10-1.64$ | 0.82 |

## Data Plot and Equation



## Apartment <br> (220)

## Average Vehicle Trip Ends vs: Dwelling Units <br> On a: Saturday, <br> Peak Hour of Generator

## Number of Studies: 14

Avg. Number of Dwelling Units: 178
Directional Distribution: Not available

Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.52 | $0.26-1.05$ | 0.74 |

## Data Plot and Equation



Fitted Curve Equation: $T=0.41(X)+19.23$
$\mathrm{R}^{2}=0.56$

## Apartment <br> (220)

## Average Vehicle Trip Ends vs: Dwelling Units <br> On a: Weekday

Number of Studies: 88
Avg. Number of Dwelling Units: 210
Directional Distribution: 50\% entering, 50\% exiting
Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 6.65 | $1.27-12.50$ | 3.07 |

## Data Plot and Equation



## Shopping Center <br> (820)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Leasable Area
On a: Weekday,
Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Number of Studies: 104
Average 1000 Sq. Feet GLA: 310
Directional Distribution: 62\% entering, 38\% exiting
Trip Generation per 1000 Sq. Feet Gross Leasable Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 0.96 | $0.10-9.05$ | 1.31 |

## Data Plot and Equation



## Shopping Center (820)

## Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Leasable Area On a: Weekday, <br> Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Number of Studies: 426
Average 1000 Sq. Feet GLA: 376
Directional Distribution: 48\% entering, $52 \%$ exiting
Trip Generation per 1000 Sq. Feet Gross Leasable Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 3.71 | $0.68-29.27$ | 2.74 |

Data Plot and Equation


## Shopping Center (820)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Leasable Area
On a: Saturday, Peak Hour of Generator

| Number of Studies: | 128 |
| ---: | :--- |
| Average 1000 Sq. Feet GLA: | 458 |

Directional Distribution: 52\% entering, $48 \%$ exiting
Trip Generation per 1000 Sq. Feet Gross Leasable Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 4.82 | $1.46-18.32$ | 3.10 |

Data Plot and Equation


## Shopping Center <br> (820)

## Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Leasable Area On a: Weekday

Number of Studies: 302<br>Average 1000 Sq. Feet GLA: 331<br>Directional Distribution: 50\% entering, 50\% exiting

Trip Generation per 1000 Sq. Feet Gross Leasable Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 42.70 | $12.50-270.89$ | 21.25 |

## Data Plot and Equation



## Specialty Retail Center (826)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Leasable Area On a: Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Number of Studies: 5
Average 1000 Sq. Feet GLA: 69
Directional Distribution: 44\% entering, $56 \%$ exiting
Trip Generation per 1000 Sq. Feet Gross Leasable Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 2.71 | $2.03-5.16$ | 1.83 |

Data Plot and Equation Caution - Use Carefully - Small Sample Size


## Specialty Retail Center

(826)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Leasable Area On a: Saturday

Number of Studies: 3
Average 1000 Sq. Feet GLA: 28 Directional Distribution: 50\% entering, 50\% exiting

Trip Generation per 1000 Sq. Feet Gross Leasable Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 42.04 | $22.57-54.47$ | 13.97 |

Data Plot and Equation


## Specialty Retail Center <br> (826)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Leasable Area On a: Weekday

Number of Studies: 4
Average 1000 Sq. Feet GLA: 25
Directional Distribution: 50\% entering, 50\% exiting
Trip Generation per 1000 Sq. Feet Gross Leasable Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 44.32 | $21.30-64.21$ | 15.52 |

Data Plot and Equation

$\times$ Actual Data Points
Fitted Curve Equation: $\mathbf{T}=\mathbf{4 2 . 7 8 ( X )} \mathbf{+ 3 7 . 6 6}$

$$
R^{2}=0.69
$$

# Pharmacy/Drugstore with Drive-Through Window (881) 

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Number of Studies: 13
Average 1000 Sq. Feet GFA: 13
Directional Distribution: 52\% entering, $48 \%$ exiting
Trip Generation per 1000 Sq. Feet Gross Floor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 3.45 | $1.93-5.92$ | 2.10 |

## Data Plot and Equation



## Pharmacy/Drugstore with Drive-Through Window (881)

Average Vehicle Trip Ends vs: 1000 Sq . Feet Gross Floor Area On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Number of Studies: 31
Average 1000 Sq. Feet GFA: 14
Directional Distribution: 50\% entering, 50\% exiting
Trip Generation per 1000 Sq. Feet Gross Floor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 9.91 | $4.85-20.43$ | 5.04 |

Data Plot and Equation


## Pharmacy/Drugstore with Drive-Through Window

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area<br>On a: Saturday,<br>Peak Hour of Generator

| Number of Studies: | 14 |
| ---: | :--- |
| Average 1000 Sq. Feet GFA: | 14 |
| Directional Distribution: | $49 \%$ entering, $51 \%$ exiting |

Trip Generation per 1000 Sq. Feet Gross Floor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 8.20 | $4.31-11.40$ | 3.57 |

## Data Plot and Equation



## Pharmacy/Drugstore with Drive-Through Window

## Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area On a: Weekday

Number of Studies: 10
Average 1000 Sq. Feet GFA: 13
Directional Distribution: $50 \%$ entering, $50 \%$ exiting
Trip Generation per 1000 Sq. Feet Gross Floor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 96.91 | $67.09-133.45$ | 21.59 |

Data Plot and Equation


## High-Turnover (Sit-Down) Restaurant (932)

## Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area

On a: Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Number of Studies: 24
Average 1000 Sq. Feet GFA: 6
Directional Distribution: 55\% entering, 45\% exiting
Trip Generation per 1000 Sq. Feet Gross Floor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 10.81 | $2.32-25.60$ | 6.59 |

## Data Plot and Equation



## High-Turnover (Sit-Down) Restaurant

(932)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area
On a: Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Number of Studies: 60
Average 1000 Sq. Feet GFA: 6
Directional Distribution: 60\% entering, $40 \%$ exiting
Trip Generation per 1000 Sq. Feet Gross Floor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 9.85 | $0.92-62.00$ | 8.54 |

## Data Plot and Equation



## High-Turnover (Sit-Down) Restaurant

 (932)Average Vehicle Trip Ends vs: 1000 Sa. Feet Gross Floor Area<br>On a: Saturday,<br>Peak Hour of Generator

Number of Studies: 8
Average 1000 Sq. Feet GFA: 4
Directional Distribution: $53 \%$ entering, $47 \%$ exiting
Trip Generation per 1000 Sc. Feet Gross Ploor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 14.07 | $4.44-50.40$ | 12.19 |

## Data Plot and Equation



[^3]Fitted Curve Equation: Not given
$R^{2}=* * * *$

## High-Turnover (Sit-Down) Restaurant (932)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area On a: Weekday

Number of Studies: 14
Average 1000 Sq. Feet GFA: 7
Directional Distribution: 50\% entering, 50\% exiting
Trip Generation per 1000 Sq. Feet Gross Floor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 127.15 | $73.51-246.00$ | 41.77 |

Data Plot and Equation


# Fast-Food Restaurant with Drive-Through Window (934) 

Average Vehicle Tun Ents us: 1000 Sq. Feet Gross Floor Area
On a: Neeleday,
 me hour soveen 7 and 9 arm.

## Number of Sudies: 75

Average 1000 Sq . Feet GFA: A
Directional Distribution: $51 \%$ entering, $49 \%$ exiting
Trip Generation per 1000 Sg Feet Croms por ${ }^{\text {Guta }}$

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 45.42 | $1.02-163.33$ | 28.63 |

Data Plot ant Equation


## Fast-Food Restaurant with Drive-Through Window (934)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area Ona: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Number of Studies; 132
Average 1000 Sq. Feet GFA: 3
Directional Distribution: $52 \%$ entering, $48 \%$ exiting
Trip Generation per 1000 Sq. Feet Gross Ploor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 32.65 | $7.96-117.15$ | 19.73 |

Data Plot and Equation


## Fast-Food Restaurant with Drive-Through Window

 (934)Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area<br>On ${ }^{\text {a: }}$ Saturday,<br>Peak Hour of Generator

Number of Studies: 41<br>Average 1000 Sq . Feet GFA: 4<br>Directional Distribution: $51 \%$ entering, $49 \%$ exiting

Trip Generation per 1000 Sq. Feet Cross Fioor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 59.00 | $19.21-122.49$ | 22.89 |

## Data Plot and Equation



# Fast-Food Restaurant with Drive-Through Window (934) 

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area Ona: Weekday

Number of Studies: 21<br>Average 1000 Sq. Feet GFA: 3<br>Directional Distribution: 50\% entering, $50 \%$ exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

| Average Rate | Range of Rates | Standard Deviation |
| :---: | :---: | :---: |
| 496.12 | $195.98-1132.92$ | 242.52 |

Data Plot and Equation


## APPENDIX M

## Trip Generation Diagrams

1. Prairie Centre Site Generated Traffic
2. CVS Pharmacy
3. Alternate Development
(All trips generated using ITE's Trip Generation Manual, $9^{\text {th }}$ ed.)

| AM Peak Hour | Land Use Type | Trip Generation | Development | Number of | Total | Directional | istribution |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Parcel | (ITE Code) ITE Land Use | Rate | Units | Units, $X$ | Trips, $T$ | Entering \% | Exiting \% | Entering | Exiting |
| Residential Units | (220) Apartment | $\mathrm{T}=0.49 \mathrm{X}+3.73$ | Dwelling Units | 609 | 302 | 20 | 80 | 60 | 242 |
| Retail Units | (826) Specialty Retail Center | $\operatorname{Ln}(\mathrm{T})=0.61 \operatorname{Ln}(\mathrm{X})+2.24$ | 1000 SF | 83.325 | 139 | 62 | 38 | 86 | 53 |
| Sit-down Restaurants | (932) High-turnover (Sit-down) Restaurant | 10.81 | 1000 SF | 26.200 | 283 | 55 | 45 | 156 | 127 |
| Fast-food Restaurants | (934) Fast-food Restaurant w/Drive-through Window | 45.42 | 1000 SF | 6.950 | 316 | 51 | 49 | 161 | 155 |
| Note: Pass-by reduction not applied to Residential Units in any time period studied. <br> Shopping Center (820) used for AM Peak Hour only for Retail Units. See report for explanation. |  |  | Total Trips: |  |  |  |  | 463 | 577 |
|  |  |  | 15\% Internal Capture: |  |  |  |  | (69) | (87) |
|  |  |  | Total Trips at Development Driveways: |  |  |  |  | 394 | 490 |
|  |  |  | 5\% |  |  | Pass-by Adjustment: |  | (17) | (14) |
|  |  |  | 377 | 476 |  |  |


| PM Peak Hour | Land Use Type (ITE Code) ITE Land Use | $\begin{gathered} \hline \text { Trip Generation } \\ \text { Rate } \\ \hline \end{gathered}$ | DevelopmentUnits | Number of Units, $X$ | Total <br> Trips, $T$ | Directional Distribution |  | Trips |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Parcel |  |  |  |  |  | Entering \% | Exiting \% | Entering | Exiting |
| Residential Units | (220) Apartment | $\mathrm{T}=0.55 \mathrm{X}+17.65$ | Dwelling Units | 609 | 353 | 65 | 35 | 229 | 124 |
| Retail Units | (826) Specialty Retail Center | $\mathrm{T}=2.40 \mathrm{x}+21.48$ | 1000 SF | 83.325 | 221 | 44 | 56 | 97 | 124 |
| Sit-down Restaurants | (932) High-turnover (Sit-down) Restaurant | 9.85 | 1000 SF | 26.200 | 258 | 60 | 40 | 155 | 103 |
| Fast-food Restaurants | (934) Fast-food Restaurant w/Drive-through Window | 32.65 | 1000 SF | 6.950 | 227 | 52 | 48 | 118 | 109 |
|  |  |  | Total Trips: |  |  |  |  | 599 | 460 |
|  |  |  | 15\% Internal Capture: |  |  |  |  | (90) | (69) |
|  |  |  | Total Trips at Development Driveways: |  |  |  |  | 509 | 391 |
|  |  |  | 5\% |  |  | Pass-by Adjustment: |  | (16) | (14) |
|  |  |  |  |  |  | TOTAL New Trips: |  | 493 | 377 |


| Saturday Peak Hour | Land Use Type | Trip Generation | Development | Number of | Total | Directional | istribution |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Parcel | (ITE Code) ITE Land Use | Rate | Units | Units, $X$ | Trips, $T$ | Entering \% | Exiting \% | Entering | Exiting |
| Residential Units | (220) Apartment | 0.52 | Dwelling Units | 609 | 317 | 50 | 50 | 159 | 158 |
| Retail Units | (826) Specialty Retail Center | 0.4204 | 1000 SF | 83.325 | 35 | 50 | 50 | 18 | 17 |
| Sit-down Restaurants | (932) High-turnover (Sit-down) Restaurant | 14.07 | 1000 SF | 26.200 | 369 | 53 | 47 | 196 | 173 |
| Fast-food Restaurants | (934) Fast-food Restaurant w/Drive-through Window | 59.00 | 1000 SF | 6.950 | 410 | 51 | 49 | 209 | 201 |
| Note: Specialty Retail Saturday Peak Hour Trip Generation Rate estimated as 10\% of Saturday whole day rate. |  |  | Total Trips: |  |  |  |  | 582 | 549 |
|  |  |  | 15\% Internal Capture: |  |  |  |  | (87) | (82) |
|  |  |  | Total Trips at Development Driveways: |  |  |  |  | 495 | 467 |
|  |  |  | 5\% |  |  | Pass-by Adjustment: |  | (18) | (17) |
|  |  |  | TOTAL New Trips: | 477 | 450 |


| Weekday Total | Land Use Type (ITE Code) ITE Land Use | Trip GenerationRate | Development Units | Number of Units, $X$ | $\begin{array}{\|c\|} \hline \text { Total } \\ \text { Trips, } T \\ \hline \end{array}$ | Directional Distribution |  | Trips |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Parcel |  |  |  |  |  | Entering \% | Exiting \% | Entering | Exiting |
| Residential Units | (220) Apartment | $\mathrm{T}=6.06 \mathrm{x}+123.56$ | Dwelling Units | 609 | 3814 | 50 | 50 | 1907 | 1907 |
| Retail Units | (826) Specialty Retail Center | 44.32 | 1000 SF | 83.325 | 3693 | 50 | 50 | 1847 | 1846 |
| Sit-down Restaurants | (932) High-turnover (Sit-down) Restaurant | 127.15 | 1000 SF | 26.200 | 3331 | 50 | 50 | 1666 | 1665 |
| Fast-food Restaurants | (934) Fast-food Restaurant w/Drive-through Window | 496.12 | 1000 SF | 6.950 | 3448 | 50 | 50 | 1724 | 1724 |
|  |  |  | Total Trips: |  |  |  |  | 7144 | 7142 |
|  |  |  | 15\% Internal Capture: |  |  |  |  | (1072) | (1071) |
|  |  |  | Total Trips at Development Driveways: |  |  |  |  | 6072 | 6071 |
|  |  |  | 5\% |  |  | Pass-by Adjustment: |  | (223) | (222) |
|  |  |  |  |  |  | TOTAL New Trips: |  | 5849 | 5849 |

St Charles Prairie Centre
Trip Generation Table - CVS
(All trips generated using ITE's Trip Generation Manual, $9^{\text {th }}$ ed.)
6/7/2016



| Saturday Peak Hour | Land Use Type | Trip Generation | Development | Number of | Total | Directional | istribution |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Parcel | (ITE Code) ITE Land Use | Rate | Units | Units, $X$ | Trips, $T$ | Entering \% | Exiting \% | Entering | Exiting |
| CVS | (881) Pharmacy/Drugstore w/Drive-through Window | 8.20 | 1000 SF | 13.225 | 108 | 49 | 51 | 53 | 55 |
| CVS outlot | (826) Specialty Retail Center | 0.4204 | 1000 SF | 4.620 | 2 | 50 | 50 | 1 | 1 |
| Note: Specialty Retail Saturday Peak Hour Trip Generation Rate estimated as 10\% of Saturday whole day rate. |  |  | Total Trips: |  |  |  |  | 54 | 56 |
|  |  |  | 10\% Pass-by Adjustment: |  |  |  |  | (5) | (6) |
|  |  |  | TOTAL New Trips: |  |  |  |  | 49 | 50 |



Trip Generation Table - Alternate Development
(All trips generated using ITE's Trip Generation Manual, $9^{\text {th }}$ ed.)

Hampton, Lenzini, \& Renwick, Inc., Elgin, IL
HLR Project: 16.0220.350 Analyst: HLR

|  |  |  |  |  |  |  | Analyst. HLR |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AM Peak Hour | Land Use Type(ITE Code) ITE Land Use | Trip Generation Rate | Development Units | Number of Units, X | Total Trips, $T$ | Directional Distribution |  | Trips |  |
| Parcel |  |  |  |  |  | Entering \% | Exiting \% | Entering | Exiting |
| Prairie Centre Site | (820) Shopping Center | $\operatorname{Ln}(\mathrm{T})=0.61 \mathrm{Ln}(\mathrm{X})+2.24$ | 1000 SF | 626.479 | 477 | 62 | 38 | 296 | 181 |
|  |  |  | Total Trips: |  |  |  |  | 296 | 181 |
|  |  |  | 20\% |  |  | Pass-by Adjustment: |  | (59) | (36) |
|  |  |  | TOTAL New Trips: |  |  |  |  | 237 | 145 |





## APPENDIX N

ITE Trip Generation Handbook
Chapter 7
Multi-use Development

## CHAPTER 7

## Multi-Use Development

### 7.1 Background

A basic premise behind the data presented in Trip Generation is that they were collected at single-use, free-standing sites. However, the development of mixed-use or multi-use sites is increasingly popular. While the trip generation rates for individual uses on such sites may be the same or similar to what they are for free-standing sites, there is potential for interaction among those uses within the multi-use site, particularly where the trip can be made by walking. As a result, the total generation of vehicle trips entering and exiting the multi-use site may be reduced from simply a sum of the individual, discrete trips generated by each land use.

A common example of this internal trip-making occurs at a multi-use development containing offices and a shopping/service area. Some of the trips made by office workers to shops, to restaurants, or to banks may occur on site. These types of trips are defined as internal to (i.e., "captured" within) the multi-use site.

### 7.2 What Is a MultiUse Development?

For purposes of this handbook, a mutti-use development is typically a single real-estate project tbat consists of two or more ITE land use classifications between which trips can be made without using the off-site road system. Because of the nature of these land uses, the
trip-making characteristics are interrelated, and some trips are made among the on-site uses. This capture of trips internal to the site has the net effect of reducing vehicle trip generation between the overall development site and the external street system (compared to the total number of trips generated by comparable, stand-alone sites).

Multi-use developments are commonly found ranging in size from 100,000 square feet to 2 million square feet. The data presented in this chapter correspond to multiuse developments in this size range. The recommended procedures for estimating trip generation at multiuse developments are likely applicable at even larger sites, but the analyst is encouraged to collect additional data.

A key characteristic of a multi-use development is that trips among the various land uses can be made on site and these internal trips are not made on the major street system. In some multi-use developments, these internal trips can be made either by walking or by vehicle entirely on internal pathways or internal roadways without using streets external to the site.

An internal capture rate can generally be defined as a percentage reduction that can be applied to the trip generation estimates for individual land uses to account for trips internal to the site. It is important to note that these reductions are applied externally to the site (i.e., at entrances, at adjacent intersections,

Multi-Use Devefopment
typically planned as a single real-estate project,

- typically between 100,000 and 2 million square feet in size,
- contains two or more land uses,
- some trips are between onsite land uses, and
- trips between land uses do not travel on major street system.


## Not

- a central business district,
- a suburban activity center, or
- an existing ITE land use classification with potential for a mix of land uses, such as
- a shopping center,
- an office park with retail,
- an office building with retail, or
- a hotel with limited retall and restaurant space.
and on adjacent roadways). The trip reduction for internally captured trips is separate from the reduction for pass-by trips. These are two distinct phenomena, and both could be applicable for a proposed development. The internal trips, if present, should be subtracted out before pass-by trip reductions are applied (refer to chapter 5 for a complete discussion of pass-by trip estimation).


## 7.3 What Is Not a Multi-Use Development?

In literal terms, a multi-use development could mean any combination of different land use types within a defined, congruous area. But that definition would encompass a wide range of potential applications, many of which are not intended to be the focus of this chapter.

A traditional downtown or central business district (CBD) is not considered a multi-use development for purposes of this handbook. Downtown areas typically have a mixture of diverse employment, retail, residential, commercial, recreation, and hotel uses. Extensive pedestrian interaction occurs because of the scale of the downtown area, the ease of access, and the proximity of the various uses. Automobile occupancy, particularly during peak commuting hours, is usually higher in the CBD than in the outlying areas. Some downtowns have excellent transit service. For these reasons, trip generation characteristics in a downtown environment are different from those found in outlying or suburban areas. The focus of the data presented throughout Trip Generation is on sites in suburban settings with limited or no transit service and with free parking.
Accordingly, trip generation characteristics in this cbapter, and specifically in the case of capture rates at multi-use developments, are directly applicable only to sites outside the traditional downtown. The potential effects of transit service and on site parking fees are discussed in appendix B.

A shopping center could also be considered a multi-use development. However, because data have been collected directly for them, shopping centers are considered in Trip Generation as a single land use. The associated trip generation rates and equations given in Trip Generation reflect the "multi-use" nature of the development because of the way shopping center data have been collected. Accordingly, internal capture rates are not applicable and sbould not be used to forecast trips for shopping centers if using Land Use Code 820 statistics and data. However, if the shopping center is planned to have out-parcel development of a significantly different land use classification or a very large percentage of overall GLA, the site could be considered a multi-use development for the purpose of estimating site trip generation.

Likewise, a subdivision or planned unit development containing general office buildings and support services such as banks, restaurants, and service stations arranged in a park- or campus-like atmosphere should be considered as an office park (Land Use Code 750), not as a multi-use development. Similarly, office buildings with support retail or restaurant facilities contained inside the building should be treated as general office buildings (Land Use Code 710) because the trip generation rates and equations already reflect such support uses. A hotel with an on-site restaurant and small retail falls within Land Use Code 310 and should not be treated as a multi-use development.

> Methodology for Estimating Trip Generation at MultiUse Sites

Internally captured trips can be a significant component in the travel patterns at multi-use developments. However, more studies are needed to thoroughly quantify this phenomenon. Section 7.5 presents a recommended procedure for estimating internal capture rates (and a worksheet for organizing and documenting the analysis assumptions used in the estimation of the internal capture rates) for multi-use development sites.

The internal trip-making characteristics of multi-use development sites are directly related to the mix of onsite land uses (which are typically a combination of residential, office, shopping/retail, restaurant, entertainment, and hotel/motel). When combined within a single mixed-use development, these land uses tend to interact, and thus to attract a portion of each other's trip generation.

The recommended methodology for estimating internal capture rates and trip generation at multi-use sites is based on two fundamental assumptions. First, the proportions of trips between interacting land use types (which will be satisfied internally by pairs of land uses) are assumed to be relatively stable. Second, if sufficient data were available, these internal capture percentages could be predicted with adequate confidence. The need for additional data collection at multi-use developments is described in section 7.7.

As should be expected, the observed internal capture rates for multi-use developments vary by time of day, by the site's mix of land uses, and by the size of the development.

Several premises frame the recommended methodology. An example to illustrate their application is presented in the highlighted text to the side. Key to the success of this methodology in replicating internal capture patterns at multi-use sites is its iterative, balancing steps that constrain internal trip-making levels to what are realistic given the mix of land uses.

> Illustration of Methodology Overview
> Assume a mult-use development with a mix of office, retail, and residential uses. Assume that the office building generates 500 exiting trips during the evening peak hour (based on factors presented in Thip Generation).

Based on surveys at other multi-use developments for illustration purposes), it is estimated that the 500 peak hour trips could go to the following destinations: 5 trips to another office building within the development, 115 trips to a retail site within the development, 10 trips to residential units on-site, and 370 to external sites (as illustrated in figure 7.1a).

What if there are no on-site residential units? The number of trips from the office to an internal residential destination changes to zero and the number of trips to external destinations becomes 380 (i.e,, the total trips from the office building remains constant at 500 ).

What if there are a large number of on-site residences? Assume the residential uses generate 1,000 entering trips during the evening peak hour. As illustrated in figure 7.16, the trips are assumed to originate as follows: $\mathbf{2 0}$ trips from an on-site office building, 310 trips from on-site retail, no trips from another on-site residential component, and 670 trips from external origins.

With the larger number of residences, as many as 20 trips could come from on-site office buildings. But the actual on-site office buildings generate only 10 trips to the on-site residential land use. So, 10 trips would be expected from on-site office to on-site residential in figure 7.1c. The key assumption is that the "balanced" number of internal trips will match the controlling (f.e., lower) value.

Figure 7.1 Illustration of Internal Trip Balancing for a Multi-Use Development

## DISTRIBUTION OF POTENTIAL DESTINATIONS OF TRIPS FROM OFFICE USE


b. DISTRIBUTION OF POTENTIAL ORIGINS OF TRIPS TO RESIDENTIAL USE

c. BALANCED ${ }^{1}$ DISTRIBUTION OF ORIGINS OF TRIPS TO RESIDENTIAL USE

${ }^{1}$ Only the office-to-residential values have been "balanced." Each other pair of land uses would likewise need to be balanced.

Premise 1: The distribution of trip purposes among motorists entering or exiting a development site is relatively stable. The distribution of destination land uses is likewise assumed to be relatively stable. For example, the destinations of trips from an office building are distributed among the many potential destinations (e.g., retail, residential, other office) in roughly the same pattern whether the office is stand-alone or in a multi-use development.

Premise 2: The converse of Premise 1 is also true, that the distribution of origins for trips to a particular land use is relatively stable.

Premise 3: The number of trips from a land use within a multi-use development to another land use within the same multi-use development (i.e., an internal trip) is a function of the size of the "receiving" land use and the number of trips it attracts as well as the size of
the "originating" land use and the number of trips it sends. The number of trips between a particular pair of internal land uses is limited to the smaller of these two values.

## APPENDIX O

Internal Capture Diagram



|  | Net External Trips for Multi-use Development |  |  |  |
| ---: | :---: | :---: | :---: | :---: |
|  | Land Use A | Land Use B | Land Use C | Total |
| Enter | 193 | 78 | 249 | 520 |
| Exit | 103 | 97 | 181 | 381 |
| Total | 296 | 175 | 430 | 901 |

## APPENDIX P

## Traffic Counts

Weekday AM, Weekday PM, \& Saturday Peak Hours

1. IL Route 38 \& Randall Road
2. IL Route 38 \& Jewel Driveway
3. IL Route 38 \& West Mall Entrance
4. IL Route 38 \& East Mall Entrance/Vanderbilt Drive
5. IL Route 38 \& $14^{\text {th }}$ Street/Bricher Road
6. Randall Road \& Prairie Street
7. Prairie Street \& Jewel Driveway
8. Prairie Street \& West Mall Entrance
9. Prairie Street \& East Mall Entrance
10. Prairie Street \& Covington Court/Wessel Court
11. Prairie Street \& 16th Street
12. Prairie Street \& 14th Street
13. Prairie Street \& 7th Street
14. Prairie Street \& $3^{\text {rd }}$ Street
15. $14^{\text {th }}$ Street $\&$ Vanderbilt Drive
16. $14^{\text {th }}$ Street \& Covington Court/Horne Street

















































## APPENDIX Q

## Traffic Signal Warrant Analysis Reports

2016 Existing Traffic, 2026 Base Traffic, \& 2026 Total Traffic

1. Prairie Street \& 14th Street
a. 2016 Existing Traffic
b. 2026 Base Traffic
c. 2026 Total Traffic
2. Prairie Street \& 7th Street
a. 2016 Existing Traffic
b. 2026 Base Traffic
c. 2026 Total Traffic
3. Prairie Street \& $3^{\text {rd }}$ Street
a. 2016 Existing Traffic
b. 2026 Base Traffic
c. 2026 Total Traffic

## TRAFFIC SIGNAL WARRANT REVIEW SHEET

Intersection: Prairie Street and 14th Street
2016 Existing Traffic
Municipality: City of St. Charles

Speed limit of major route: 30

Number of lanes for major approach: 1

Isolated Community with population <10,000? No
Number of lanes for minor approach: 1

SRA: No


## TRAFFIC SIGNAL WARRANT REVIEW SHEET

Intersection: Prairie Street and 14th Street

Municipality: City of St. Charles
Speed limit of major route:

Number of lanes for major approach: 1

2026 Base Traffic

Isolated Community with population $<10,000$ ? No

Number of lanes for minor approach: 1

SRA: No


| Warrant Number | Requirement <br> Satisfied? |
| :---: | :---: |
| Warrant 1 Condition <br> A |  |
| Minimum Vehicular <br> Volume |  |
| Warrant 1 Condition <br> B |  |
| Interruption of Continous <br> Traffic |  |


| Warrant 1 Combo |  |
| :---: | :---: |
| Combination of Condition <br> A and Condition B | Yeso |
| Warrant 2 | Not Evaluated |

Four Hour Volume

| Warrant 3 <br> Peak Hour | Yes No |
| :---: | :---: |
| Warrant 4 <br> Pedestrian Volume | Yes No |
| Warrant 5 <br> School Crossing | Yes No |
| Warrant 6 <br> Coordinated Signal System | Yes No |
| Warrant 7 <br> Crash Experience | Not Evaluated |
| Total \# of crashes: <br> Number of correctable crashes Less restrictive remedies tried? Volume requirements met? |  |
| Warrant 8 <br> Roadway Network | Yes No |
| Warrant 9 Grade Crossing | Yes NO |

## TRAFFIC SIGNAL WARRANT REVIEW SHEET

Intersection: Prairie Street and 14th Street

## Municipality: City of St. Charles

Speed limit of major route:

Number of lanes for major approach: 1

2026 Total Traffic

Isolated Community with population $<10,000$ ? No

Number of lanes for minor approach: 1

SRA: No


| Warrant Number | Requirement Satisfied? |
| :---: | :---: |
| Warrant 1 Condition A <br> Minimum Vehicular Volume |  |
| Warrant 1 Condition B <br> Interruption of Continous Traffic | $\mathrm{Yes} \mathrm{No}$ |
| Warrant 1 Combo Combination of Condition A and Condition B | Yes No |
| Warrant 2 <br> Four Hour Volume | Not Evaluated |
| Warrant 3 <br> Peak Hour | Yes No |
| Warrant 4 <br> Pedestrian Volume | Yes No |
| Warrant 5 <br> School Crossing | Yes No |
| Warrant 6 <br> Coordinated Signal System | $\mathrm{Yes} \text { No }$ |
| Warrant 7 <br> Crash Experience | Not Evaluated |
| Total \# of crashes: <br> Number of correctable crashes: Less restrictive remedies tried Volume requirements met? |  |
| Warrant 8 <br> Roadway Network | Yes No |
| Warrant 9 <br> Grade Crossing | Yes No |

## TRAFFIC SIGNAL WARRANT REVIEW SHEET

Intersection: Prairie Street and 7th Street
2016 Existing Traffic
Municipality: City of St. Charles

Speed limit of major route: 30

Number of lanes for major approach: 1

Isolated Community with population <10,000? No
Number of lanes for minor approach: 1

SRA: No


## TRAFFIC SIGNAL WARRANT REVIEW SHEET

Intersection: Prairie Street and 7th Street

Municipality: City of St. Charles
Speed limit of major route:

Number of lanes for major approach: 1

2026 Base Traffic

Isolated Community with population $<10,000$ ? No

Number of lanes for minor approach: 1

SRA: No


| Warrant Number | Requirement Satisfied? |
| :---: | :---: |
| Warrant 1 Condition A <br> Minimum Vehicular Volume | Yes No |
| Warrant 1 Condition B <br> Interruption of Continous Traffic | Yes No |
| Warrant 1 Combo <br> Combination of Condition A and Condition B | Yes No |
| Warrant 2 <br> Four Hour Volume | Not Evaluated |
| Warrant 3 <br> Peak Hour | Yes No |
| Warrant 4 | Yes No |
| Warrant 5 <br> School Crossing | Yes No |
| Warrant 6 <br> Coordinated Signal System | $\mathrm{Yes} \mathrm{No}$ |
| Warrant 7 <br> Crash Experience | Not Evaluated |
| Total \# of crashes: <br> Number of correctable crashes Less restrictive remedies tried Volume requirements met? |  |
| Warrant 8 <br> Roadway Network | Yes No |
| Warrant 9 Grade Crossing | Yes No |

## TRAFFIC SIGNAL WARRANT REVIEW SHEET

Intersection: Prairie Street and 7th Street

## Municipality: City of St. Charles

Speed limit of major route:
Number of lanes for major approach: 1

2026 Total Traffic

Isolated Community with population $<10,000$ ? No

Number of lanes for minor approach: 1

SRA: No


## TRAFFIC SIGNAL WARRANT REVIEW SHEET

Intersection: Prairie Street and 3rd Street

Municipality: City of St. Charles

30

Number of lanes for major approach: 1

## 2016 Existing Traffic

Number of lanes for minor approach: 1
SRA: No


## TRAFFIC SIGNAL WARRANT REVIEW SHEET

Intersection: Prairie Street and 3rd Street
2026 Base Traffic

## Municipality: City of St. Charles

Speed limit of major route: 30

Number of lanes for major approach: 1

Isolated Community with population $<10,000$ ? No

Number of lanes for minor approach: 1

SRA: No


## TRAFFIC SIGNAL WARRANT REVIEW SHEET

Intersection: Prairie Street and 3rd Street

Municipality: City of St. Charles
Speed limit of major route:
Number of lanes for major approach: 1

2026 Total Traffic

Isolated Community with population $<10,000$ ? No
Number of lanes for minor approach: 1

SRA: No

| Hour | Veh. per hr. on major street (total of both approaches) | Veh. per hr. on higher volume minor street approach (one direction only) | Check any hours that meet the following warrants |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{array}{\|c\|} \hline \text { Warrant } 1 \\ \text { Condition } \\ \text { A } \end{array}$ | $\begin{array}{\|c} \hline \text { Warrant 1 } \\ \text { Condition } \\ \text { B } \end{array}$ | Warrant 1 Combo | Warrant 2 | Warrant 3 | Warrant 4 |
|  |  |  |  |  |  |  |  |  |
| 7:00 AM | 869 | 315 | X | X | X | X | X |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $\downarrow$ |  |  |  |  |  |  |  |  |
| $55 \%$ of DHV | 584 | 220 | X |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| $\downarrow$ |  |  |  |  |  |  |  |  |
| 5:00 PM | 1062 | 400 | X | X | X | X | X |  |
|  |  |  |  |  |  |  |  |  |



Warrant 1 Condition B
Yes No
Interruption of Continous
Traffic
Warrant 1 Combo

Combination of Condition


A and Condition B
Warrant 2
Not Evaluated
Four Hour Volume

| Warrant 3 <br> Peak Hour | Yes No |
| :---: | :---: |
| Warrant 4 <br> Pedestrian Volume | $\mathrm{Yes} \mathrm{No}$ |
| Warrant 5 <br> School Crossing | Yes No |
| Warrant 6 <br> Coordinated Signal System | Yes No |
| Warrant 7 <br> Crash Experience | Not Evaluated |
| Total \# of crashes: <br> Number of correctable crashes: <br> Less restrictive remedies tried? <br> Volume requirements met? |  |
| Warrant 8 <br> Roadway Network | Yes No |
| Warrant 9 Grade Crossing | Yes ${ }^{\text {No }}$ |


[^0]:    Copyright © 2016 University of Florida. All Rights Reserved.

[^1]:    Prairie Centre 7:15 am 4/12/2016 2026 Projected Traffic
    Timing Plan: AM

[^2]:    Prairie Centre 11：15 am 4／16／2016 2026 Total Traffic with Randall Widening Timing Plan：Sat

[^3]:    Actual Data Points
    Average Rate

